## Noise Study Report Addendum No. 2

Florida Department of Transportation

District One

SR 710 Widening and Extension

From US 441 to the L-63N Canal

Okeechobee County, Florida

FPID Number: 419344-3-32-01

June 2024

The environmental review, consultation, and other actions required by applicable federal environmental laws for this project are being, or have been, carried out by FDOT pursuant to 23 U.S.C. § 327 and a Memorandum of Understanding dated May 26, 2022, and executed by the FHWA and FDOT.

## **Table of Contents**

1.0	Introduction	1	1-1
1.1	Project De	escription	1-1
1.2	Summary	of PD&E and 2019 Design Noise Study Results	1-2
1.	2.1 Okeed	chobee Health Care Facility	1-2
1.	2.2 Reside	ential Sites	1-2
1.	2.3 Noise	Abatement Considerations	1-2
1.3	Design Im	nprovements	1-3
1.4	Changes i	in Noise Regulations	1-3
2.0	Methodolog	]y	2-1
2.1	Noise Me	trics	2-1
2.	1.1 Traffic	c Noise Modeling and Traffic Data	2-2
2.2	Noise Aba	atement Criteria	2-3
2.3	Noise Aba	atement Measures	2-5
3.0	Traffic Noise	e Analysis	3-1
3.1	Predicted	Noise Levels and Abatement Analysis	3-1
3.2	Noise Impa	act Contours	3-12
4.0	Conclusions		4-1
5.0	Construction	n Noise and Vibration	5-1
6.0	References		6-1

## List of Figures

Figure 3-1	Design Concepts Comparison Map	.3-3
Figure 3-2	Noise Evaluation Map	.3-4
List of T	Tables	
Table 2-1	Sound Levels of Typical Noise Sources and Environments	2-2
Table 2-2	Noise Abatement Criteria [Hourly A-Weighted Sound Level-decibels (dB(A))]	2-4
Table 3-1	Representative Noise Sensitive Receptor Sites and Noise Impact Analysis Results	3-11
Table 3-2	Design Year Noise Abatement Criteria Contour Distances	3-12

## **Appendices**

Appendix A	Approved Traffic Data for Noise Analysis
Appendix B	Referenced Roadway Design Plans
Appendix C	Referenced Okeechobee Health Care Facility Design Plans
Appendix D	TNM Files (provided separately in electronic format)

#### **Executive Summary**

The Florida Department of Transportation (FDOT), District One, is preparing final design plans for the realignment and extension of SR 710 in Okeechobee County. The project includes a new four-lane divided roadway on new alignment from US 441 to SR 710 just west of the L-63N Canal Bridge and widening the existing two-lane SR 710 roadway from just west of the L-63N Canal Bridge to CR 714.

A Project Development and Environment (PD&E) study was completed for SR 710 and FDOT's Office of Environmental Management (OEM) granted Location and Design Concept Acceptance (LDCA) for the SR 710 PD&E Study on March 16, 2017. As part of the PD&E Study, a traffic noise study was conducted (Final Noise Study Report completed in 2012) that determined that no noise sensitive sites would be impacted by the proposed improvements. As part of the ongoing design phase, a more detailed traffic noise study was performed and documented in a Noise Study Report Addendum (NSRA) in 2019. This NSRA is an update to the 2019 NSRA due to proposed design changes. The proposed changes include approximately one mile of the SR 710 being realigned to avoid impacts to the Okeechobee Utility Authority wellfield.

This study was conducted based on the methodology described in the latest version of the FDOT PD&E Manual, the *Traffic Noise Modeling and Analysis Practitioners Handbook* and in accordance with *Title 23 CFR Part 772, Procedures for Abatement of Highway Traffic Noise and Construction Noise.* FHWA's Traffic Noise Model (TNM) Version 2.5 (February 2004) was used to predict traffic noise levels and to analyze the effectiveness of noise barriers, if warranted. The TNM 2.5 model Is FHWA's latest approved noise model and was the noise model that was used in both the 2012 PD&E and the 2019 NSRA.

