

#### **INTERSECTION SAFETY STUDY**

U.S. BUSINESS 41/S.R. 45/8<sup>TH</sup> AVENUE WEST AT 23<sup>RD</sup> STREET WEST

MANATEE COUNTY SECTION NO.: 13030000 MILEPOST: 0.867

FM No. 409224-1-32-91 Contract No. C9M86

September 2018

#### PREPARED FOR

Florida Department of Transportation, District 1

Project Manager: Michael Kautz 801 N. Broadway Street

Bartow, Florida 33830

#### PROFESSIONAL ENGINEER ENDORSEMENT

I hereby certify that I am a Registered Professional Engineer in the State of Florida and practicing with HNTB Corporation. HNTB Corporation is authorized via Certificate Number EB-0006500 to operate as an Engineering Business by the Florida State Board of Professional Engineers, State of Florida Department of Professional Regulation. I have prepared or supervised the preparation of the evaluation, findings, conclusions, recommendations, or professional opinions/advice contained in this document. My endorsement constitutes my approval of these items.

PROJECT:	District-Wide Safety Studies
	FPN: 409224-1-32-91
LOCATION:	Safety Study – Section No. 13030000
	U.S. Business 41/S.R. 45/8 <sup>th</sup> Avenue West at 23 <sup>rd</sup> Street
	West, Milepost 0.867
CLIENT:	Florida Department of Transportation
	District One

The results contained in this report were developed using procedures and references standard to the transportation engineering practice. These references and procedures were applied using professional judgment and experience.

Name: Steffanie Lynne Workman, PE

P.E. No.: 58218

Date: September 4, 2018

Signature\_



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#### **EXECUTIVE SUMMARY**

The Florida Department of Transportation (FDOT) District One Safety Office has retained HNTB Corporation as part of the district-wide traffic safety contract to perform a safety study for the intersection of U.S. 41/S.R. 45 at 23<sup>rd</sup> Street West in the City of Palmetto in Manatee County, Florida. Based on the results of the data collection, crash analysis, qualitative assessment, operational assessment and engineering judgment, the following recommendations were developed:

Recommendation: Consider reconstructing the existing full median opening at 23rd Street West

to a northbound directional median opening, including milling and

resurfacing the intersection.

Justification: Additional uncertified crash reports from January 2016 through April 2018 were

reviewed to assess the intersection safety to the present. The occurrence of angle crashes increased significantly for vehicles traveling eastbound on 23<sup>rd</sup> Street West to turn left turn onto northbound U.S. Business 41. One of the angle crashes involved a fatality in 2017. As mentioned in the *Signal Warrant Study - U.S. 41* (Business) at 23<sup>rd</sup> Street West by Traffic Engineering Data Solutions, Inc., "Should the angle crash pattern continue or the frequency of angle crashes increase, it is recommended to reconstruct the existing full median opening to a northbound

directional median opening."

Recommendation: Consider extending the length of the southbound left turn lane at 20th Street

West from 140 to 290 feet, and reduce the length of the northbound left turn

lane at 23<sup>rd</sup> Street West from 655 to 570 feet to accommodate the displaced

U-turn movement from 23<sup>rd</sup> Street West.

Justification: During both the AM peak hour and the PM peak hour, 28 right turn movements are

anticipated to be displaced as U-turn movements at 20<sup>th</sup> Street West resulting from the reconstruction of 23<sup>rd</sup> Street West. The total deceleration and storage

distances provided by the existing left turn lane at 20th Street West are 140 and 50

feet, respectively. Per FDM 212.6 (Exhibit 212-1), a total deceleration distance of

240 feet plus a minimum queue length of 50 feet (FDM 212.14.2) is required for the

50 mph design speed. Therefore, the total proposed left turn lane length is 290

feet (including 50-foot taper). In order to extend the southbound left turn lane at

20th Street West, reducing the length of the northbound left turn lane at 23rd Street

West from 655 feet to 570 feet would be required. The proposed length of the

northbound left turn lane at 23rd Street West would accommodate the required total

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deceleration length plus provide a queue length of 330 feet for storage, which exceeds the total required length.

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#### 1. INTRODUCTION

The Florida Department of Transportation (FDOT) District One Safety Office has retained HNTB Corporation as part of the district-wide traffic safety contract to perform a safety study for the intersection of U.S. Business 41/S.R. 45/8<sup>th</sup> Avenue West at 23<sup>rd</sup> Street West in the City of Palmetto in Manatee County, Florida.

#### 2. EXISTING CONDITIONS

This section of the report describes the existing physical and operational conditions of the U.S. Business 41 at 23<sup>rd</sup> Street West intersection.

U.S. Business 41 at 23<sup>rd</sup> Street West is a one-way stop-controlled intersection with a STOP sign on 23<sup>rd</sup> Street West. Within the study area, U.S. Business 41 is a north-south roadway with a 4-lane divided, suburban typical section with two 12-foot lanes in each direction that are separated by a variable width median (maximum 18 feet). The facility has 12-foot outside shoulder with 5-foot paved in each direction. U.S. Business 41 is classified as an urban principal arterial (other). 23<sup>rd</sup> Street West is an east-west roadway with a 2-lane undivided typical section with 11-foot lanes. The northbound approach to the intersection consists of two through lanes and an exclusive left-turn lane. The southbound approach to the intersection consist of two through lanes and an exclusive right-turn lane. The eastbound approach consists of an exclusive left-turn lane and exclusive right-turn lane. There are no sidewalks or designated bicycle lanes provided along either U.S. Business 41 or 23<sup>rd</sup> Street West within the study corridor. Lighting within the vicinity of the intersection is provided along both sides of U.S. Business 41 from the intersection to the north and on the east side of U.S. Business 41 south of the intersection. **Figure 1** shows an overview of the study intersection.

The nearest signalized intersection to the north is a flashing beacon at SW 26<sup>th</sup> Street West 49<sup>th</sup>, which is located approximately 0.40 miles away. The nearest signalized intersection to the south is 17<sup>th</sup> Street West/Memphis Road which is about 0.40 miles away. There are no signalized intersections to the west.

The posted speed limit along U.S. Business 41 is 50 mph north of the intersection and 45 mph south of the intersection. The posted speed limit along 23<sup>rd</sup> Street West is 35 mph with a 20 mph advisory speed approaching the intersection. Land use within the vicinity of the intersection consists of vacant/agricultural land.

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**Figure 1: Project Location Map** 

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#### 3. DATA COLLECTION

This section of the report presents the data that was obtained for this study. For the purposes of this study, it was determined that the following information would be required:

- Crash data from January 2011 through December 2015.
- Field observations of the intersection operations during an average weekday.
- Vehicle count data from count station located within the study limits.
- Eight-hour turning movement counts.
- Recently-completed or future planned projects within the study limits.

#### 3.1 Crash Data

Crash data was provided by the FDOT CARS Database and Signal 4 Analytics for the period extending from January 2011 to December 2015. Crash summaries for this time-period are presented in **Table 1.** Areas of concern are discussed in the Crash Analysis section of this report.

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# Table 1: Crash Summary U.S. Business 41/S.R. 45/8<sup>th</sup> Avenue West at 23<sup>rd</sup> Street West January 2011 – December 2015

COLLISION TYPE	2011	2012	2013	2014	2015	TOTAL
Rear End	1	0	0	0	1	2
Left Turn	1	0	0	0	0	1
Angle	1	1	1	1	2	6
Sideswipe	0	0	1	1	0	2
Right Turn	0	0	0	2	0	2
Head On	0	0	0	0	1	1
Run Off Road	0	0	1	1	0	2
Backed Into	0	0	0	0	0	0
Collision w/ Object	0	0	0	0	0	0
Pedestrian/Bicycle	0	0	0	0	0	0
Other	0	1	0	0	0	1
Total	3	2	3	5	4	17
Fatal	0	0	0	0	0	0
Injury	2	1	2	1	1	7
Property Damage Only	1	1	1	4	3	10
Day	3	2	1	4	3	13
Night	0	0	2	1	1	4
Wet	0	0	0	0	0	0
Dry	3	2	3	5	4	17
Spot Crash Rate (Crashes/MEV)	0.337	0.244	0.354	0.561	0.451	0.389
Statewide Avg Crash Rate (Crashes/MEV)	0.192	0.220	0.255	0.270	0.289	0.245

### 3.1.1 Daily Traffic Volume Data

Traffic volume data along U.S. Business 41 was obtained from the Florida Department of Transportation's Transportation Statistics Office's 2017 Historical AADT Report. Within the project limits, there was one portable traffic monitoring site identified. Site 135027 is located on U.S. Business 45, north of 21st Street West, and was reported to have an AADT of 23,000 with 3.8% truck traffic in 2017.



#### 3.1.2 Turning Movement Count Data

Eight hour turning movement counts were performed at the intersection of U.S. Business 41/U.S. 41 and 23<sup>rd</sup> Street West. Pedestrian and bicycle count data was also collected during the turning movement counts. This data was collected on Wednesday, June 6, 2018, from 7:00 AM to 10:00 AM, 11:00 AM to 1:00 PM, and 3:00 PM to 6:00 PM. The count data is summarized in **Appendix D** of this report.

#### 3.2 Recently Completed/Future Projects

Based on a review of FDOT's Work Program, two projects were identified within the study limits. The first project (FPID 195902-2) was a milling and resurfacing project along U.S. Business 41 from north of 17<sup>th</sup> Street West to North of Bayshore Road. Construction was completed and the project was accepted November 2004. The second project (FPID 436983-1) is a sidewalk construction project along 14<sup>th</sup> Avenue West, 23<sup>rd</sup> Street West and U.S. Business 41. Pre-Construction is underway and is scheduled to be complete within 30 days. There are no future planned projects identified in the FDOT Work Program for this segment.

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#### 4. QUALITATIVE ASSESSMENT

The intersection of U.S. Business 41 and 23<sup>rd</sup> Street West was observed by a registered professional engineer during the morning and afternoon peak periods of an average weekday to assess the existing operating conditions and to determine what, if any, improvements could be made to improve the safety and efficiency at this intersection.

#### 4.1 Operational Analysis

The operational analysis includes the efficiency of operations and interaction of motor vehicles, pedestrians, and bicycles on the roadway. The results of these observations are summarized below.

• U.S. Business 41 intersects 23<sup>rd</sup> Street West near the apex of a horizontal curve that becomes tangent north of 23<sup>rd</sup> Street West with a radius of approximately 1,146 feet. U.S. Business 41 has a 50 mph design speed with degree of curvature equal to 5°00'00.00" and a superelevation rate of 2.3%. This horizontal alignment creates a perception issue for motorists turning right and left onto U.S. Business 41 from 23<sup>rd</sup> Street West. The eastbound to northbound left turn movement was most affected because it was difficult for motorists to identify and judge the speed and lane assignment of on-coming northbound vehicles before executing their left turn. This movement was accomplished as a two-stage maneuver.



Figure 2: Horizontal Curve on Southbound U.S. Business 41

• 23<sup>rd</sup> Street West intersects U.S. Business 41 within a crest vertical curve.

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Figure 3: 23<sup>rd</sup> Street West, Crest Vertical Curve at Intersection Approach

- Traffic on U.S. Business 41 is low to moderate with approximately 12,700 vehicles traveling north
  and south during the eight-hour count period. Vehicles appeared to be traveling at, or slightly
  above, the posted speed limit. The speed limit transitions between 45 mph and 50 mph at the
  intersection, which adds to the complexity of the intersection.
- The intersection has a full median opening (18 feet wide, 92 feet in length from nose to nose) with STOP control on 23<sup>rd</sup> Street West.
- U.S. Business 41 has an access management designation of Class 7 (both median types), which
  requires 660 feet separation for full median openings, 330 feet separation for directional median
  openings, 1,320 feet separation for signalized intersections and 125 feet separation for driveway
  connections.
- Vehicles arriving along the southbound and eastbound intersection approaches were random, due to the absence of adjacent signalized intersections, which effected the availability of adequate gaps for traffic from 23<sup>rd</sup> Street West to enter U.S. Business 41.
- During the eight-hour count and field review, two pedestrians and one bicyclist were observed crossing the west leg of the intersection.
- Palmetto High School is located approximately 0.9 miles southwest of the intersection, on the northeast corner of 14<sup>th</sup> Avenue West and 17<sup>th</sup> Street West/Memphis Road.
- Blackstone Park is located approximately 0.4 miles west of the intersection, on the southeast corner of 23<sup>rd</sup> Street West and 14<sup>th</sup> Avenue West. Traffic volumes at this intersection are anticipated to increase during sporting events held at Blackstone Park, which typically occur on the weekends.

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- The highest approach volume from 23<sup>rd</sup> Street West was 103 vehicles from 8:30 AM to 9:30 AM for the left and right turn movements. The maximum queue was observed to be two vehicles with an hourly left turn volume of 40 vehicles. Westbound left and right turn movements were observed to have minimal delay queued during the AM peak period.
- Southbound is the peak direction with a directional split of 62% / 38%. Maximum vehicle queue observed for the northbound to westbound left turn movement was two.
- The peak hour occurs between 5:00 PM 6:00 PM. The northbound approach is the peak direction in the PM peak hour with a directional split of 54% / 46%.
- The highest turning movement volume occurred on U.S. Business 41 during the PM peak period and was the northbound to westbound left turn movement with 97 vehicles from 4:00 PM to 5:00 PM. The southbound to westbound right turn movement had 62 vehicles from 5:00 PM to 6:00 PM. The northbound to westbound left turn movement had a maximum of five vehicles queued in the median resulting in sight distance issues for the eastbound to northbound left turn movement. Sight distance was identified as an issue when motorists had to rotate their heads to identify oncoming traffic and identify a gap to complete their left turn maneuver onto U.S. Business 41.



Figure 4: U.S. Business 41, Vehicles Queued in the Median

An operational analysis was conducted using Synchro 9 to determine the operational effects of the recommendations. The intersections were analyzed with existing traffic volumes during the AM and PM peak hours with the existing geometry and with the proposed geometry. During AM peak hour conditions, the overall intersection delay for the intersection of US 41 Business/23<sup>rd</sup> Street reduced from 27.3 seconds per vehicle to 14.6 seconds per vehicle. During PM peak hour conditions, the overall intersection delay reduced from 47.7 seconds per vehicle to 13.3 seconds per vehicle. Synchro summary sheets are included in **Appendix E** of this report.

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#### 4.2 Crash Analysis

Vehicle, pedestrian, and bicycle safety at the intersection was assessed through review of crash reports, identification of significant crash trends, and correlation with field conditions. Following are the observations relating to the safety of this intersection:

- Crash data was obtained from FDOT for the period extending between January 2011 and December 2015. The collision data is summarized in Table 1.
- According to the collision data, there have been 17 collisions reported at this intersection during this time-period. The types of collisions that occurred at this intersection consisted of six (34%) angle crashes, two (12%) rear end, two (12%) sideswipes, two (12%) right turn, two (12%) run off road, one (6%) left turn, one (6%) head on collision, and one (6%) other collision. Of the total crashes, seven (41%) resulted in personal injury. Also, four (24%) of the collisions occurred at night. None of the collisions occurred during wet road conditions or involved a fatality.
- The six angle crashes involved a vehicle traveling along eastbound 23<sup>rd</sup> Street West, crossing U.S. Business 41 and turning left directly into the path of an oncoming vehicle. In addition, two right turn crashes occurred when vehicles traveling eastbound along 23<sup>rd</sup> Street West turned right onto southbound U.S. Business 41.
- The two sideswipe crashes occurred along southbound U.S. Business 41, one north and one south of the intersection. The sideswipe crashes were caused when vehicles made an improper lane change.
- Out of the two rear end crashes, one of the crashes occurred along southbound U.S. Business 41, south of 23<sup>rd</sup> Street West, and the other crash occurred along eastbound 23<sup>rd</sup> Street West.
- The two run-off the road crashes resulted when vehicles traveling along eastbound 23<sup>rd</sup> Street
  West failed to turn left onto northbound U.S. Business 41 instead continued to travel eastbound,
  hitting the curb and then a tree.
- The left turn crash involved a vehicle turning left from northbound U.S. Business 41 onto 23<sup>rd</sup> Street West, which turned into the path a vehicle traveling along southbound U.S. 41.
- A head on collision occurred when a vehicle traveling southbound along U.S. Business 41 made a wide right turn into the path of a vehicle in the left turn lane along eastbound 23<sup>rd</sup> Street West.
- The other collision involved a vehicle speeding and hitting a tree. The driver was charged with driving under the influence (DUI).
- The crash data is summarized in Appendix F of this report.

Additional uncertified crash reports from January 2016 through April 2018 were reviewed to assess the intersection safety to the present. A total of 12 crashes were reported for this 28-month period consisting of nine (76%) angle, one (8%) rear end, one (8%) sideswipe and one (8%) overturned

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vehicle crashes. The occurrence of angle crashes increased significantly for vehicles traveling eastbound on 23<sup>rd</sup> Street West to turn left turn onto northbound U.S. Business 41. One of the angle crashes involved a fatality in 2017. As mentioned in the *Signal Warrant Study - U.S. 41 (Business) at 23<sup>rd</sup> Street West* by Traffic Engineering Data Solutions, Inc., "Should the angle crash pattern continue or the frequency of angle crashes increase, it is recommended to reconstruct the existing full median opening to a northbound directional median opening." **Consider reconstructing the existing full median opening at 23<sup>rd</sup> Street West to a northbound directional median opening, including milling and resurfacing the intersection.** 

A total of 56 right turn movements (28 during both AM and PM peaks) are anticipated to be displaced from the reconstruction of 23<sup>rd</sup> Street West as additional U-turn movements at 20<sup>th</sup> Street West. The total deceleration and storage distances provided by the existing left turn lane at 20<sup>th</sup> Street West are 140 and 50 feet, respectively. Per FDM 212.6 (Exhibit 212-1), a total deceleration distance of 240 feet plus a minimum queue length of 50 feet (FDM 212.14.2) is required for the 50 mph design speed. Therefore, the total proposed left turn lane length is 455 feet (including 50-foot taper). In order to extend the southbound left turn lane at 20<sup>th</sup> Street West, reducing the length of the northbound left turn lane at 23<sup>rd</sup> Street West would be required. The proposed length of the northbound left turn lane at 23<sup>rd</sup> Street West would accommodate the required total deceleration length plus provide a queue length of 330 feet for storage, which exceeds the total proposed left turn lane length requirement. In addition, the existing roadway network provides an alternate route to access northbound U.S. Business 41 via 14<sup>th</sup> Avenue West and the signalized intersection of 17<sup>th</sup> Street West/Memphis Road.