This study found that design year (2051) traffic noise levels are predicted to range from 48.8 dB(A) to 62.3 dB(A) at exterior noise sensitive sites along the corridor. Therefore, design year traffic noise levels for the Build Alternative are not predicted to approach or exceed the Noise Abatement Criteria (NAC) at any noise sensitive sites along the project corridor.

#### 1.0 **Introduction**

The Florida Department of Transportation (FDOT), District One, completed a Project Development and Environment (PD&E) study for SR 710 from US 441 in Okeechobee County to County Road (CR) 714 (SW Martin Highway) in Martin County (FPID# 419344-2-22-01). As part of the PD&E, a noise study was performed and documented in a Noise Study Report (NSR) completed in 2012. On March 16, 2017, the FDOT's Office of Environmental Management (OEM) granted Location and Design Concept Acceptance (LDCA) for the SR 710 PD&E study. As a commitment made during the 2012 PD&E study, a design-phase noise study was performed and documented in a Noise Study Report Addendum (NSRA) dated 2019. This current NSRA, prepared by RS&H (under contract with FDOT), is an update to the 2019 NSRA due to proposed design changes and to confirm the commitments made in the 2012 PD&E study NSR.

The current design concept differs from the previous concept (2019) in that approximately one mile of the new SR 710 is being realigned to avoid impacts to the Okeechobee Utility Authority wellfield. The proposed project improvements are described in **Section 1.1**. **Section 1.2** provides a summary of the results from the 2012 PD&E Noise study and the 2019 NSRA. **Section 1.3** provides the design changes made after the 2019 NSRA. **Section 1.4** provides insight into the alterations in noise regulations that have occurred since the completion of the PD&E study. The methodology and results of this noise study analysis are summarized in **Sections 2.0** and **3.0**, respectively.

#### 1.1 **Project Description**

In order to accommodate future projected traffic volumes, improve safety and enhance the emergency evacuation capabilities of SR 710, the existing two-lane undivided arterial roadway is proposed to be improved to a four-lane divided roadway from SR 70 in Okeechobee County to CR 714 (SW Martin Highway) in Martin County, Florida. In addition, a new four-lane divided roadway extension of SR 710 is proposed from the vicinity of the current terminus at SR 70 to US 441 (SR 15 / South Parrott Avenue). The extension of SR 710 is proposed to improve mobility and connectivity within the regional transportation network and to provide for a multi-modal connection to the planned industrial park adjacent to Okeechobee County Airport. The design segment associated with this report extends from US 441 to the bridge over the L-63N Canal (located approximately one mile south of SR 70). The new extension will serve as a bypass route to address traffic congestion at the intersection of SR 70 and US 441 and will also help to reduce truck traffic through the City of Okeechobee.

The project begins with a new intersection on US 441 located approximately one mile north of the existing SR 70 and US 441 intersection. The new extension traverses southeasterly to connect with existing SR 710 at the L-63N Canal (see **Figure 3-1**). The total project length is approximately

3.864 miles. The SR 710 extension will include two lanes in each direction with curb along both the median and outside lanes, minor design modifications were added and are discussed in **Section 1.3**. The posted speed will be 45 mph. The posted speed will reduce to 40 mph near the new intersection at US 441. The SR 710 extension will include 7-foot bicycle lanes, a 6-foot sidewalk along the south side of the roadway, and a 10-foot shared use path along the north side of the roadway. The SR 710 extension will have new signals at the intersections with US 441, SR 70, and SE 40th Avenue. The proposed roadway alignment is depicted in **Figure 3-1**.

#### 1.2 Summary of PD&E and 2019 Design Noise Study Results

As part of the 2012 PD&E study, a traffic noise study was conducted in accordance with FDOT's PD&E Manual and a NSR, dated March 2012, was completed. The 2012 PD&E and 2019 Noise Studies evaluated all noise sensitive sites along the proposed SR 710 alignment and predicted design year traffic noise levels. Noise sensitive land uses included the Okeechobee Health Care Center (Activity Category C of the Noise Abatement Criteria (NAC)) and multiple residential areas (Activity Category B of the NAC). The majority of the properties along the proposed alignment are undeveloped and are not considered noise sensitive.

#### 1.2.1 Okeechobee Health Care Facility

The Okeechobee Health Care Center is located east of US 441 and north of the proposed SR 710 alignment. The PD&E and 2019 Noise Studies predicted design year noise levels associated with the project would not approach or exceed the NAC for Activity Category C at this location. Existing noise levels measured and reported in the PD&E and 2019 Noise Studies were compared and noise levels were not predicted to substantially increase above existing conditions (i.e.: increase more than 15 dB(A)). The representative sites used in the analysis are identified in **Figure 3-2**.

#### 1.2.2 Residential Sites

There are several scattered and isolated residences, as well as small neighborhood communities, along the proposed SR 710 new alignment. None of the residences were predicted to approach or exceed the NAC for residential land use with the design year proposed conditions in the PD&E and 2019 Noise Studies. In addition, no residential sites were predicted to experience a substantial increase above existing conditions as a result of this project.

#### 1.2.3 Noise Abatement Considerations

Noise barriers were not evaluated or recommended since no impacts were predicted for the noise sensitive sites within the project limits of the PD&E and 2019 Noise Studies. The PD&E and 2019 Noise Studies stated that there was no further commitment regarding the consideration of noise barriers during the design phase of the project.

#### 1.3 **Design Improvements**

The current concept proposed differs from the prior 2019 concept in that approximately one mile of the new SR 710 is being realigned to avoid impacts to the Okeechobee Utility Authority wellfield. Starting approximately 150 feet east of Taylor Creek, the centerline of the road shifts north of the prior alignment, before converging with the original alignment east of the proposed Pond 2 site. There is no change in the proposed roadway typical section. The maximum difference between the two alignments is 275 feet, occurring near Station 536+00. For a comparison of the original and realigned roadway design see **Figure 3-1**.

#### 1.4 Changes in Noise Regulations

Since the completion of the 2012 PD&E NSR, the FDOT has updated the PD&E Manual to accommodate the changes related to FHWA's National Environmental Policy Act (NEPA) delegation to FDOT, and has an effective date of July 1, 2023. However, the basic policies and procedures used in the identification of noise impacts remain the same. Therefore, there have not been any changes to FDOT's or FHWA's policies or regulations that would change the number of impacted residences or the consideration of noise abatement measures for any sites impacted by design year traffic noise from the proposed improvements to SR 710.

#### 2.0 **Methodology**

This study was conducted based on the methodology described in the latest version of the FDOT PD&E Manual, the *Traffic Noise Modeling and Analysis Practitioners Handbook*, and in accordance with *Title 23 CFR Part 772*, *Procedures for Abatement of Highway Traffic Noise and Construction Noise*. The noise study involved the following procedures to determine changes in noise impacts along the project corridor and to evaluate the feasibility and reasonableness of warranted noise abatement measures at impacted noise sensitive sites:

- A review of the 2012 PD&E Noise Study Report to identify all potentially impacted noise sensitive sites and any commitments concerning noise abatement within the project limits (Section 1.2);
- A review of the current project design to determine if any major changes in the proposed project occurred since the date of the approved environmental document (**Section 1.3**);
- A review of all state and federal requirements related to the traffic noise impact assessment (**Section 1.4**);
- A review of the existing land use to determine if additional noise sensitive sites have been built or permitted since the completion of the 2012 PD&E noise study between when the NSR was completed (i.e., March 2012) and the Date of Public Knowledge (DOPK) of March 16, 2017 (see Section 3.1); and
- Prediction of future design year traffic noise levels, assessment of traffic noise impacts and, consideration of abatement measures, if warranted (**Section 3.1**).

The following sections describe the noise metrics, traffic noise model, traffic data, and the noise abatement criteria used in this noise analysis.