#### 4.3 Maintenance

In addition to observing operational and safety conditions during the field review, correctible maintenance items were also identified. A summary of the maintenance items identified at this intersection is provided below.

• The pavement at the intersection is in poor condition with extensive raveling and pavement gouging present.

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Figure 5: Existing Intersection Pavement Condition

• There is one missing object marker (south of the intersection, east side) and one broken object marker (north of the intersection, west side) at the drainage end walls.



Figure 6: U.S. Business 41, Broken Object Marker at Drainage End Wall

**Appendix B** includes a condition diagram of the study area and photographs of each of the intersection approaches are included in **Appendix G**.

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#### 5. RECOMMENDATIONS

Based on the results of the data collection, crash analysis, qualitative assessment, operational assessment and engineering judgment, the following recommendations were developed:

- 1. Consider reconstructing the existing full median opening at 23<sup>rd</sup> Street West to a northbound directional median opening, including milling and resurfacing the intersection.
- Consider extending the length of the southbound left turn lane at 20<sup>th</sup> Street West from 140 to 290 feet, and reduce the length of the northbound left turn lane at 23<sup>rd</sup> Street West from 655 to 570 feet to accommodate the displaced U-turn movement from 23<sup>rd</sup> Street West.

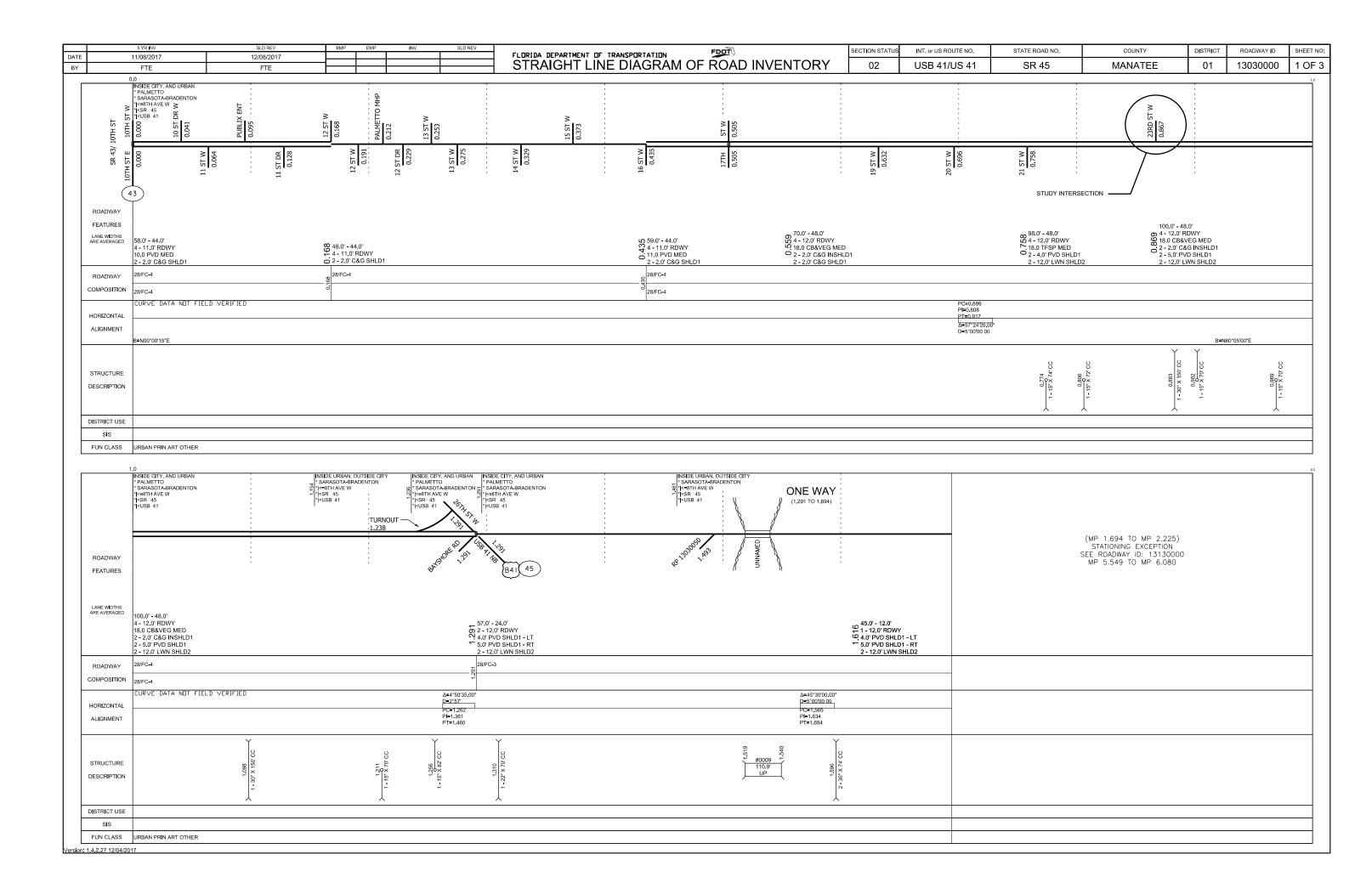
Refer to **Appendix H** for the corresponding Conceptual Design Plans.

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## **APPENDIX A: STRAIGHT LINE DIAGRAM**

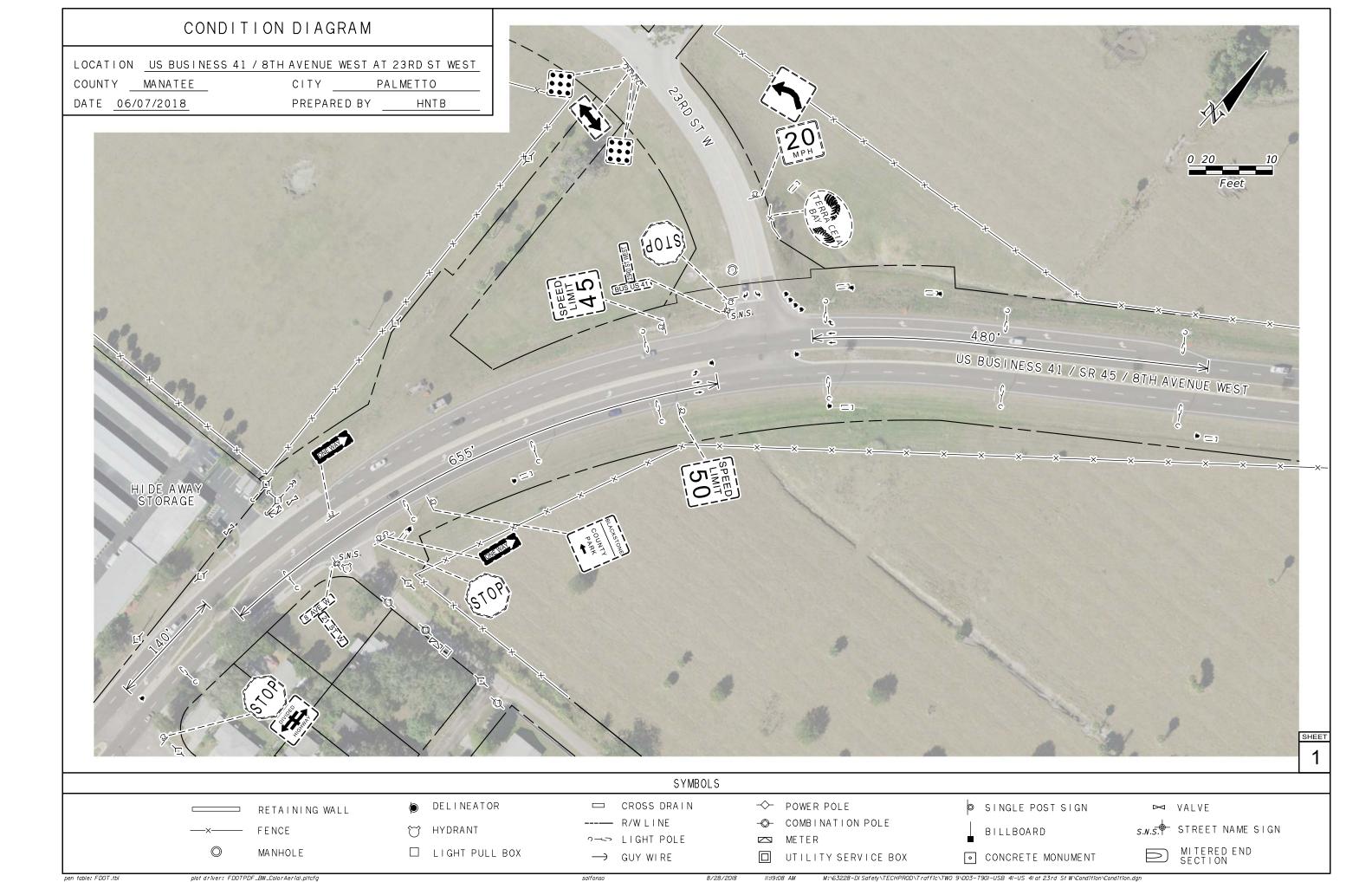
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**APPENDIX B: CONDITION DIAGRAM** 

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**APPENDIX C:** AS-BUILT PLANS

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#### COMPONENTS OF CONTRACT PLANS SET

ROADWAY PLANS SIGNING AND PAVEMENT MARKING PLANS

A DETAILED INDEX APPEARS ON THE KEY SHEET OF EACH COMPONENT

#### INDEX OF ROADWAY PLANS

HEET NO.	SHEET DESCRIPTION
ı	KEY SHEET
2	SUMMARY OF PAY ITEMS
3-4	TYPICAL SECTION
5	REFERENCE POINTS/BENCH MARKS
6	GENERAL NOTES AND SUMMARY OF QUANTITIES
7 -22	ROADWAY PLAN SHEETS
23	TRAFFIC CONTROL PLAN
24-30	INTERIM DESIGN STANDARDS
31-33	STORMWATER POLLUTION PREVENTION PLAN

#### ASPHALT COMPUTER INVOICES

# OF BOOKS	DESIGN MIX#
1	02-2202A
10	04-3459A
4	04-3055A

"This project was constructed in substantial compliance with these plans as provided by the Engineer of Record. It changes were made, those changes are indicated by black ink revision and bear the seal and signature of the Responsible Engineer."

GOVERNING STANDARDS AND SPECIFICATIONS: FLORIDA DEPARTMENT OF TRANSPORTATION, DESIGN STANDARDS DATED JANUARY 2002, AND STANDARDS SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION DATED 2004, AS AMENDED BY CONTRACT DOCUMENTS.

REVISIONS



## STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

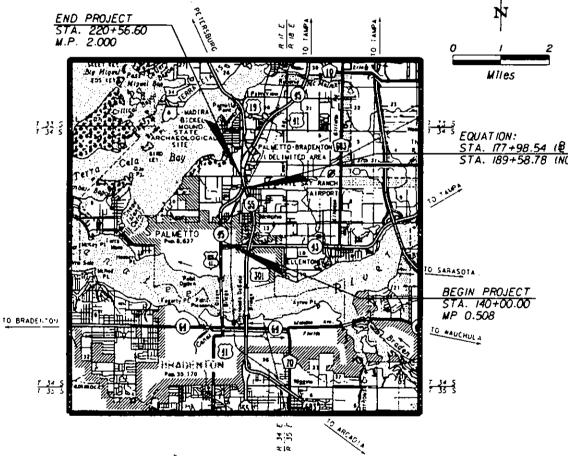
# FINAL'AS-BUILT PLANS -CONTRACT PLANS

FINANCIAL PROJECT ID 195902-2-52-01

MANATEE COUNTY (13030)

STATE ROAD 45

(BUSINESS U.S. 41)





CONTRACTOR: AJAX PAVING INDUSTRIES, INC.
DISTRICT SECRETARY: STANLEY M. CANN, P.E.
OPERATIONS ENGINEER: LANCE GRACE, P.E.
PROJECT MANAGER: JAY WINTER
PROJECT ADMINISTRATOR: SCOTT GUTHRIE
DATE WORK STARTED: 7/26/2004
DATE WORK ACCEPTED: 11/5/2004

STA. 177+98.54 (B SURVEY) BACK= STA. 189+58.78 (NORTHBOUND) AHEAD

> PLANS PREPARED BY: ENGINEER OF RECORD.

Jacobs Civil Inc.
18302 Highwoods Preserve Parkway
Highwoods Plaza, Suite 200
Tampo, FL. 33647
Tel. (813) 977 - 3434
Certificate of Authorization No. 6572

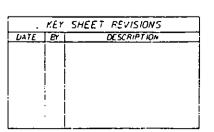
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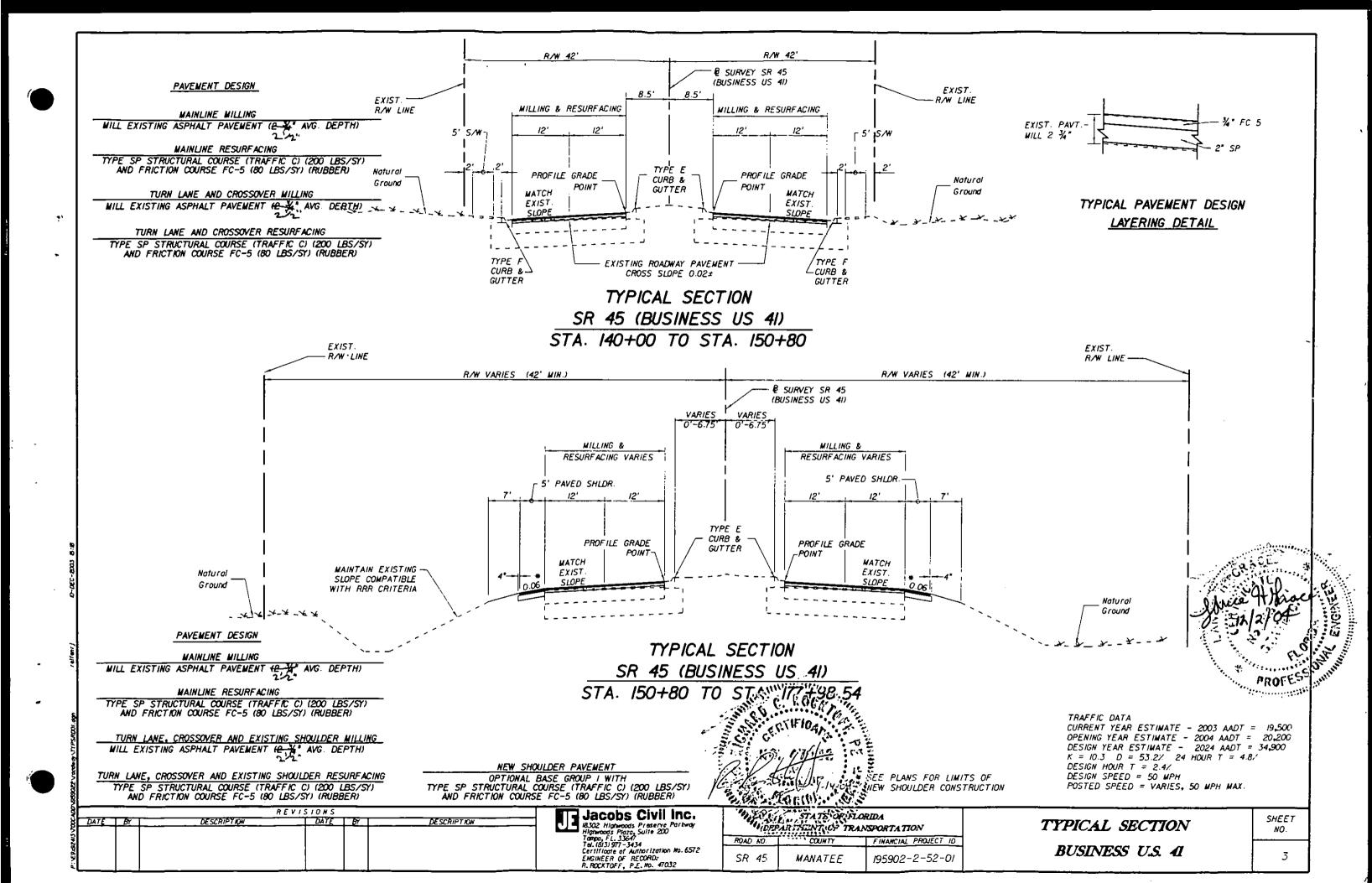
PROJECT LENGTH IS BASED ON & SURVEY NORTHBOUND SR 45.

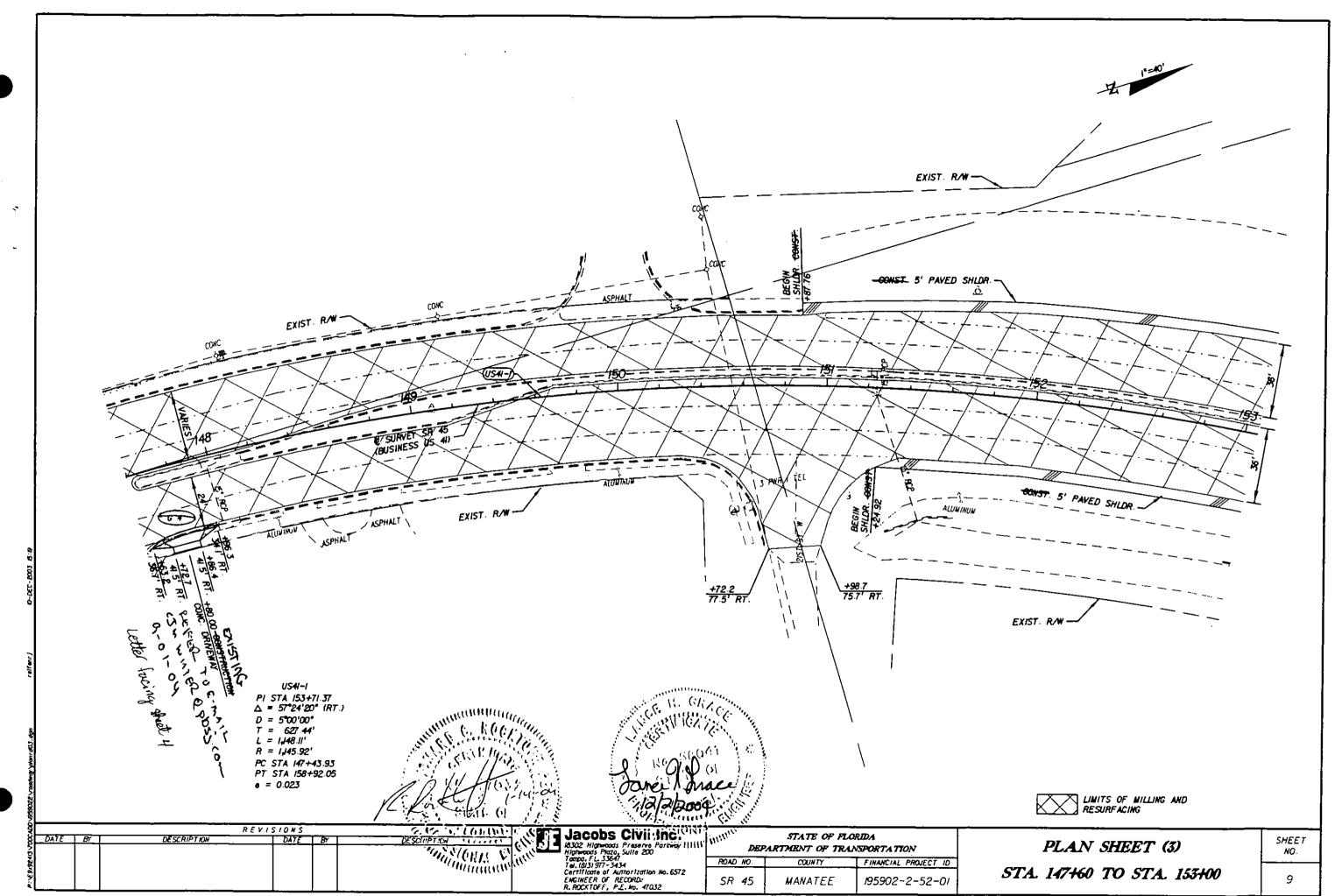
LENGTH OF PROJECT					
	LINEAR FEET	MILES			
ROADWAY	6896 . 36	1.306			
BRIDGE 5		-			
NET LENGTH OF PROJECT	6896 . 36	1.306			
EXCEPTIONS	-	-			
GROSS LENGTH OF PROJECT	6896 - 36	1.306			

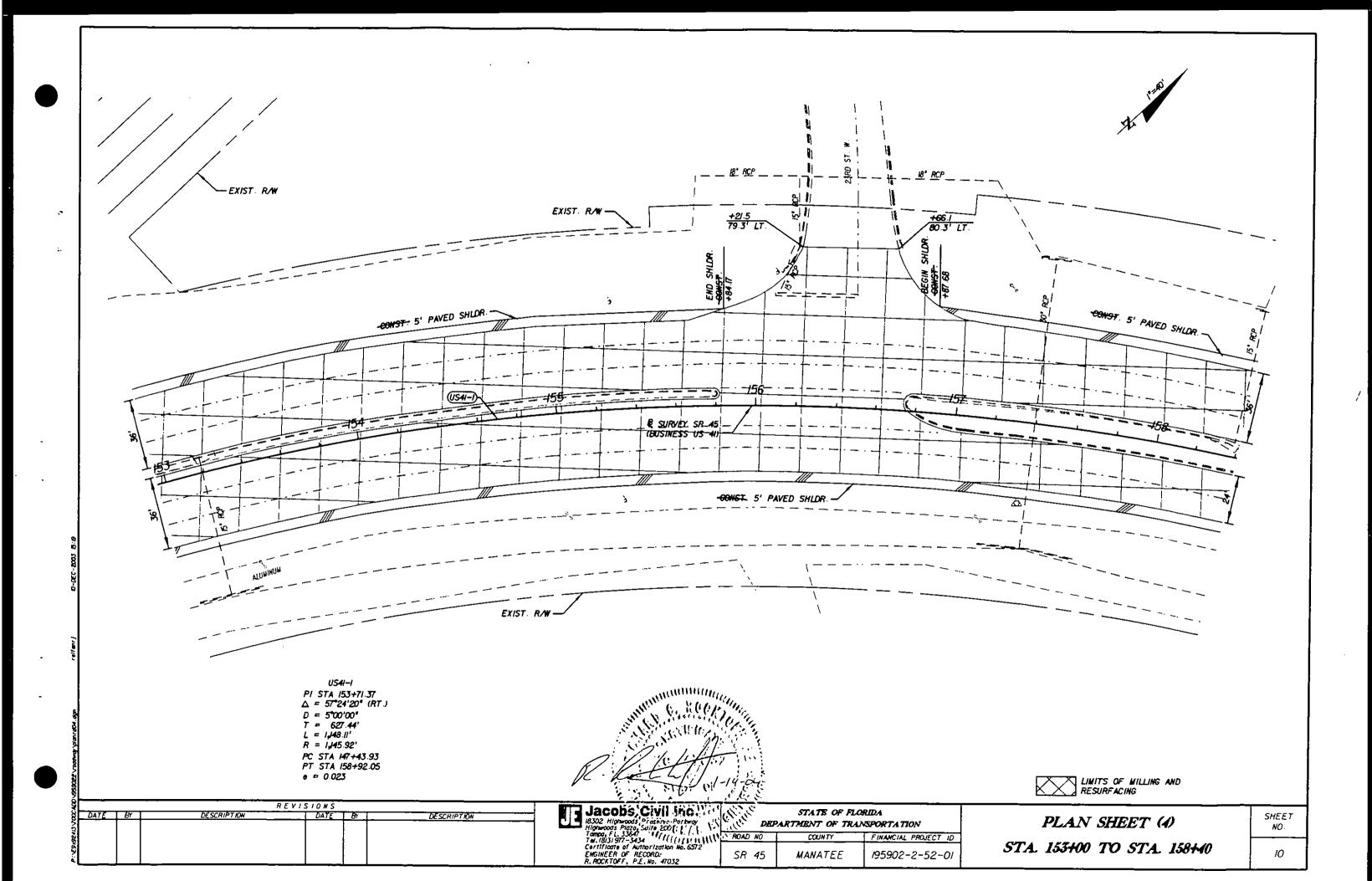
FDOT PROJECT MANAGER: JAY WINTER .



04









EXIST. R/W --CONST. 5' PAVED SHLDR.-(US4I-I) N 57"52"34" E B SURVEY SR 45 (BUSINESS US 41) - ONSE. 5' PAVED SHLDR. -EXIST. R/W -

US41-1

PI STA 153+71.37  $\Delta = 57^{\circ}24^{\circ}20^{\circ} (RT.)$   $D = 5^{\circ}00^{\circ}00^{\circ}$   $T = 627.44^{\circ}$   $L = 148.11^{\circ}$   $R = 145.92^{\circ}$ PC STA 147+43.93

PT STA 158+92.05  $\theta = 0.023$ 

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UĽ	18302 Highwoods Preserve Partway Highwoods Plaza, Sulte 200 Tampa, FL. 33647
	T W. (813) 977-3434
	Certificate of Authorization No. 657 ENGINEER_OF_RECORD:
	R. ROCKTOFF, P.E. No. 47032

DRP/	STATE OF FLA ARTMENT OF TRA	NSPORTATI	777.	111 1 10
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CD 15	WANATEE		2-52-01	<b>S</b> :

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STA.	158+40	<i>70</i>	STA.	163480

## STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

## CONTRACT PLANS

#### INDEX OF SIGNING AND PAVEMENT MARKING PLANS

SHEET NO. SHEET DESCRIPTION

S-1 KEY SHEET
S-2 GENERAL NOTES
S-3 - S-15 PLAN SHEETS

S-16 - SI7 GUIDE SIGN WORK SHEETS

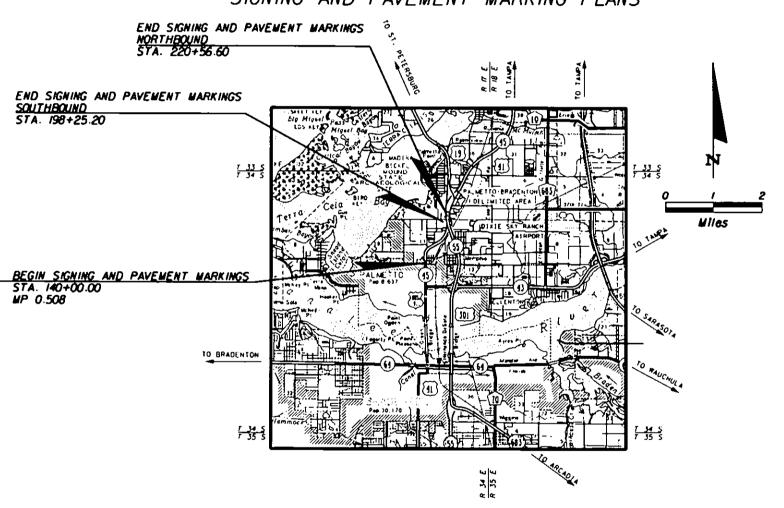
S-18 SIGNING AND MARKING CROSS SECTION SHEET

FINANCIAL PROJECT ID 195902-2-52-01

MANATEE COUNTY (13030)

U.S 41/BUSINESS 41

SIGNING AND PAVEMENT MARKING PLANS



PROJECT LOCATION

TAMPA SAFES

TAMPA SAFES

TO PERCE

TAMPA SAFES

TO LARGE NOME

BUMI

SIGNING AND PAYEMENT MARKING PLANS ENGINEER OF RECORD

ANY J. SMAFER, P.E.
FLORIDA DEPARTMENT OF TRANSPORTATION
DISTRICT | OFFICE
P.O. 80X 1249

BARTON, FL 33831-1249

GR

401 N. BROADBAY AVE.
BARTON, FL 33830-3809

NOTE: THE SCALE OF THESE PLANS WAY HAVE CHANGED DUE TO REPRODUCTION.

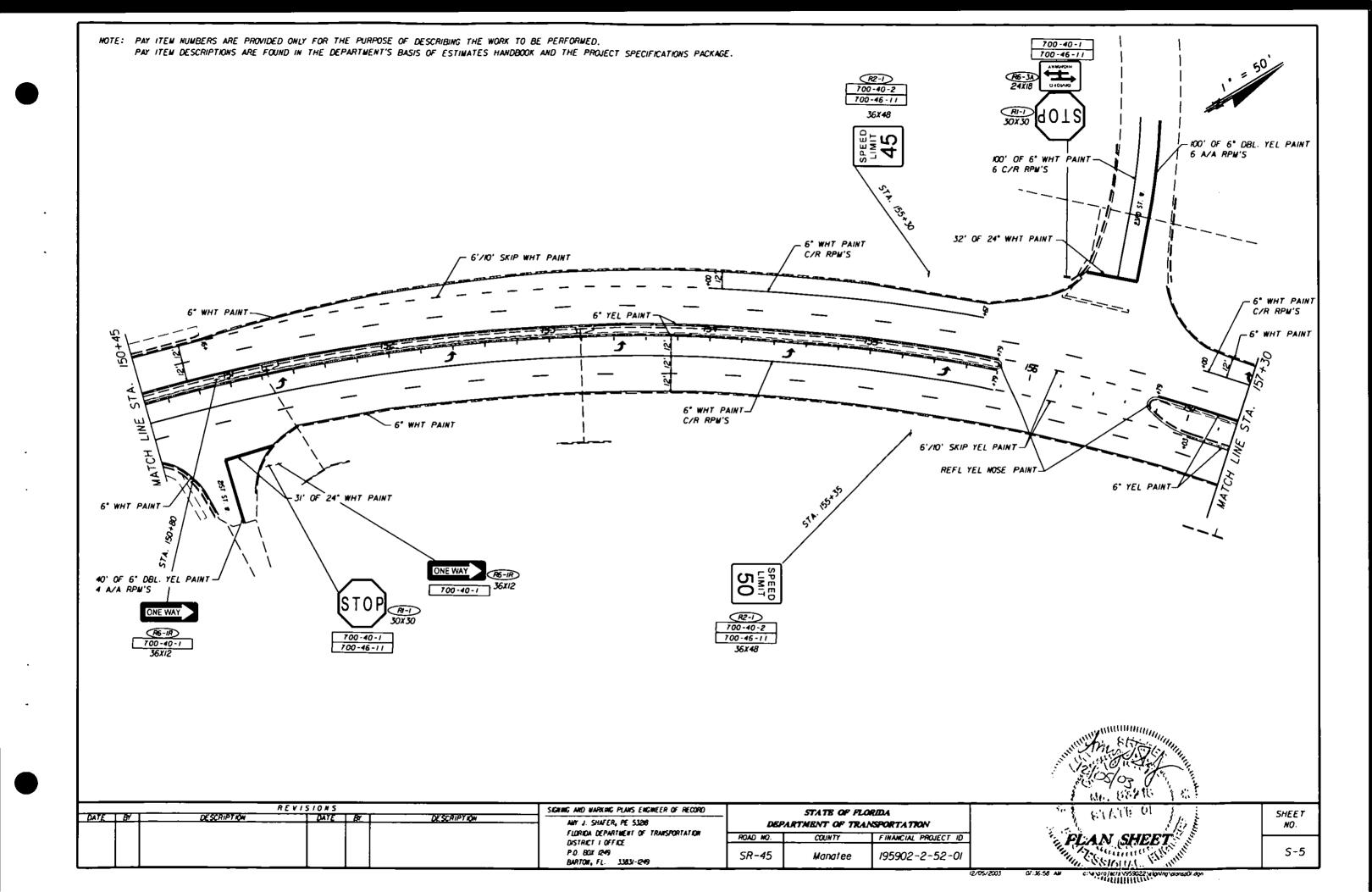
COVERNING STANDARDS AND SPECIFICATIONS: FLORIDA DEPARTMENT OF TRANSPORTATION, DESIGN STANDARDS DATED JANUARY 2004, AND STANDARDS SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION DATED 2004, AS AMENDED BY CONTRACT DOCUMENTS.

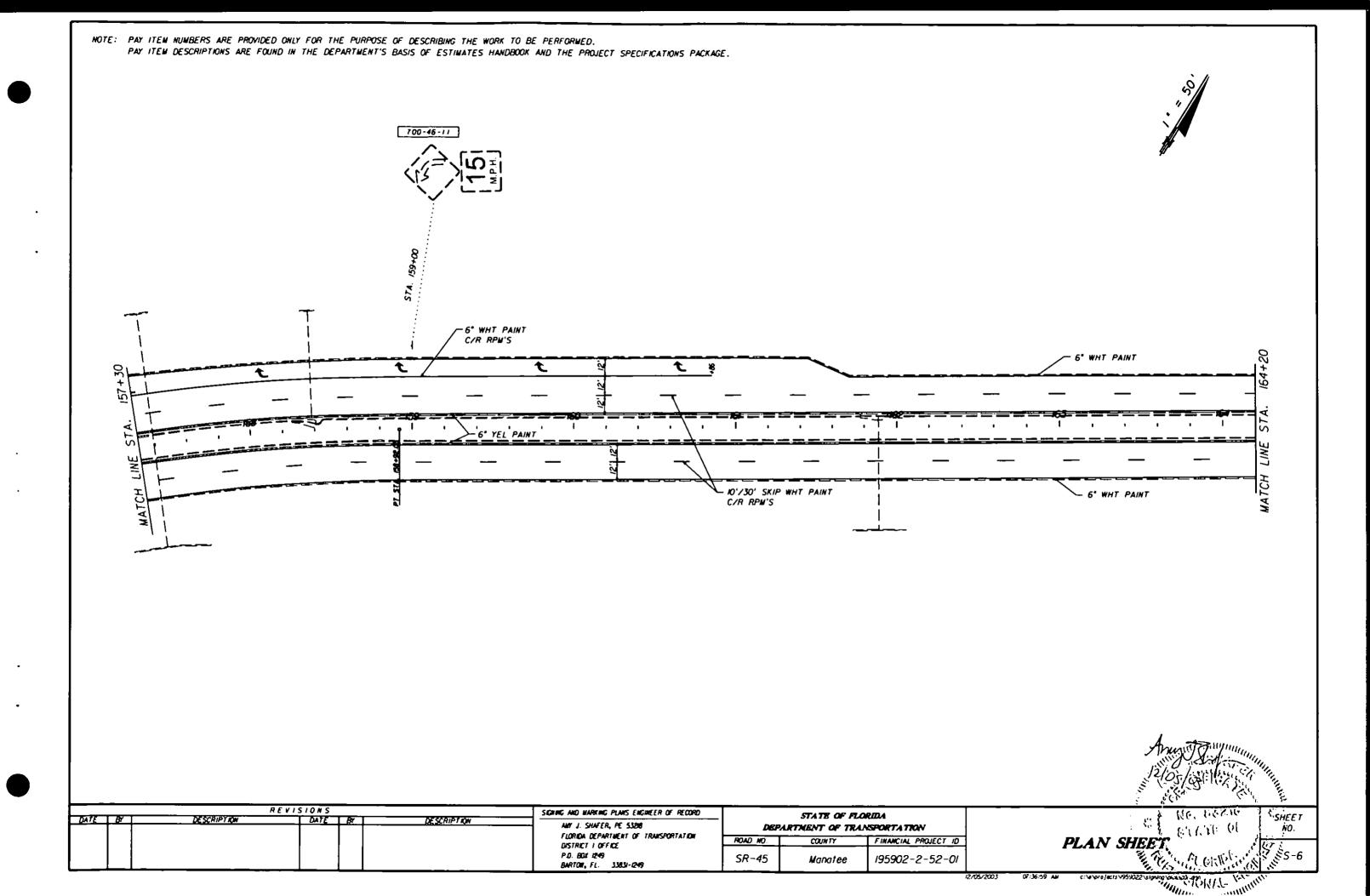
REVISIONS

KEY SHEET REVISIONS					
DATE	BY	BY DESCRIPTION			

String Total	M. C. Many
HING AND PAVENENT HARPYIC PLANS	ER (PE
12/05/03/58 PE.NOT \$ 532/8	
11:11	ISCAL SHEET
The State of	EAR NO.
The Contract of the Contract o	104 1/4 111115-1

Mannanna Committee





#### CONTRACT PLANS COMPONENTS

ROADWAY PLANS SIGNING AND PAVEMENT MARKING PLANS

## STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

### CONTRACT PLANS

## FINANCIAL PROJECT ID 436983-1-52-01 (FEDERAL FUNDS) MANATEE COUNTY (13000)

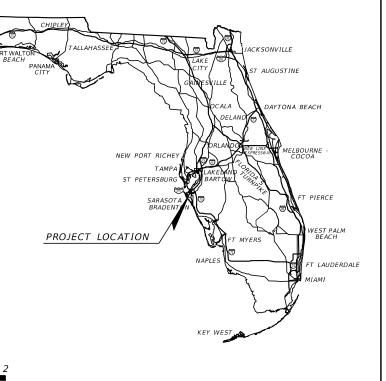
BLACKSTONE PARK 14TH AVENUE WEST, 23RD STREET WEST, AND BUSINESS 41 (VALENCIA DRIVE)

#### INDEX OF ROADWAY PLANS

111027	7107121717 7 271713
SHEET NO.	SHEET DESCRIPTION
1	KEY SHEET
2	SUMMARY OF PAY ITEMS
3 - 4	TYPICAL SECTIONS
SQ-1 TO SQ-4	SUMMARY OF QUANTITIES
5	SUMMARY OF DRAINAGE STRUCTURES
6	OPTIONAL MATERIALS TABULATION
7	PROJECT LAYOUT
8 - 10	PROJECT CONTROL
1 1	GENERAL NOTES
12-20	ROADWAY PLANS
21-22	DRAINAGE STRUCTURES
23 - 48	CROSS SECTIONS
49 - 50	STORMWATER POLLUTION PREVENTION PLAN
51	TEMPORARY TRAFFIC CONTROL PLAN
52-60	UTILITY ADJUSTMENTS
61	SELECTIVE CLEARING AND GRUBBING
62	CONTAMINATION SITE PLAN
UTV - 1 *	VERIFIED UTILITY LOCATE SHEET

<sup>\*</sup> This sheet is included in the Index of Roadway Plans only to indicate that it is part of the Roadway Plans. This sheet is contained in a separate digitally signed and sealed document.

#### TO ST. PETERSBURG TO TAMPA STATION EQUATION STA. 243+22.81 (BL SURVEY 23RD ST) T-33-. $MP. \ 0.613 =$ STA. 156+23.78 (AH.) (BL SURVEY BUS US 41) STATION EQUATION STA. 210+87.82 (BK.) MP 0.861 (BL SURVEY 14TH AVE) END PROJECT MP 1.450 =STA. 150+79.50 STA. 210+87.82 (AH.) (BL SURVEY BUS US 41) (BL SURVEY 23RD ST) MP 0.758 MP 0.000 BEGIN PROJECT ΤO STA. 200+73.67 SARASOTA (BL SURVEY 14TH AVE) MP 1.258 ΤO SNEAD ISLAND Florida Department of Transportation, FY 2017-18 Design Standards eBook (DSeB) and applicable Design Standards Revisions (DSRs) at the following website:



## No 65623 STATE OF SIONAL ENV

Miles

THIS DOCUMENT HAS BEEN DIGITALLY SIGNED AND SEALED BY:

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED. THE SIGNATURE MUST BE VERFIED ON THE ELECTRONIC DOCUMENTS.

#### ROADWAY PLANS ENGINEER OF RECORD:

KEITH A. JOHNSON, P.E. P.E. NO.: 65623 PEGASUS ENGINEERING, LLC 301 WEST STATE ROAD 434 SUITE 309 WINTER SPRINGS, FL 32708 (407) 992-9160 CONSULTANT CONTRACT NO.: C9C26 VENDOR NO.: F260806410-001 CERTIFICATE OF AUTHORIZATION NO. 27770

#### FDOT PROJECT MANAGER: ANTONE SHERRARD

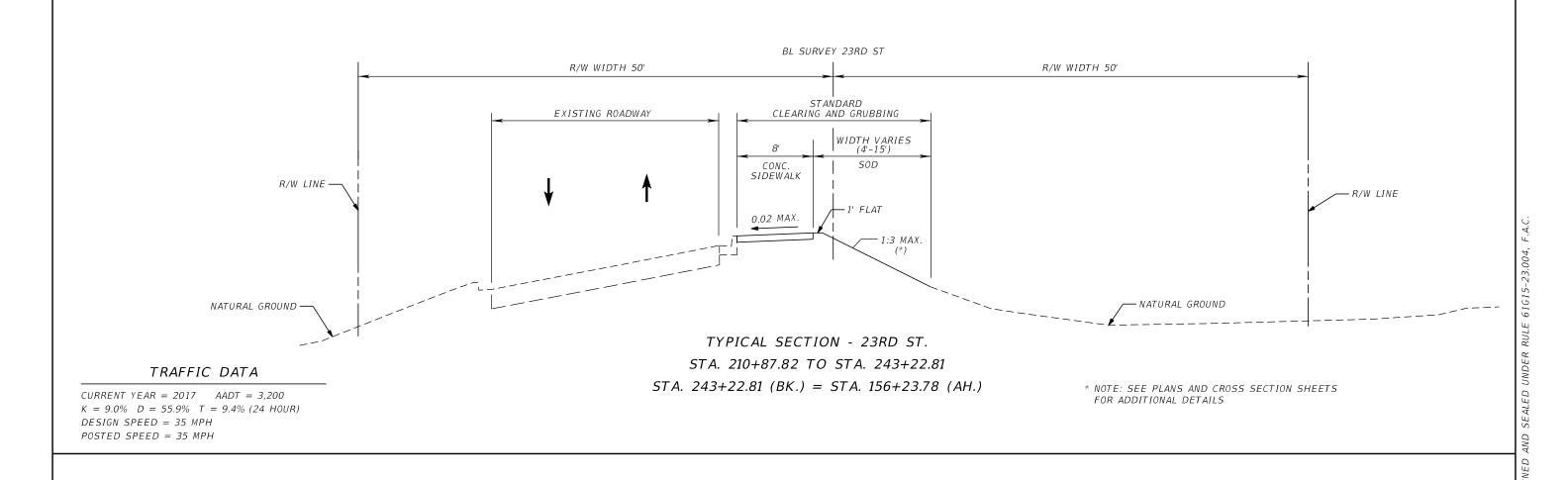
CONSTRUCTION	FISCAL	SHEET
CONTRACT NO.	YEAR	NO.
T1699	18	1

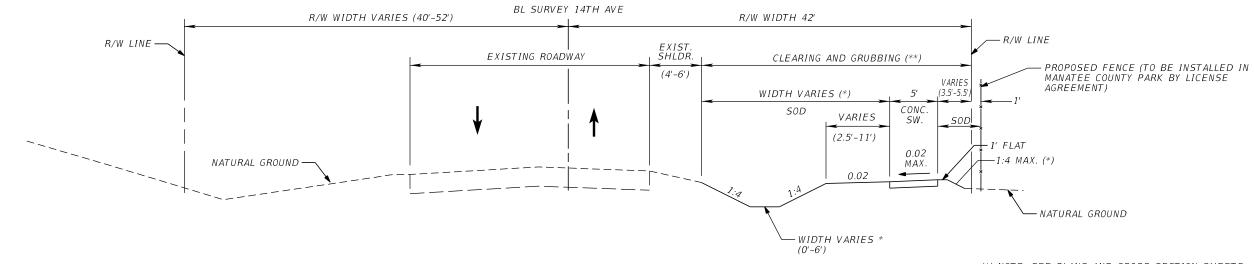
#### GOVERNING STANDARD SPECIFICATIONS:

http://www.fdot.gov/roadway/designstandards/standards.shtm

GOVERNING DESIGN STANDARDS:

Florida Department of Transportation, January 2018, Standard Specifications for Road and Bridge Construction at the following website: http://www.fdot.gov/programmanagement/Implemented/SpecBooks





#### TRAFFIC DATA

CURRENT YEAR = 2017 AADT = 2,050K = 9.0% D = 55.9% T = 1.9% (24 HOUR) DESIGN SPEED = 35 MPH POSTED SPEED = 35 MPH

TYPICAL SECTION - 14TH AVE. STA. 200+73.67 TO STA. 210+87.82 (\*) NOTE: SEE PLANS AND CROSS SECTION SHEETS FOR ADDITIONAL DETAILS

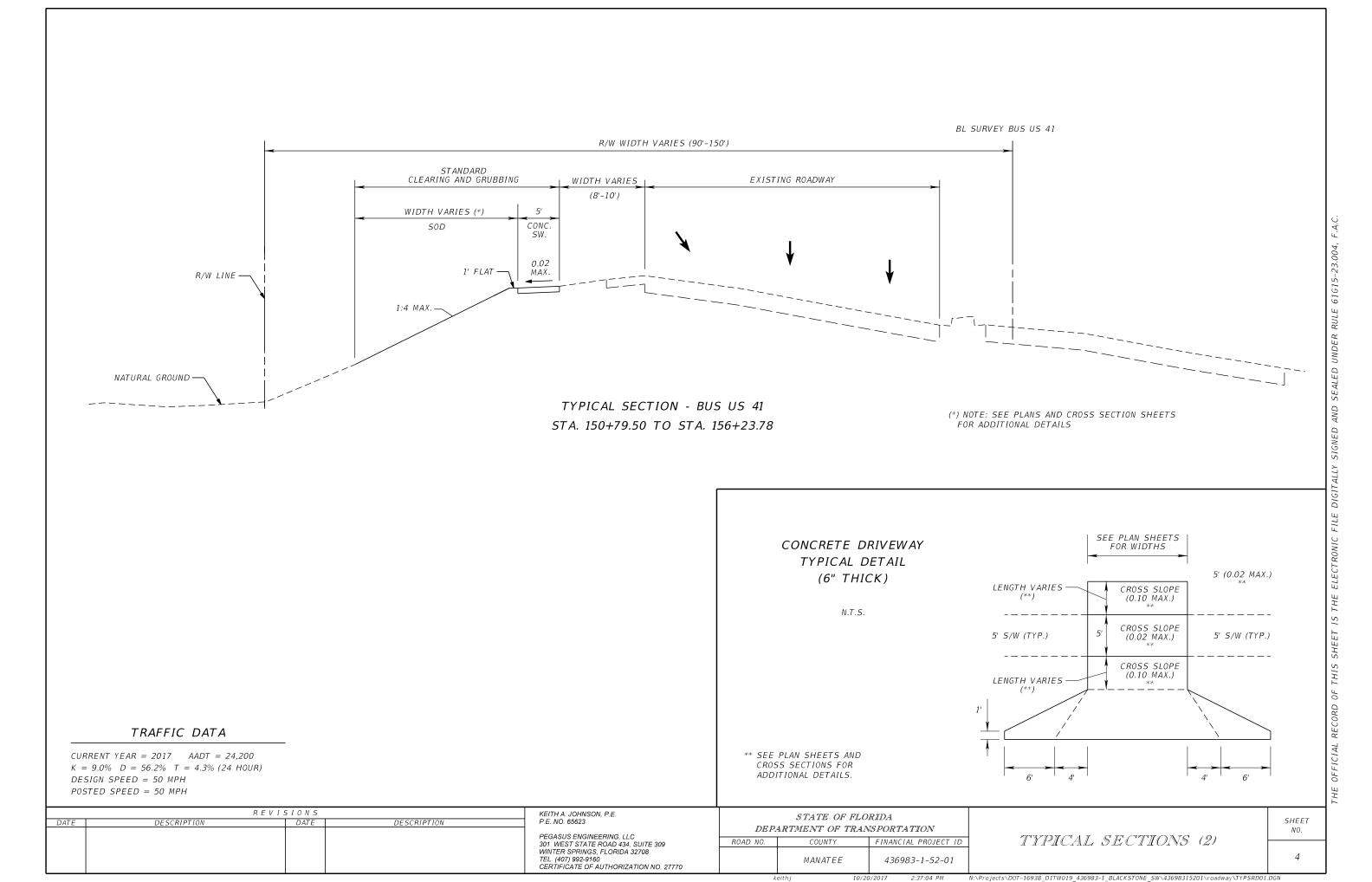
(\*\*) STANDARD CLEARING AND GRUBBING FROM STA. 200+73.67 TO STA. 202+10.00 AND STA. 207+80.00 TO STA. 210+87.82

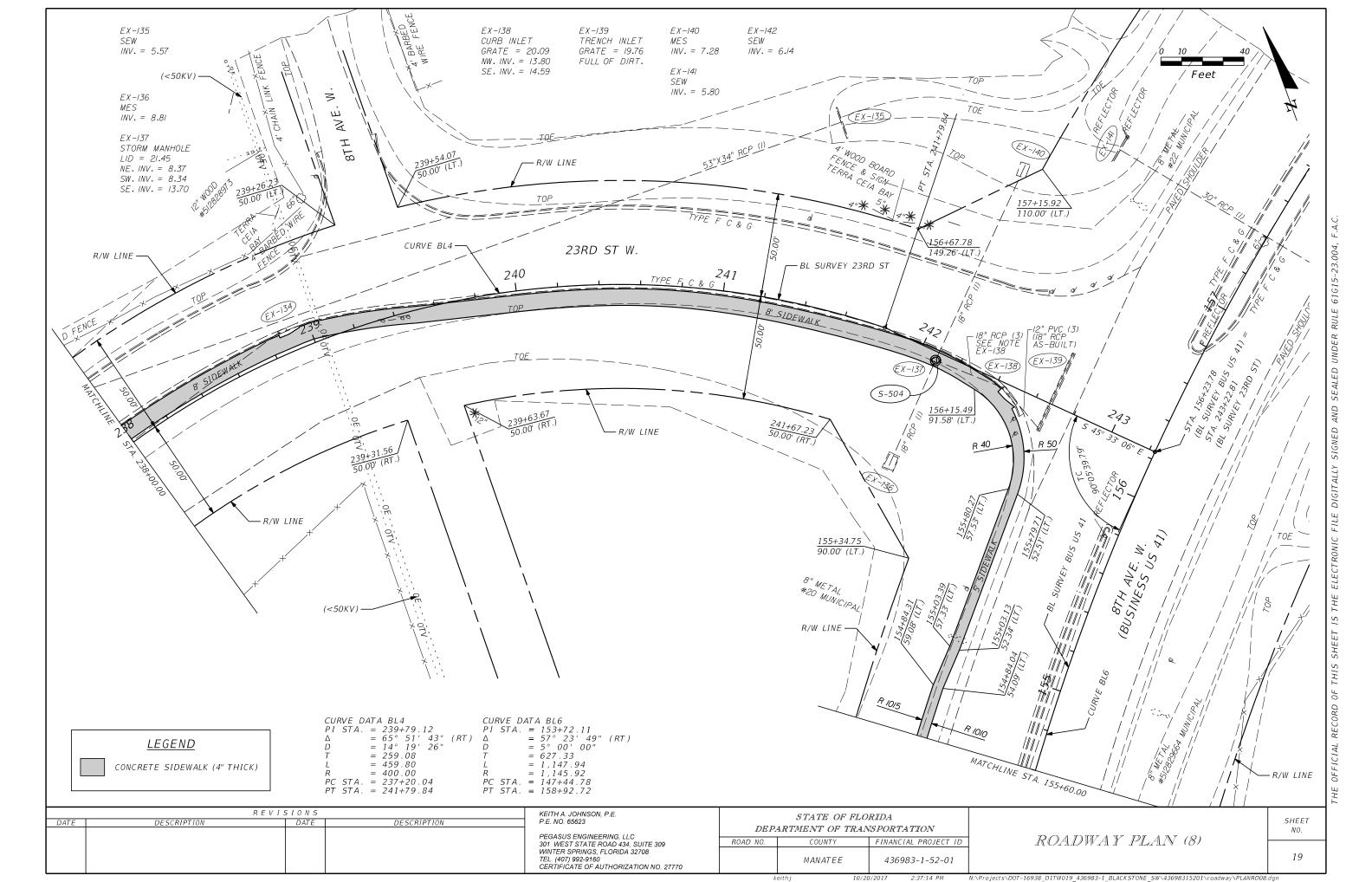
SELECTIVE CLEARING AND GRUBBING FROM STA. 202+10.00 TO STA. 207+80.00

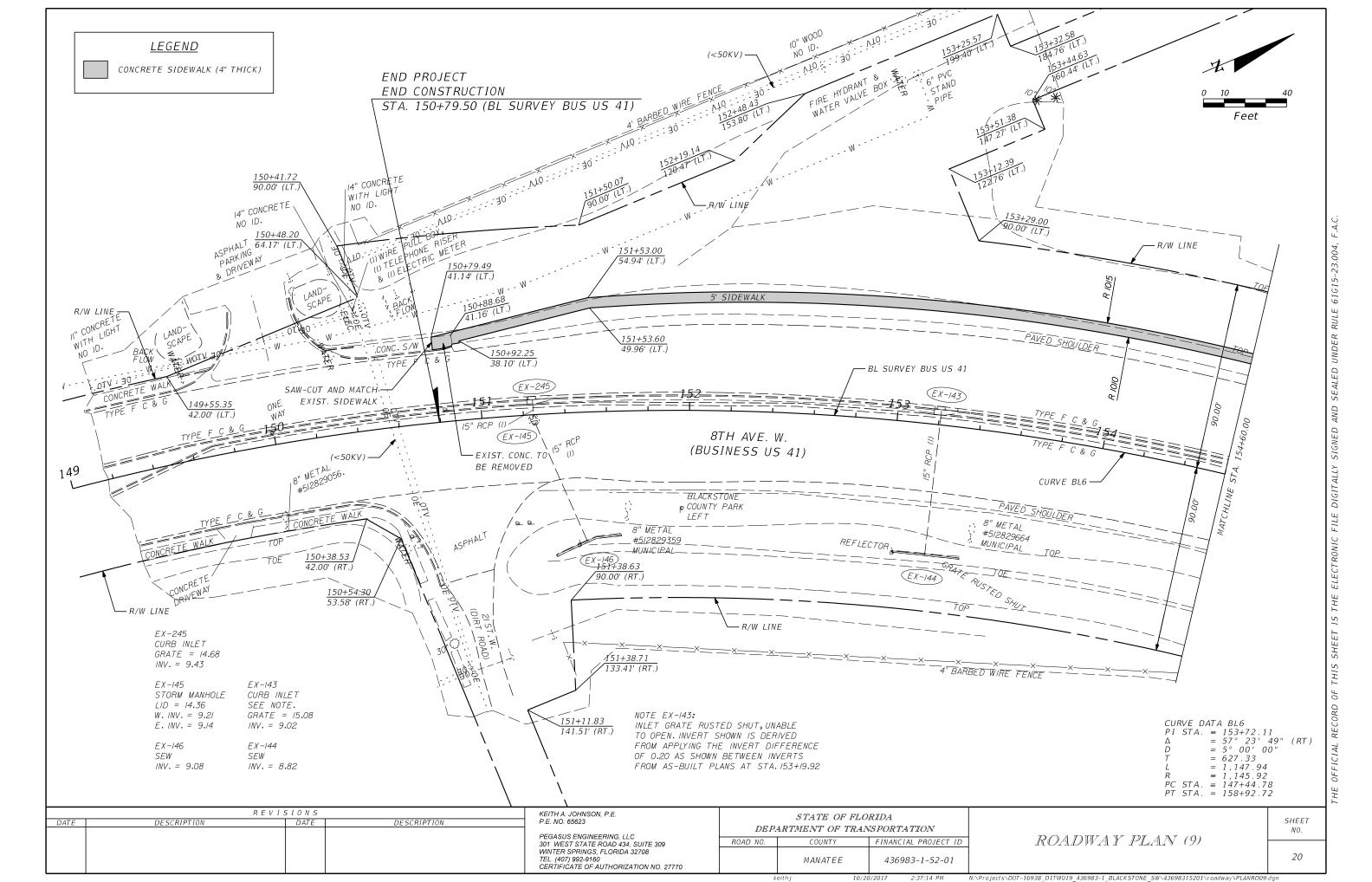
REVISIONS			KEITH A. JOHNSON, P.E.	STATE OF FLORIDA				
DATE	DESCRIPTION	DATE	DESCRIPTION	P.E. NO. 65623	DED			
				PEGASUS ENGINEERING, LLC	DEPARTMENT OF TRANSPORTATION			
				301 WEST STATE ROAD 434, SUITE 309	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	1
				WINTER SPRINGS, FLORIDA 32708 TEL. (407) 992-9160		MANATEE	436983-1-52-01	

TYPICAL SECTIONS (1)

SHEET NO. 3









# **APPENDIX D:** TURNING MOVEMENT COUNT DATA

HNTB Corporation September 2018

Street Name		SB (USE	3 41	)		WB	( )			NB (USI	B 41 )			EB (23r	d ST W)	
Start Time	Left	Thru	Right	<b>U</b> Turn	Left	Thru	Right	U Turn	Left	Thru	Right	<b>U</b> Turn	Left	Thru	Right	U Turn
7:00	0	186	4	0	0	0	0	0	4	141	0	1	15	0	11	0
7:15	0	213	6	0	0	0	0	0	2	125	0	0	8	0	10	0
7:30	0	248	13	0	0	0	0	0	6	154	0	0	7	0	11	0
7:45	0	277	12	0	0	0	0	0	6	135	0	2	4	0	14	0
8:00	0	245	23	0	0	0	0	0	7	128	0	0	12	0	15	0
8:15	0	244	16	0	0	0	0	0	11	123	0	1	5	0	12	0
8:30	0	219	19	0	0	0	0	0	6	118	0	1	12	0	17	0
8:45	0	248	11	0	0	0	0	0	15	129	0	0	7	0	12	0
9:00	0	216	11	0	0	0	0	0	5	97	0	1	12	0	20	0
9:15	0	161	8	0	0	0	0	0	8	106	0	0	7	0	12	0
9:30	0		8	0	0	0	0	0	14	114	0	1	7	0	14	0
9:45	0	187	6	0	0	0	0	0	11	111	0	1	7	0	16	0
11:00	0	175	8	0	0	0	0	0	11	112	0	1	2	0	10	0
11:15	0	196	9	0	0	0	0	0	24	160	0	1	9	0	17	0
11:30	0	168	12	0	0	0	0	0	18	111	0	2	6	0	14	0
11:45	0	148	10	0	0	0	0	0	17	143	0	1	25	0	6	0
12:00	0	165	6	0	0	0	0	0	14	113	0	0	4	0	8	0
12:15	0		4	0	0	0	0	0	24	163	0	0	12	0	9	0
12:30	0	151	9	0	0	0	0	0	16	141	0	0	1	0	14	0
12:45	0	132	1	0	0	0	0	0	16	131	0	0	8	0	22	0
15:00	0	161	6	1	0	0	0	0	28	166	0	1	1	0	14	0
15:15	0		9	1	0	0	0	0	18	153	0	1	10	0	11	0
15:30	0	153	10	0	0	0	0	0	17	200	0	0	13	0	14	0
15:45	0	181	9	0	0	0	0	0	10	197	0	0	9	0	10	0
16:00	0	157	12	0	0	0	0	0	26	208	0	2	3	0	13	0
16:15	0	162	7	0	0	0	0	0	27	201	0	0	4	0	2	0
16:30	0	200	7	0	0	0	0	0	22	270	0	1	15	0	9	0
16:45	0	_	18	0	0	0	0	0	21	272	0	_	8	0	13	0
17:00	0	218	10	0	0	0	0	0	26	257	0	3	4	0	17	0
17:15	0	_	11	0	0	0	0	0	21	270	0	1	9	0	14	0
17:30	0	227	20	0	0	0	0	0	17	272	0	2	8	0	8	0
17:45	0	233	21	0	0	0	0	0	30	271	0	1	6	0	13	0

### Trucks

Street Name		SB (USE	3 41	)		WB	( )			NB (USI	B 41			EB (23r	d ST W)	
Start Time	Left	Thru	Right	U Turn	Left	Thru	Right	<b>U</b> Turn	Left	Thru	Right	<b>U</b> Turn	Left	Thru	Right	<b>U</b> Turn
7:00	0	4	0	0	0	0	0	0	0	1	0	0	1	0	0	0
7:15	0	7	0	0	0	0	0	0	0	8	0	0	0	0	0	0
7:30	0	9	1	0	0	0	0	0	0	2	0	0	0	0	0	0
7:45	0	12	0	0	0	0	0	0	0	4	0	0	0	0	1	0
8:00	0	6	0	0	0	0	0	0	1	6	0	0	0	0	0	0
8:15	0	10	0	0	0	0	0	0	0	2	0	0	0	0		0
8:30	0	10	0	0	0	0	0	0	0	3	0	0	1	0		0
8:45	0	4	1	0	0	0	0	0	2	0	0	0	0	0		0
9:00	0	3	0	0	0	0	0	0	0	1	0		1	0		0
9:15	0	10	1	0	0	0	0	0	0	4	0	0	0	0	0	0
9:30	0	8	0	0	0	0	0	0	1	2	0	0	0	0	1	0
9:45	0	8	0	0	0	0	0	0	0	2	0	0	0	0	0	0
11:00	0	5	1	0	0	0	0	0	0	3	0	_	0	0	-	0
11:15	0	4	0	0	0	0	0	0	0	5	0	0	0	0	0	0
11:30	0	8	0	0	0	0	0	0	0	3	0	_	0	0	-	0
11:45	0	4	0	0	0	0	0	0	0	4	0		0	0		0
12:00	0	0	0	0	0	0	0	0	0	12	0	0	1	0	2	0
12:15	0	5	0	0	0	0	0	0	1	2	0	0	0	0	0	0
12:30	0	7	1	0	0	0	0	0	0	1	0	_	0	0		0
12:45	0	4	1	0	0	0	0	0	1	5	0	0	0	0		0
15:00	0	4	0	0	0	0	0	0	0	1	0	0	0	0		0
15:15	0	4	1	0	0	0	0	0	0	4	0	0	0	0		0
15:30	0	4	2	0	0	0	0	0	1	4	0	0	2	0	1	0
15:45	0	0	2	0	0	0	0	0	0	4	0	0	1	0	0	0
16:00	0	4	0	0	0	0	0	0	1	7	0	1	1	0	-	0
16:15	0	3	0	0	0	0	0	0	0	6	0	· ·	0	0	•	0
16:30	0	6	0	0	0	0	0	0	0	3	0	0	0	0	0	0
16:45	0	5	0	0	0	0	0	0	0	1	0	0	0	0	0	0
17:00	0	2		0	0	0	0	0	1	6	0	_	0	0	_	0
17:15	0	6	0	0	0	0	0	0	0	0	0	0	0	0		0
17:30	0	4	0	0	0	0	0	0	1	1	0	0	1	0	0	0
17:45	0	0	0	0	0	0	0	0	0	5	0	0	0	0	0	0

TURNING MOVEMENT COUNT: USB 41 @ 23rd St W EAST/WEST ST: 23rd St W

TIME: 7am-6pm NORTH/SOUTH ST: USB 41 COUNTED BY: Video Cam2

### ALL VEHICLES

START		N	ORTHBOUND	)			s	OUTHBOUND						EASTBOUND				1	WESTBOUND			GRAND
TIME	LEFT	THRU	RIGHT	U-TURN	TOTAL	LEFT	THRU	RIGHT	U-TURN	TOTAL	NS TOTAL	LEFT	THRU	RIGHT	U-TURN	TOTAL	LEFT	THRU	RIGHT U-TURN	TOTAL	EW TOTAL	TOTAL
7:00	4	142	0	1	147	0	190	4	0	194	341	16	0	11	0	27	0	0	0 0	0	27	368
7:15	2	133	0	0	135	0	220	6	0	226	361	8	0	10	0	18	0	0	0 0	0	18	379
7:30	6	156	0	0	162	0	257	14	0	271	433	7	0	11	0	18	0	0	0 0	0	18	451
7:45	6	139	0	2	147	0	289	12	0	301	448	4	0	15	0	19	0	0	0 0	0	19	467
Total	18	570	0	3	591	0	956	36	0	992	1,583	35	0	47	0	82	0	0	0 0	0	82	1,665
8:00	8	134	0	0	142	0	251	23	0	274	416	12	0	15	0	27	0	0	0 0	0	27	443
8:15	11	125	0	1	137	0	254	16	0	270	407	5	0	12	0	17	0	0	0 0	0	17	424
8:30	6	121	0	1	128	0	229	19	0	248	376	13	0	17	0	30	0	0	0 0	0	30	406
8:45	17	129	0	0	146	0	252	12	0	264	410	7	0	13	0	20	0	0	0 0	0	20	430
Total	42	509	0	2	553	0	986	70	0	1,056	1,609	37	0	57	0	94	0	0	0 0	0	94	1,703
9:00	5	98	0	1	104	0	219	11	0	230	334	13	0	21	0	34	0	0	0 0	0	34	368
9:15	8	110	0	0	118	0	171	9	0	180	298	7	0	12	0	19	0	0	0 0	0	19	317
9:30	15	116	0	1	132	0	187	8	0	195	327	7	0	15	0	22	0	0	0 0	0	22	349
9:45	11	113	0	1	125	0	195	6	0	201	326	7	0	16	0	23	0	0	0 0	0	23	349
Total	39	437	0	3	479	0	772	34	0	806	1,285	34	0	64	0	98	0	0	0 0	0	98	1,383
11:00	11	115	0	1	127	0	180	9	0	189	316	2	0	10	0	12	0	0	0 0	0	12	328
11:15	24	165	0	1	190	0	200	9	0	209	399	9	0	17	0	26	0	0	0 0	0	26	425
11:30	18	114	0	2	134	0	176	12	0	188	322	6	0	14	0	20	0	0	0 0	0	20	342
11:45	17	147	0	1	165	0	152	10	0	162	327	25	0	8	0	33	0	0	0 0	0	33	360
Total	70	541	0	5	616	0	708	40	0	748	1,364	42	0	49	0	91	0	0	0 0	0	91	1,455
12:00	14	125	0	0	139	0	165	6	0	171	310	5	0	10	0	15	0	0	0 0	0	15	325
12:15	25	165	0	0	190	0	178	4	0	182	372	12	0	9	0	21	0	0	0 0	0	21	393
12:30	16	142	0	0	158	0	158	10	0	168	326	1	0	14	0	15	0	0	0 0	0	15	341
12:45	17	136	0	0	153	0	136	2	0	138	291	8	0	23	0	31	0	0	0 0	0	31	322
Total	72	568	0	0	640	0	637	22	0	659	1,299	26	0	56	0	82	0	0	0 0	0	82	1,381
15:00	28	167	0	1	196	0	165	6	1	172	368	1	0	14	0	15	0	0	0 0	0	15	383
15:15	18	157	0	1	176	0	157	10	1	168	344	10	0	11	0	21	0	0	0 0	0	21	365
15:30	18	204	0	0	222	0	157	12	0	169	391	15	0	15	0	30	0	0	0 0	0	30	421
15:45	10	201	0	0	211	0	181	11	0	192	403	10	0	10	0	20	0	0	0 0	0	20	423
Total	74	729	0	2	805	0	660	39	2	701	1,506	36	0	50	0	86	0	0	0 0	0	86	1,592
16:00	27	215	0	3	245	0	161	12	0	173	418	4	0	13	0	17	0	0	0 0	0	17	435
16:15	27	207	0	0	234	0	165	7	0	172	406	4	0	2	0	6	0	0	0 0	0	6	412
16:30	22	273	0	1	296	0	206	7	0	213	509	15	0	9	0	24	0	0	0 0	0	24	533
16:45	21	273	0	0	294	0	220	18	0	238	532	8	0	13	0	21	0	0	0 0	0	21	553
Total	97	968	0	4	1,069	0	752	44	0	796	1,865	31	0	37	0	68	0	0	0 0	0	68	1,933
17:00	27	263	0	3	293	0	220	10	0	230	523	4	0	17	0	21	0	0	0 0	0	21	544
17:15	21	270	0	1	292	0	229	11	0	240	532	9	0	14	0	23	0	0	0 0	0	23	555
17:30	18	273	0	2	293	0	231	20	0	251	544	9	0	8	0	17	0	0	0 0	0	17	561
17:45	30	276	0	1	307	0	233	21	0	254	561	6	0	13	0	19	0	0	0 0	0	19	580
Total	96	1,082	0	7	1,185	0	913	62	0	975	2,160	28	0	52	0	80	0	0	0 0	0	80	2,240

### FLORIDA DEPARTMENT OF TRANSPORTATION

### **SUMMARY OF VEHICLE MOVEMENTS**

SECTION: N/A CITY: Palmetto COUNTY: Manatee

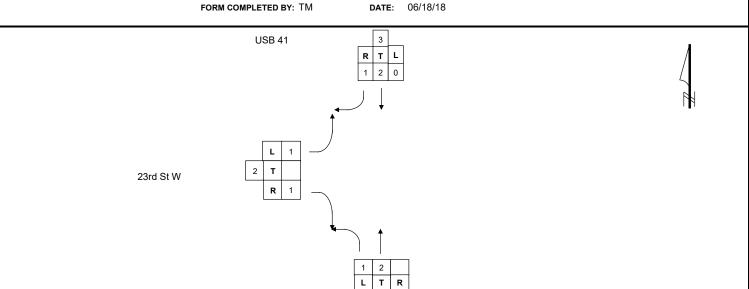
INTR ROUTE: 23rd St W STATE ROUTE: USB 41

 OBSERVER:
 Video Cam2
 DATE:
 06/06/18
 MILEPOST:

 WEATHER:
 Sunny - Clear
 ROAD CONDITION: Dry - Good

REMARKS:

USB 41



TIME		NO	RTHBO	UND			so	итнвоі	JND		TOTAL		E/	ASTBOL	JND			WE	STBOU	ND		TOTAL
BEGIN/END	L	Т	R	O	тот	L	Т	R	Ω	тот	N/S	L	Т	R	U	тот	L	Т	R	U	тот	E/W
7 - 8	18	570	0	3	591	0	956	36	0	992	1,583	35	0	47	0	82	0	0	0	0	0	82
8 - 9	42	509	0	2	553	0	986	70	0	1,056	1,609	37	0	57	0	94	0	0	0	0	0	94
9 - 10	39	437	0	3	479	0	772	34	0	806	1,285	34	0	64	0	98	0	0	0	0	0	98
11 - 12	70	541	0	5	616	0	708	40	0	748	1,364	42	0	49	0	91	0	0	0	0	0	91
12 - 1	72	568	0	0	640	0	637	22	0	659	1,299	26	0	56	0	82	0	0	0	0	0	82
3 - 4	74	729	0	2	805	0	660	39	2	701	1,506	36	0	50	0	86	0	0	0	0	0	86
4 - 5	97	968	0	4	1,069	0	752	44	0	796	1,865	31	0	37	0	68	0	0	0	0	0	68
5 - 6	96	1,082	0	7	1,185	0	913	62	0	975	2,160	28	0	52	0	80	0	0	0	0	0	80
TOTAL	508	5,404	0	26	5,938	0	6,384	347	2	6,733	12,671	269	0	412	0	681	0	0	0	0		681

#### FLORIDA DEPARTMENT OF TRANSPORTATION PEDESTRIAN MOVEMENT SUMMARY SECTION: CITY: Palmetto **COUNTY:** Manatee N/A STATE ROUTE: USB 41 INTER. ROUTE: 23rd St W OBSERVER: Video Cam2 **DATE:** 06/06/18 REMARKS: FORM COMPLETED BY: TM **DATE**: 06/18/18 **USB 41** 11-12 7-8 8-9 9-1 12-1 3-4 4-5 5-6 Total 7-8 7-8 8-9 8-9 9-10 9-10 11-12 11-12 12-1 12-1 23rd St W 3-4 3-4 4-5 4-5 5-6 5-6 Total Total 7-8 8-9 9-1 11-12 12-1 3-4 4-5 5-6 Total

**USB 41** 



**APPENDIX E:** SYNCHRO SUMMARY SHEETS

HNTB Corporation September 2018

Intersection						
Int Delay, s/veh	1.4					
Movement	SEL	SER	NEL	NET	SWT	SWR
Lane Configurations	) j	JLIK	NLL 7	<b>^</b>	<b>↑</b> ↑	7
Traffic Vol, veh/h	28	53	31	554	1051	65
Future Vol, veh/h	28	53	31	554	1051	65
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	340	675	-	_	475
Veh in Median Storage,	-	-	-	0	0	473
Grade, %	0			0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	30	58	34	602	1142	71
Major/Minor N	/linor2	N	Major1	N	Major2	
Conflicting Flow All	1510	571	1142	0	-	0
Stage 1	1142	_	_	_	_	_
Stage 2	368	_	_	_	-	_
Critical Hdwy	6.84	6.94	4.14	_	-	-
Critical Hdwy Stg 1	5.84	-	_	_	_	_
Critical Hdwy Stg 2	5.84	_	_	_	_	_
Follow-up Hdwy	3.52	3.32	2.22	_	-	_
Pot Cap-1 Maneuver	111	464	608	_	_	_
Stage 1	266	-	-	_	-	_
Stage 2	670	_	_	_	_	_
Platoon blocked, %	0.0			_	_	_
Mov Cap-1 Maneuver	105	464	608	_	_	_
Mov Cap-2 Maneuver	105	-0-	-	_	_	_
Stage 1	266	_	_	_	_	_
•	633	_	_	_	-	-
Stage 2	000	<u>-</u>	<u>-</u>	<u>-</u>	-	-
Approach	SE		NE		SW	
HCM Control Delay, s	27.3		0.6		0	
HCM LOS	D					
		NIEL	NET	SELn1 S	SEL 52	C///T
Minor Lang/Major Musel				3ELIII (	oELIIZ .	SWT
Minor Lane/Major Mvmt	t	NEL				
Capacity (veh/h)	t	608	- INET	105	464	-
Capacity (veh/h) HCM Lane V/C Ratio		608 0.055		105 0.29	464 0.124	-
Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s)		608 0.055 11.3	- - -	105 0.29 52.7	464 0.124 13.9	-
Capacity (veh/h) HCM Lane V/C Ratio		608 0.055		105 0.29	464 0.124	-

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Intersection						
Int Delay, s/veh	0.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
	WDL	WDN		NDI	SDL N	
Lane Configurations		10	<b>↑</b> ↑	10		1004
Traffic Vol, veh/h	10	10	575	10	10	1094
Future Vol, veh/h	10	10	575	10	10	1094
Conflicting Peds, #/hr	0	0	_ 0	_ 0	_ 0	_ 0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None		None	-	None
Storage Length	0	-	-	-	125	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	11	625	11	11	1189
Major/Minor	Minor1	N	/lajor1	N	Major2	
Conflicting Flow All	1246	318	0	0	636	0
Stage 1	630	-	-	-	-	-
Stage 2	616	-	-	-	-	-
Critical Hdwy	6.84	6.94	-	-	4.14	-
Critical Hdwy Stg 1	5.84	-	-	-	-	-
Critical Hdwy Stg 2	5.84	-	-	-	-	-
Follow-up Hdwy	3.52	3.32	-	-	2.22	-
Pot Cap-1 Maneuver	166	678	-	-	943	-
Stage 1	493	-	-	-	-	-
Stage 2	501	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	164	678	-	_	943	_
Mov Cap-2 Maneuver	164	-	_	_	-	_
Stage 1	493	_	_	_	_	_
Stage 2	495	-	_	_	_	_
Stage 2	433					
Approach	WB		NB		SB	
HCM Control Delay, s	19.9		0		0.1	
HCM LOS	С					
NA: 1 /24 : 24		NET	NIDD	MDL 4	051	007
Minor Lane/Major Mvm	11	NBT		VBLn1	SBL	SBT
Capacity (veh/h)		-	-	201	943	-
HCM Lane V/C Ratio		-	-	0.082		-
				10.0	8.9	_
HCM Control Delay (s)		-	-			
HCM Control Delay (s) HCM Lane LOS		-	-	С	Α	-
HCM Control Delay (s)						

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2.2						
SFI	SFR	NFI	NFT	SWT	SWR	
				-		
				-		
				-		
_						
30	57	104	1176	992	67	
Minor2	N	Maior1		Maior2		
					Ω	
	-	-				
	6.04	111				
		4.14				
		-		-		
	-	-	-	-	-	
			-	-	-	
	519	693	-	-	-	
	-	-	-	-	-	
404	-	-	-	-	-	
			-	-	-	
61	519	693	-	-	-	
61	-	-	-	-	-	
320	-	-	-	-	-	
343	-	_	_	-	_	
47.7		0.9		0		
Е						
mt	NEL	NET	SELn1	SFI n2	SWT	
TIT.		INL I	61	519	OVVI	
	ເດວ			:119	-	
	693	-				
	0.151		0.499	0.109	-	
s)	0.151 11.1	-	0.499 112.4	0.109 12.8	-	
s) 1)	0.151		0.499	0.109		
	SEL  28 28 0 Stop - 0 0 e, # 0 92 2 30  Minor2 1789 992 797 6.84 5.84 3.52 72 320 404 61 61 320 343  SE 47.7	SEL SER  28 52 28 52 0 0 0 Stop Stop - None 0 340 e, # 0 - 92 92 2 2 30 57  Minor2 N 1789 496 992 - 797 - 6.84 6.94 5.84 - 3.52 3.32 72 519 320 - 404 - 61 519 61 - 320 - 343 -  SE 47.7	SEL         SER         NEL           28         52         96           28         52         96           0         0         0           Stop         Stop         Free           None         -         -           0         340         675           e, # 0         -         -           92         92         92           2         2         2           30         57         104           Minor2         Major1         Major1           1789         496         992           992         -         -           797         -         -           6.84         6.94         4.14           5.84         -         -           3.52         3.32         2.22           72         519         693           320         -         -           404         -         -           320         -         -           343         -         -           343         -         -           47.7         0.9	SEL         SER         NEL         NET           28         52         96         1082           0         0         0         0           Stop         Stop         Free         Free           -         None         -         None           0         340         675         -           e, # 0         -         -         0           92         92         92         92           2         2         2         2           30         57         104         1176    Minor2  Major1  1789  496  992	SEL         SER         NEL         NET         SWT           28         52         96         1082         913           28         52         96         1082         913           0         0         0         0         0           Stop         Stop         Free         Free         Free           - None         -         None         -           0         340         675         -         -           e, # 0         -         -         0         0           92         92         92         92         92           2         2         2         2         2           30         57         104         1176         992           1789         496         992         0         -           992         -         -         -         -           797         -         -         -         -           5.84         -         -         -         -           5.84         -         -         -         -           3.52         3.32         2.22         -         -           72 <td>SEL         SER         NEL         NET         SWT         SWR           28         52         96         1082         913         62           28         52         96         1082         913         62           0         0         0         0         0         0           Stop         Stop         Free         Free</td>	SEL         SER         NEL         NET         SWT         SWR           28         52         96         1082         913         62           28         52         96         1082         913         62           0         0         0         0         0         0           Stop         Stop         Free         Free

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Int Delay, s/veh   Movement   WBL   WBR   NBT   NBR   SBL   SBT	Intersection						
Movement         WBL         WBR         NBT         NBR         SBL         SBT           Lane Configurations         ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑		0.4					
Lane Configurations         Y         ↑         ↑           Traffic Vol, veh/h         10         10         1168         10         10         955           Future Vol, veh/h         10         10         1168         10         10         955           Conflicting Peds, #/hr         0         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1	-		14/5-			0=:	05=
Traffic Vol, veh/h         10         10         1168         10         10         955           Future Vol, veh/h         10         10         1168         10         10         955           Conflicting Peds, #/hr         0         0         0         0         0         0         0           Sign Control         Stop         Stop         Free			WBR		NBR		
Future Vol, veh/h         10         10         1168         10         10         955           Conflicting Peds, #/hr         0							
Conflicting Peds, #/hr         0         0         0         0         0         0         0           Sign Control         Stop         Stop         Free         6         0         0         2         2         2         2         2 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>							
Sign Control         Stop         Stop         Free         Reg         Free         Free         Free         Free         Free         Free         Reg         Free         Free         Free         Free         Free         Free         Free         Free         Free         Reg         Stop         None         Do							
RT Channelized         - None         - None         - None         - None         Storage Length         0         125         125         0         Grade, which is a considered in the property of							
Storage Length		Stop				Free	
Veh in Median Storage, #         0         -         0         -         -         0           Grade, %         0         -         0         -         -         0           Peak Hour Factor         92         92         92         92         92         92           Heavy Vehicles, %         2 <td< td=""><td></td><td></td><td>None</td><td>-</td><td>None</td><td></td><td>None</td></td<>			None	-	None		None
Grade, %         0         -         0         -         -         0           Peak Hour Factor         92		-	-		-	125	
Peak Hour Factor         92			-	-	-	-	
Heavy Vehicles, %   2   2   2   2   2   2   2   2   2		-		-			
Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         1816         640         0         0         1280         0           Stage 1         1275         -         -         -         -         -           Critical Hdwy         6.84         6.94         -         -         4.14         -           Critical Hdwy Stg 1         5.84         -         -         -         -         -           Critical Hdwy Stg 2         5.84         -         -         -         -         -           Critical Hdwy Stg 2         5.84         -         -         -         -         -         -           Critical Hdwy Stg 2         5.84         -							
Major/Minor         Minor1         Major1         Major2           Conflicting Flow All         1816         640         0         0         1280         0           Stage 1         1275         -         -         -         -         -           Stage 2         541         -         -         -         -         -           Critical Hdwy         6.84         6.94         -         4.14         -           Critical Hdwy Stg 1         5.84         -         -         -         -         -           Critical Hdwy Stg 2         5.84         -	Heavy Vehicles, %	2	2	2	2	2	2
Conflicting Flow All         1816         640         0         0         1280         0           Stage 1         1275         -	Mvmt Flow	11	11	1270	11	11	1038
Conflicting Flow All         1816         640         0         0         1280         0           Stage 1         1275         -							
Conflicting Flow All         1816         640         0         0         1280         0           Stage 1         1275         -	Maio#/Mino#	Min = #1		11-:1		4-10	
Stage 1       1275       - <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Stage 2       541       -				0	0	1280	
Critical Hdwy       6.84       6.94       -       -       4.14       -         Critical Hdwy Stg 1       5.84       -       -       -       -       -         Critical Hdwy Stg 2       5.84       -				-	-	-	-
Critical Hdwy Stg 1         5.84         -				-	-	-	-
Critical Hdwy Stg 2         5.84         -	•		6.94	-	-	4.14	-
Follow-up Hdwy 3.52 3.32 - 2.22 - Pot Cap-1 Maneuver 69 418 - 538 - Stage 1 226 Stage 2 548 Platoon blocked, % Mov Cap-1 Maneuver 68 418 - 538 - Mov Cap-2 Maneuver 68 Stage 1 226 Stage 2 537  Approach WB NB SB HCM Control Delay, s 42.7 HCM LOS E  Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - 117 538 - HCM Lane V/C Ratio - 0.186 0.02 - HCM Control Delay (s) - 42.7 11.8 - HCM Lane LOS - E B			-	-	-	-	-
Pot Cap-1 Maneuver         69         418         -         -         538         -           Stage 1         226         -         -         -         -         -           Stage 2         548         -         -         -         -         -           Platoon blocked, %         -         -         -         -         -         -         -           Mov Cap-1 Maneuver         68         418         -         -         538         -           Mov Cap-2 Maneuver         68         -	Critical Hdwy Stg 2			-	-		-
Stage 1         226         -	Follow-up Hdwy		3.32	-	-		-
Stage 2       548       -       -       -       -         Platoon blocked, %         Mov Cap-1 Maneuver       68       418       -       -       538       -         Mov Cap-2 Maneuver       68       - <td< td=""><td>Pot Cap-1 Maneuver</td><td></td><td>418</td><td>-</td><td>-</td><td>538</td><td>-</td></td<>	Pot Cap-1 Maneuver		418	-	-	538	-
Platoon blocked, %	Stage 1	226	-	-	-	-	-
Mov Cap-1 Maneuver         68         418         -         -         538         -           Mov Cap-2 Maneuver         68         -	Stage 2	548	-	-	-	-	-
Mov Cap-2 Maneuver         68         -	Platoon blocked, %			-	-		-
Mov Cap-2 Maneuver         68         -		68	418	-	-	538	-
Stage 1         226         -			-	_	-	_	-
Stage 2         537         -			_	-	-	-	_
Approach         WB         NB         SB           HCM Control Delay, s         42.7         0         0.1           HCM LOS         E           Minor Lane/Major Mvmt         NBT         NBRWBLn1         SBL         SBT           Capacity (veh/h)         -         -         117         538         -           HCM Lane V/C Ratio         -         -         0.186         0.02         -           HCM Control Delay (s)         -         -         42.7         11.8         -           HCM Lane LOS         -         E         B         -	•		_	_	_	_	_
HCM Control Delay, s   42.7   0   0.1	olago 2	001					
HCM Control Delay, s   42.7   0   0.1							
Minor Lane/Major Mvmt         NBT         NBRWBLn1         SBL         SBT           Capacity (veh/h)         -         -         117         538         -           HCM Lane V/C Ratio         -         -         0.186         0.02         -           HCM Control Delay (s)         -         -         42.7         11.8         -           HCM Lane LOS         -         E         B         -							
Minor Lane/Major Mvmt         NBT         NBRWBLn1         SBL         SBT           Capacity (veh/h)         -         -         117         538         -           HCM Lane V/C Ratio         -         -         0.186         0.02         -           HCM Control Delay (s)         -         -         42.7         11.8         -           HCM Lane LOS         -         E         B         -				0		0.1	
Capacity (veh/h)       -       -       117       538       -         HCM Lane V/C Ratio       -       -       0.186       0.02       -         HCM Control Delay (s)       -       -       42.7       11.8       -         HCM Lane LOS       -       E       B       -	HCM LOS	Е					
Capacity (veh/h)       -       -       117       538       -         HCM Lane V/C Ratio       -       -       0.186       0.02       -         HCM Control Delay (s)       -       -       42.7       11.8       -         HCM Lane LOS       -       E       B       -							
Capacity (veh/h)       -       -       117       538       -         HCM Lane V/C Ratio       -       -       0.186       0.02       -         HCM Control Delay (s)       -       -       42.7       11.8       -         HCM Lane LOS       -       E       B       -	Minor Lane/Major Mym	t .	NRT	NRRV	WRI n1	SRI	SRT
HCM Lane V/C Ratio       -       -       0.186       0.02       -         HCM Control Delay (s)       -       -       42.7       11.8       -         HCM Lane LOS       -       E       B       -							
HCM Control Delay (s) 42.7 11.8 - HCM Lane LOS - E B -							
HCM Lane LOS E B -				-			
				-			
			-	-			
HCM 95th %tile Q(veh) 0.6 0.1 -	HUM 95Th %THE (J(Veh)		-	-	0.6	0.1	-

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Intersection						
Int Delay, s/veh	0.9					
Mayamant	CEL	CED	MEL	NICT	CMT	CIVID
Movement	SEL	SER	NEL	NET	SWT	SWR
Lane Configurations	_	7	<u>ነ</u>	<b>^</b>	<b>^</b>	7
Traffic Vol, veh/h	0	81	31	582	1051	65
Future Vol, veh/h	0	81	31	582	1051	65
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	675	-	-	475
Veh in Median Storage,	# 0	_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	88	34	633	1142	71
Major/Minor N	1inor2	N	Major1	Λ	//ajor2	
Conflicting Flow All	-	571	1142	0	-	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-	-
Pot Cap-1 Maneuver	0	464	608	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	_	_	_	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	_	464	608	_	_	_
		404	000	_		
Mov Cap-2 Maneuver	-	-	-	-	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	SE		NE		SW	
HCM Control Delay, s	14.6		0.6		0	
HCM LOS	В					
Minor Lane/Major Mvmt		NEL	NET	SELn1	SWT	SWR
		608		464	CVVI	OVVIC
Capacity (veh/h)			-		-	
HCM Lane V/C Ratio		0.055	-	0.19	-	-
HCM Control Delay (s)		11.3	-	14.6	-	-
HCM Lane LOS		В	-	В	-	-
HCM 95th %tile Q(veh)		0.2	-	0.7	-	-

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Intersection							
Int Delay, s/veh	0.5						
Movement	WBL	WBR	NBT	NBR	SBU	SBL	SBT
Lane Configurations	W		<b>†</b> 1>			ă	<b>^</b>
Traffic Vol, veh/h	10	10	575	10	28	10	1094
Future Vol, veh/h	10	10	575	10	28	10	1094
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-		-	-	None
Storage Length	0	-	-	-	-	125	-
Veh in Median Storage	, # 0	-	0	-	-	-	0
Grade, %	0	-	0	-	-	-	0
Peak Hour Factor	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	11	11	625	11	30	11	1189
Major/Minor	Minor1	N	Major1		Major2		
Conflicting Flow All	1307	318	0	0	635	636	0
Stage 1	630	310	-	U	000	030	-
Stage 2	677	-	_	_	-	_	_
Critical Hdwy	6.84	6.94	-	-	6.44	4.14	-
Critical Hdwy Stg 1	5.84	0.34	_	_	0.44	4.14	_
Critical Hdwy Stg 2	5.84		_			_	
Follow-up Hdwy	3.52	3.32	_	_	2.52	2.22	-
Pot Cap-1 Maneuver	151	678	_	_	568	943	
Stage 1	493	070	_		300	343	
Stage 2	466		_			-	
Platoon blocked, %	400	_	_	_	_	-	_
Mov Cap-1 Maneuver	151	678	_		627	627	
Mov Cap-1 Maneuver	151	-	_	_	- 021	021	-
Stage 1	493	-	-	-	<u>-</u>	-	<del>-</del>
Stage 2	466	-	_	-	-	-	-
Staye Z	400	<u>-</u>	_	<u>-</u>	<u>-</u>	_	_
Approach	WB		NB		SB		
HCM Control Delay, s	21		0		0.4		
HCM LOS	С						
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1	SBL	SBT	
Capacity (veh/h)		-	-	247	627	-	
HCM Lane V/C Ratio		_		0.088		_	
HCM Control Delay (s)		_	_	21	11.1	_	
HCM Lane LOS		_	_	C	В	_	
HCM 95th %tile Q(veh)		_	_	0.3	0.2	_	
TIOM JOHN JUHO Q(VOII)				0.0	0.2		

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Intersection						
Int Delay, s/veh	1					
Movement	CEL	CED	MEL	NICT	CWT	CMD
Movement	SEL	SER	NEL	NET	SWT	SWR
Lane Configurations	_	7	<u>ነ</u>	<b>^</b>	<b>^</b>	7
Traffic Vol, veh/h	0	80	96	1110	913	62
Future Vol, veh/h	0	80	96	1110	913	62
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	675	-	-	475
Veh in Median Storage,	# 0	-	_	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
	0		104			67
Mvmt Flow	U	87	104	1207	992	07
Major/Minor N	linor2	N	/lajor1	N	/lajor2	
Conflicting Flow All	_	496	992	0	_	0
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Critical Hdwy	-	6.94	4.14	-	-	-
Critical Hdwy Stg 1	-	-	-	-	-	-
Critical Hdwy Stg 2	-	-	-	-	-	-
Follow-up Hdwy	-	3.32	2.22	-	-	-
Pot Cap-1 Maneuver	0	519	693	-	-	-
Stage 1	0	-	-	-	-	-
Stage 2	0	-	_	_	_	-
Platoon blocked, %	-			_	_	_
Mov Cap-1 Maneuver	_	519	693	_	_	_
		513	030	_		_
Mov Cap-2 Maneuver	-	-	-	_	-	-
Stage 1	-	-	-	-	-	-
Stage 2	-	-	-	-	-	-
Approach	SE		NE		SW	
			0.9		0	
HCM Control Delay, s	13.3		0.9		U	
HCM LOS	В					
Minor Lane/Major Mvmt		NEL	NFT:	SELn1	SWT	SWR
Capacity (veh/h)		693	-		0111	STALL
HCM Lane V/C Ratio					-	-
		0.151		0.168	-	-
HCM Control Delay (s)		11.1	-	13.3	-	-
HCM Lane LOS		В	-	В	-	-
HCM 95th %tile Q(veh)		0.5	-	0.6	-	-

Synchro 9 Report Page 1 US 41B at 23rd St E 08/24/2018

Intersection							
Int Delay, s/veh	0.8						
Movement	WBL	WBR	NBT	NBR	SBU	SBL	SBT
Lane Configurations	Y	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	<b>†</b>	11511	-050	Ä	<b>†</b> †
Traffic Vol, veh/h	10	10	1168	10	28	10	955
Future Vol, veh/h	10	10	1168	10	28	10	955
Conflicting Peds, #/hr	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free	Free
RT Channelized	-	None	-		-	-	None
Storage Length	0	-	-	-	-	125	-
Veh in Median Storage	, # 0	-	0	-	-	-	0
Grade, %	0	-	0	-	-	-	0
Peak Hour Factor	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2
Mvmt Flow	11	11	1270	11	30	11	1038
Major/Minor I	Minor1	N	Major1	ı	Major2		
	1877	640	0	0	1279	1280	0
Conflicting Flow All Stage 1	1275	040		U	1219	1200	-
Stage 2	602	-	-	-	_	-	-
Critical Hdwy	6.84	6.94		_	6.44	4.14	-
Critical Hdwy Stg 1	5.84	0.34	-	-	0.44	4.14	-
Critical Hdwy Stg 2	5.84		-	-	<u>-</u>	_	-
Follow-up Hdwy	3.52	3.32	_	_	2.52	2.22	-
Pot Cap-1 Maneuver	63	418	-	-	2.52	538	-
Stage 1	226	710	_	-	213	550	-
Stage 2	510		_	-	<u>-</u>	-	-
Platoon blocked, %	310		_	_			_
Mov Cap-1 Maneuver	63	418			254	254	
Mov Cap-1 Maneuver	63	- 10	_	_	- 207	- 207	_
Stage 1	226	_		_		_	
Stage 2	510	_	_	_	_	_	_
Olaye Z	310		_			•	-
Approach	WB		NB		SB		
HCM Control Delay, s	46.1		0		0.8		
HCM LOS	E						
Minor Lane/Major Mvm	nt	NBT	NBRV	WBLn1	SBL	SBT	
Capacity (veh/h)		-	-	109	254	_	
HCM Lane V/C Ratio		_		0.199		_	
HCM Control Delay (s)		-	-		21.9	-	
HCM Lane LOS		-	-	Ε	С	-	
HCM 95th %tile Q(veh)	)	-	-	0.7	0.6	-	
					3.0		

US 41B at 23rd St E 08/24/2018 Synchro 9 Report Page 2



**APPENDIX F: CRASH DATA** 

HNTB Corporation September 2018

Section:	13030	State Route: US B 41	
Location:	23rd St W	M.P.: 0.867 -	Engineer: HNTB
Study Period:	1/1/2011 to 12/31/2011	County: Manatee	·

							Prop.	Day/	Wet/	Contributing	
No.	Date	Day	Time	Type	Fatal	Injury	Damage	Night	Dry	Cause	Comments
1	1/20/2011	Thu.	16:00	Left Turn		х		Day	Dry	FTYRW	
2	2/21/2011	Mon.	8:00	Angle		х		Day	Dry	FTYRW	
3	8/2/2011	Tue.	7:55	Rear End			х	Day	Dry	Careless Driving	
	•										

Total			Property		Left	Head	Ran Into	Rear	Side	Collision w/		Collision w/	Hit	Run Off	
No.	Fatal	Injury	Damage	Angle	Turn	On	Ditch	End	Swipe	Sign	Overturned	Pole	Animal	Road	Other
3	0	2	1	1	1	0	0	1	0	0	0	0	0	0	0
%	0	67	33	33	33	0	0	33	0	0	0	0	0	0	0
One	Ped/					Careless		Improper		Improper	Disregarded	Failed to	Improper	No Improper	
Vehicle	Bike	Day	Night	Dry	Wet	Driving	FTYRW	Turn	DUI	Lane Change	Traffic Signal	Maintain Vehicle	Load	Driving	Other
0	0	3	0	3	0	1	2	0	0	0	0	0	0	0	0
0	0	100	0	100	0	33	67	0	0	0	0	n	0	0	0

Section: 13030		State Route:	US B 41	
Location: 23rd St	N	M.P.: 0.867 -	·	Engineer: HNTB
Study Period: 1/1/20	12 to 12/31/2012	County: Mar	natee	-
No. of Years:	1			

							Prop.	Day/	Wet/	Contributing	
No.	Date	Day	Time	Type	Fatal	Injury	Damage	Night	Dry	Cause	Comments
1	1/5/2012	Thu.	13:03	Other			х	Day	Dry	Careless Driving	
2	10/13/2012	Sat.	14:40	Angle		Х		Day	Dry	Careless Driving	

Total			Property		Left	Head	Ran Into	Rear	Side	Collision w/		Collision w/	Hit	Run Off	
No.	Fatal	Injury	Damage	Angle	Turn	On	Ditch	End	Swipe	Sign	Overturned	Pole	Animal	Road	Other
2	0	1	1	1	0	0	0	0	0	0	0	0	0	0	1
%	0	50	50	50	0	0	0	0	0	0	0	0	0	0	50
One	Ped/					Careless		Improper		Improper	Disregarded	Failed to	Improper	No Improper	
Vehicle	Bike	Day	Night	Dry	Wet	Driving	FTYRW	Turn	DUI	Lane Change	Traffic Signal	Maintain Vehicle	Load	Driving	Other
0	0	2	0	2	0	2	0	0	0	0	0	0	0	0	0
0	0	100	0	100	0	100	0	0	0	0	0	0	0	0	0

Section:	13030	State Route: US B 41	
Location:	23rd St W	M.P.: 0.867 -	Engineer: HNTB
Study Period:	1/1/2013 to 12/31/2013	County: Manatee	

							Prop.	Day/	Wet/	Contributing	
No.	Date	Day	Time	Type	Fatal	Injury	Damage	Night	Dry	Cause	Comments
1	2/26/2013	Tue.	7:20	Angle		х		Night	Dry	FTYRW	
2	9/13/2013	Fri.	9:45	Sideswipe			Х	Day	Dry	Careless Driving	
3	10/5/2013	Sat.	4:38	Off Road		Х		Night	Dry	Disregarded Stop Sign	
	·									·	
	·										

Total			Property		Left	Head	Ran Into	Rear	Side	Collision w/		Collision w/	Hit	Run Off	
No.	Fatal	Injury	Damage	Angle	Turn	On	Ditch	End	Swipe	Sign	Overturned	Pole	Animal	Road	Other
3	0	2	1	1	0	0	0	0	1	0	0	0	0	1	0
%	0	67	33	33	0	0	0	0	33	0	0	0	0	33	0
One	Ped/					Careless		Improper		Improper	Disregarded	Disregarded	Improper	No Improper	
Vehicle	Bike	Day	Night	Dry	Wet	Driving	FTYRW	Turn	DUI	Lane Change	Traffic Signal	Stop Sign	Load	Driving	Other
0	0	1	2	3	0	1	1	0	0	0	0	1	0	0	0
0	0	33	67	100	0	33	33	0	0	0	0	33	0	0	0

 Section:
 13030
 State Route:
 US B 41

 Location:
 23rd St W
 M.P.:
 0.867 Engineer:
 HNTB

 Study Period:
 1/1/2014 to 12/31/2014
 County:
 Manatee

 No. of Years:
 1

							Prop.	Day/	Wet/	Contributing	
No.	Date	Day	Time	Type	Fatal	Injury	Damage	Night	Dry	Cause	Comments
1	1/9/2014	Thu.	15:31	Angle			х	Day	Dry	FTYRW	
2	4/11/2014	Fri.	7:40	Right Turn			Х	Day	Dry	Careless Driving	
3	4/14/2014	Mon.	14:56	Right Turn		х		Day	Dry	FTYRW	
4	8/16/2014	Sat.	4:38	Off Road			Х	Night	Dry	Disregarded Stop Sign	
5	9/2/2014	Tue.	9:30	Sideswipe			х	Day	Dry	No Improper Driving	

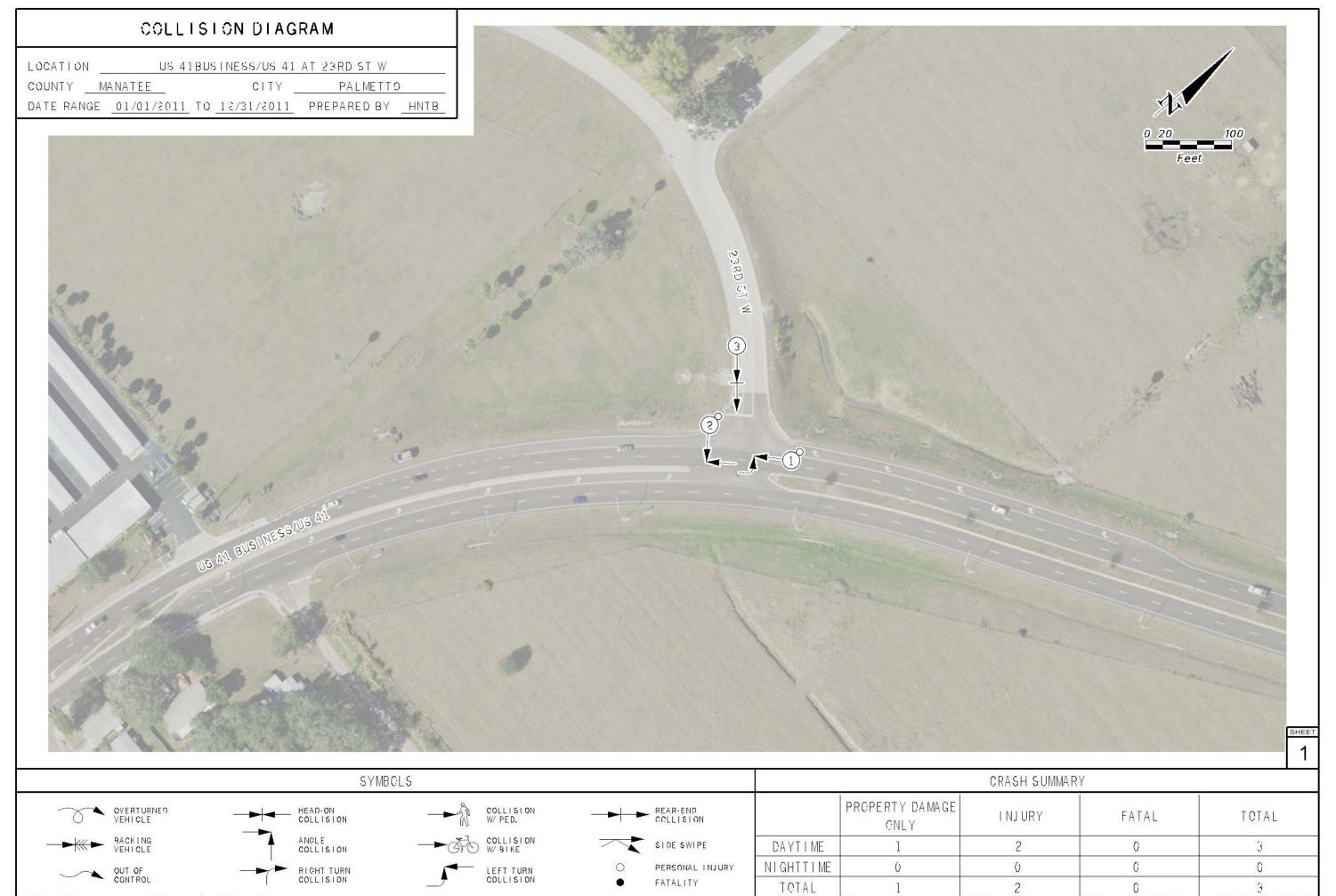
Total			Property		Left	Head	Right	Rear	Side	Collision w/		Collision w/	Hit	Run Off	
No.	Fatal	Injury	Damage	Angle	Turn	On	Turn	End	Swipe	Sign	Overturned	Pole	Animal	Road	Other
5	0	1	4	1	0	0	2	0	1	0	0	0	0	1	0
%	0	20	80	20	0	0	40	0	20	0	0	0	0	20	0
One	Ped/					Careless		Improper		Improper	Disregarded	Disregarded	Improper	No Improper	
Vehicle	Bike	Day	Night	Dry	Wet	Driving	FTYRW	Turn	DUI	Lane Change	Traffic Signal	Stop Sign	Load	Driving	Other
0	0	4	1	5	0	1	2	0	0	0	0	1	0	1	0
0	0	80	20	100	0	20	40	0	0	0	0	20	0	20	0

### Notice:

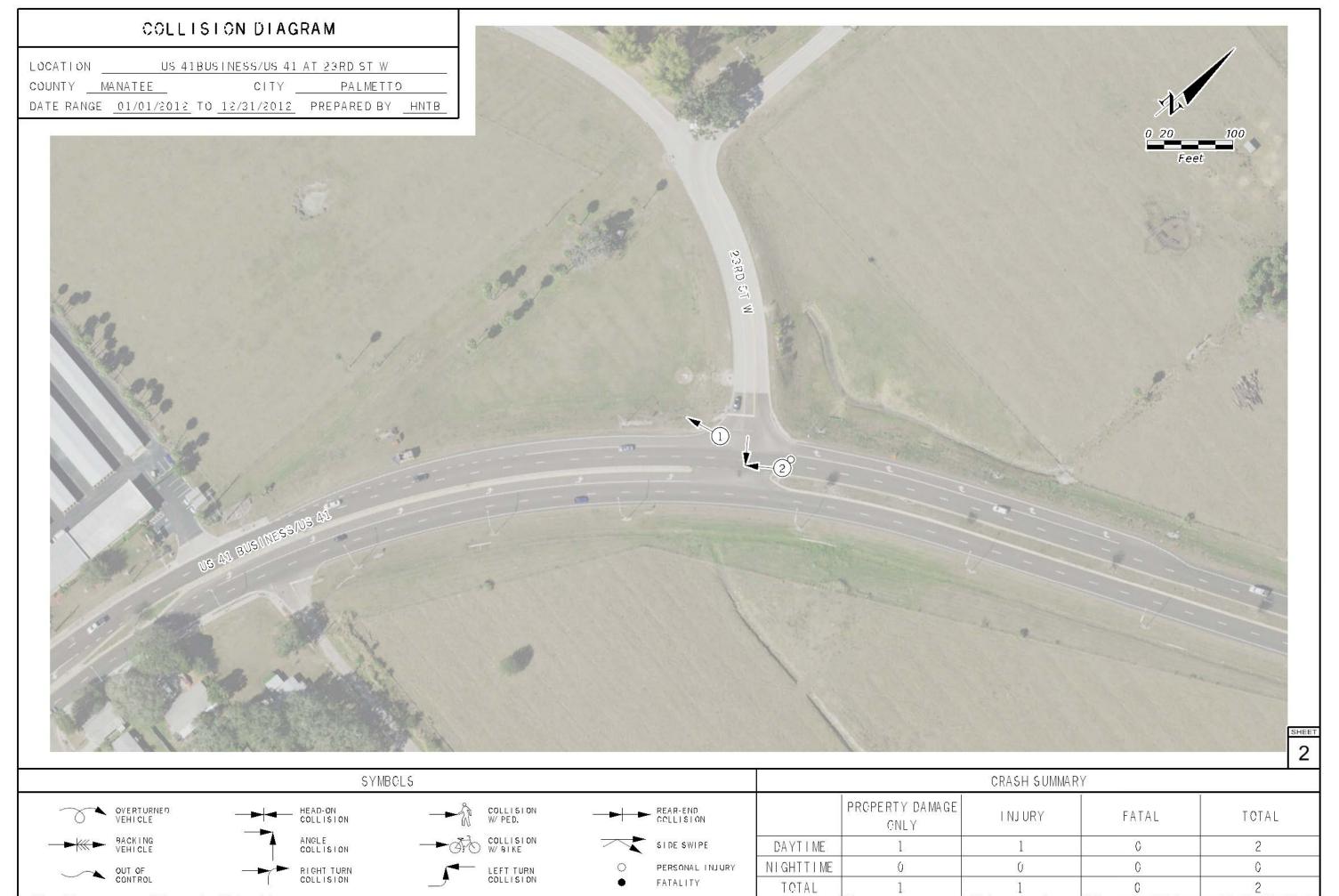
Section:	13030	State Route: US B 41	
Location:	23rd St W	M.P.: 0.867 -	Engineer: HNTB
Study Period:	1/1/2015 to 12/31/2015	County: Manatee	•

							Prop.	Day/	Wet/	Contributing	
No.	Date	Day	Time	Type	Fatal	Injury	Damage	Night	Dry	Cause	Comments
1	2/6/2015	Fri.	16:20	Head On			х	Day	Dry	Careless Driving	
2	2/19/2015	Thu.	21:00	Rear End			Х	Night	Dry	Careless Driving	
3	5/18/2015	Mon.	14:45	Angle		х		Day	Dry	Careless Driving	
4	5/26/2015	Tue.	14:02	Angle			Х	Day	Dry	Careless Driving	

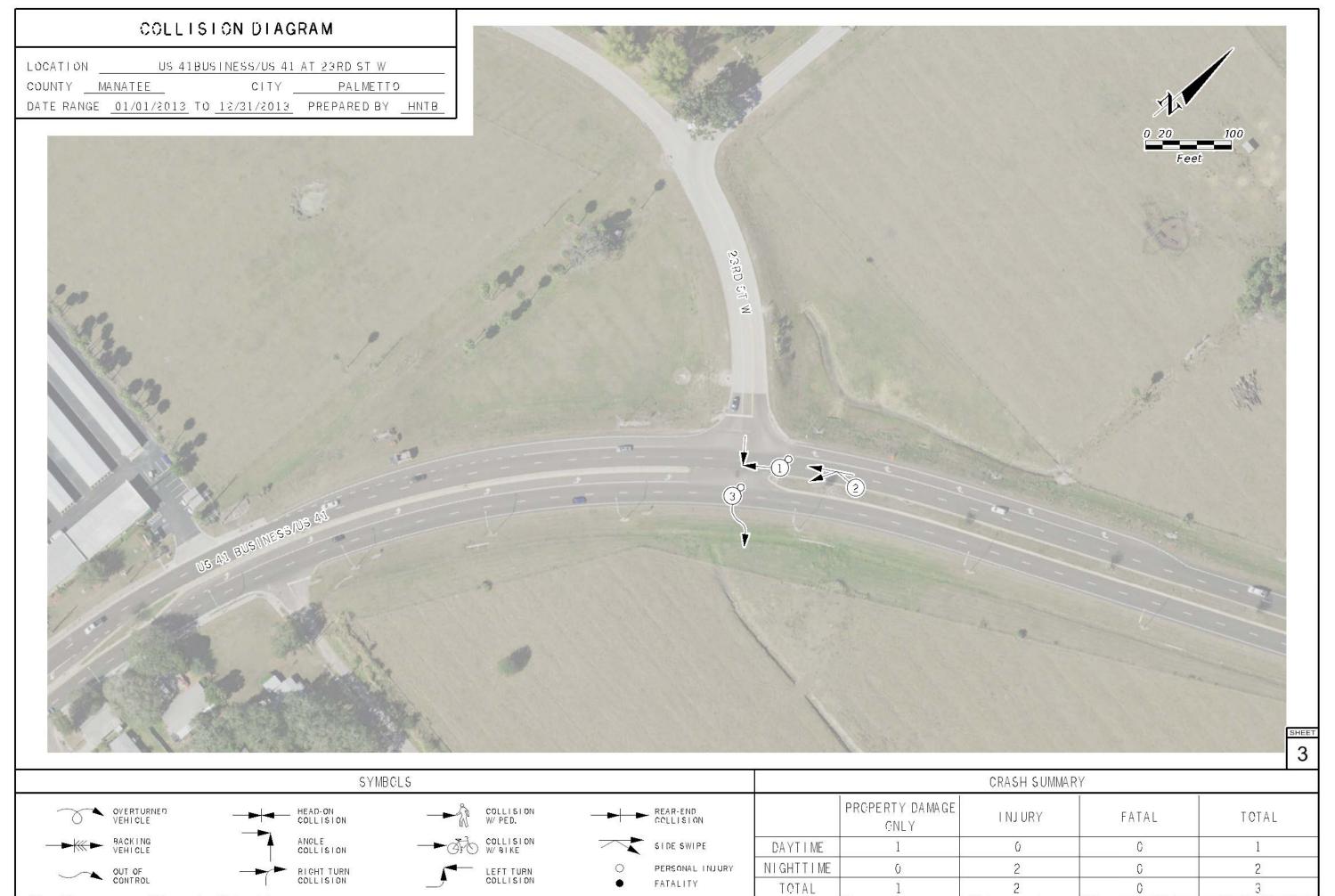
Total			Property		Left	Head	Ran Into	Rear	Side	Collision w/		Collision w/	Hit	Run Off	
No.	Fatal	Injury	Damage	Angle	Turn	On	Ditch	End	Swipe	Sign	Overturned	Pole	Animal	Road	Other
4	0	1	3	2	0	1	0	1	0	0	0	0	0	0	0
%	0	25	75	50	0	25	0	25	0	0	0	0	0	0	0
One	Ped/					Careless		Improper		Improper	Disregarded	Failed to	Improper	No Improper	
Vehicle	Bike	Day	Night	Dry	Wet	Driving	FTYRW	Turn	DUI	Lane Change	Traffic Signal	Maintain Vehicle	Load	Driving	Other
0	0	3	1	4	0	4	0	0	0	0	0	0	0	0	0
0	0	75	25	100	0	100	0	0	0	0	0	0	0	0	0



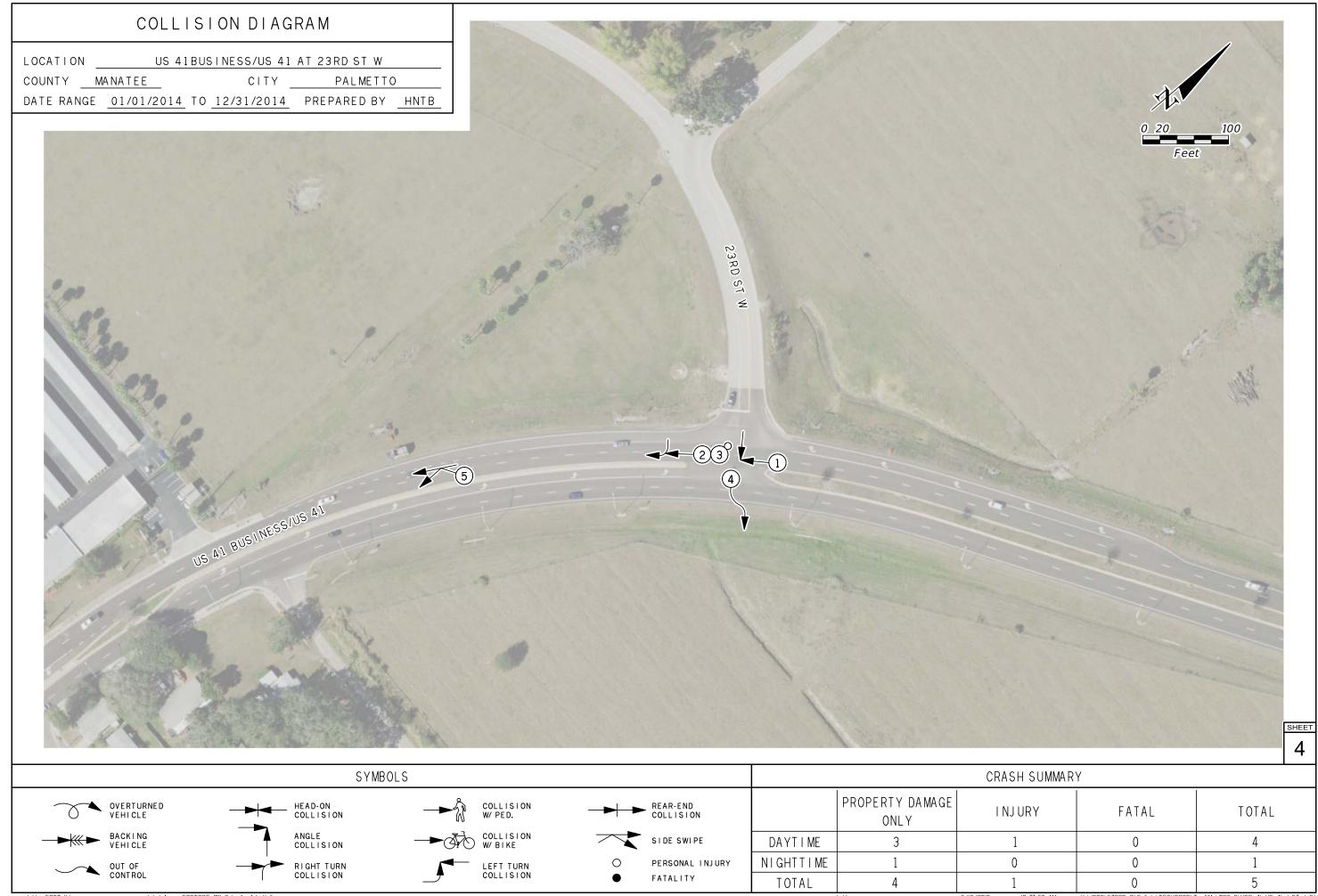
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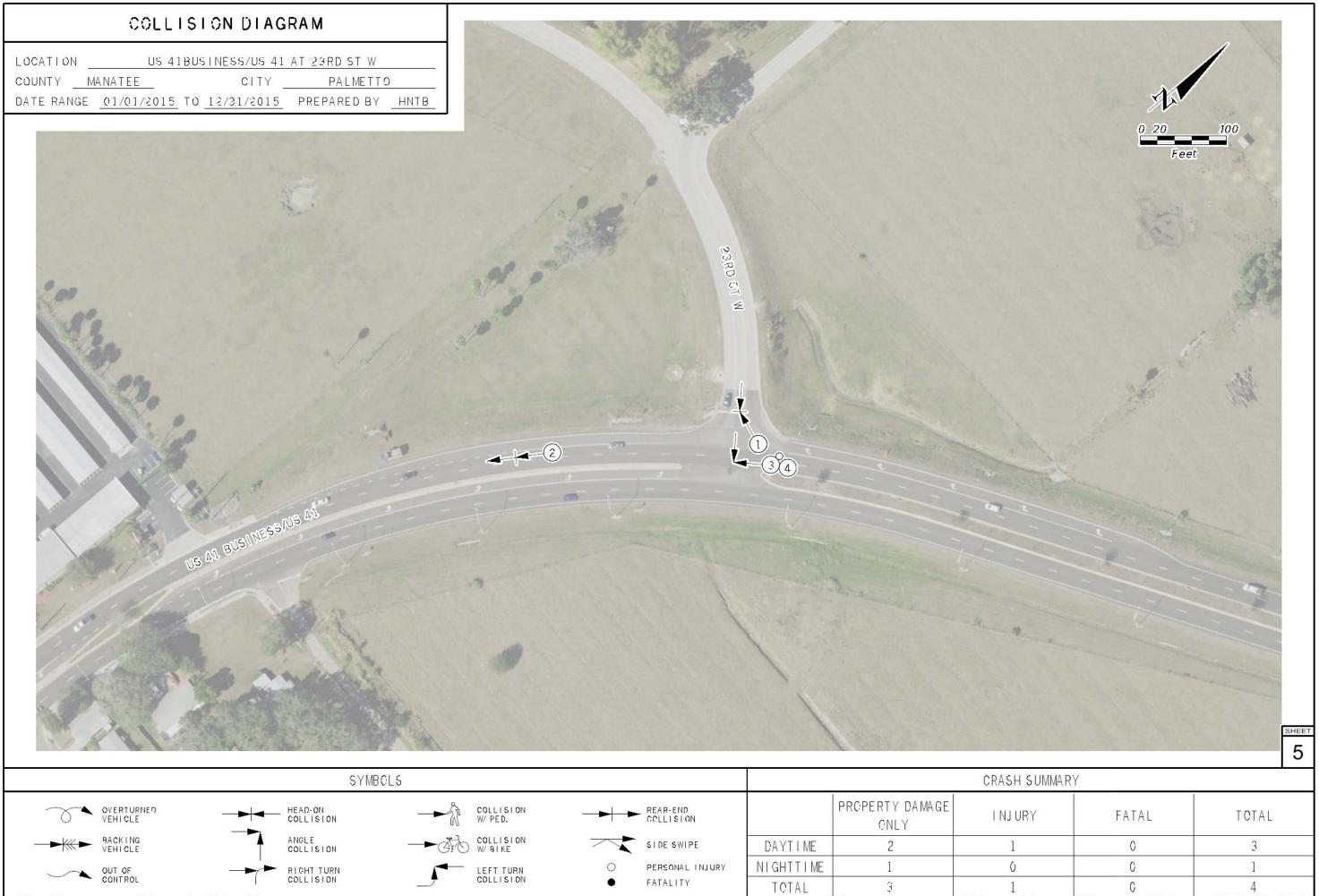


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rbabb 6/11/2018 8:07:19 AM V:\\085\63228-DI Safety\\TECHPROD\\Trafflo\\TWO 9\\S8 4I-\US 4I at 23rd 5f W







**APPENDIX G: INTERSECTION PHOTOS** 

HNTB Corporation September 2018

# **Northbound Approach**



Looking north into the intersection from U.S. Business 41

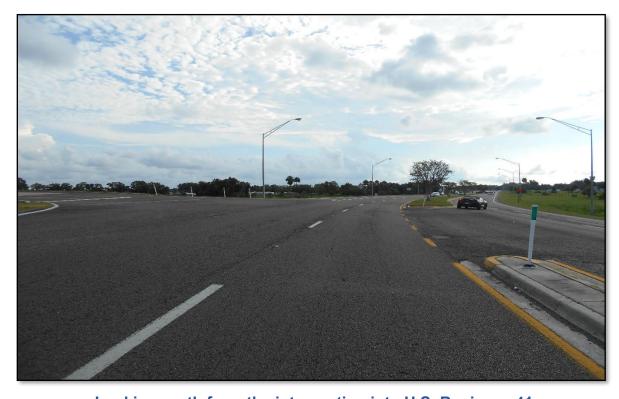


Looking south from the intersection into U.S. Business 41

# **Southbound Approach**



Looking south into the intersection from U.S. Business 41



Looking north from the intersection into U.S. Business 41

# **Eastbound Approach**



Looking east into the intersection from 23<sup>rd</sup> Street West



Looking west from the intersection into 23<sup>rd</sup> Street West



## **APPENDIX H:** CONCEPTUAL DESIGN PLANS

HNTB Corporation September 2018