#### 2.1 **Noise Metrics**

Noise levels documented in this report represent the hourly equivalent sound level [Leq(h)]. Leq(h) is the steady-state sound level, which contains the same amount of acoustic energy as the actual time-varying sound level over a 1-hour period. Leq(h) is measured in A-weighted decibels [dB(A)], which closely approximate the human frequency response. Sound levels of typical noise sources and environments are provided in **Table 2-1** as a frame of reference.

The public reaction to noise levels is usually a function of location (i.e., urban, suburban, or rural), time of day, fluctuations, duration, and the individual listening. Under normal conditions, a change in noise level of at least 3 dB(A) is required for the average person to perceive a difference. Examples of the magnitude of individual change in either traffic volume, vehicle speed, or distance from the noise source to the receptor, necessary to result in a 3 dB(A) increase include:

- A doubling in hourly traffic volumes with no change in speed;
- An increase in vehicular speed of 15 miles per hour (mph); or

• A reduction in the distance by half between the receptor and the noise source (e.g., vehicle traffic).

**Table 2-1 Sound Levels of Typical Noise Sources and Environments** 

COMMON OUTDOOR ACTIVITIES	NOISE LEVEL dB(A)	COMMON INDOOR ACTIVITIES
	110	Rock Band
Jet Fly-over at 1000 ft		
	100	
Gas Lawn Mower at 3 ft		
	90	
Diesel Truck at 50 ft, at 50 mph		Food Blender at 1 m (3 ft)
	80	Garbage Disposal at 1 m (3 ft)
Noise Urban Area (Daytime)		
Gas Lawn Mower at 100 ft	70	Vacuum Cleaner at 10 ft
Commercial Area		Normal Speech at 3 ft
Heavy Traffic at 300 ft	60	
		Large Business Office
Quiet Urban Daytime	50	Dishwasher Next Room
Quiet Urban Nighttime	40	Theater, Large Conference Room
Quiet Suburban Nighttime		(Background)
	30	Library
Quiet Rural Nighttime		Bedroom at Night, Concert Hall
	20	(Background)
	10	
Lowest Threshold of Human Hearing	0	
Source: California Dept. of Transportation Te	Ů	Lowest Threshold of Human Hearing plement, Oct. 1998, Page 18.

2.1.1 Traffic Noise Modeling and Traffic Data

FHWA's Traffic Noise Model (TNM) Version 2.5 (February 2004) was used to predict future traffic noise levels and to analyze the effectiveness of noise barriers. The TNM 2.5 model was also used in the 2012 PD&E noise study and the 2019 NSRA. This model estimates the acoustic intensity at noise sensitive receptor sites from a series of roadway segments (the source). Model-predicted noise levels are influenced by several factors, such as vehicle speed and distribution of vehicle types. Noise levels are also affected by characteristics of the source-to-receptor site path, including the effects of intervening barriers, structures (houses, buildings, etc.), ground surface type (hard or soft), and topography.

Noise receptor sites adjacent to the proposed SR 710 alignment, representing the noise sensitive land uses, were used as inputs to the TNM 2.5 to predict noise levels associated with the future

design year (2051) conditions. Traffic noise levels were predicted at the edge of the residence (or other noise sensitive site) closest to the nearest primary roadway. The horizontal and vertical roadway geometry and ground elevations used in the TNM were based on the latest proposed design plans and cross sections as well as available design survey and LiDAR data. The relevant pages from the design plans (i.e., plan view sheets) are included in **Appendix B**.

No commitment was made to reevaluate noise abatement measures in the PD&E and 2019 Noise Studies; only to evaluate noise sensitive land uses permitted between the Final Noise Study Report and the Date of Public Knowledge (DOPK). No additional qualifying noise sensitive sites were identified in the 2019 NSRA. Per the FDOT PD&E Manual, the FDOT is not responsible for providing noise abatement for noise sensitive land uses permitted for construction after the DOPK.

The peak hour traffic volumes and percentage of cars, medium trucks, heavy trucks, buses, and motorcycles for the future design year build conditions, as well as the level of service (LOS) C volumes for SR 710 Extension and US 441 are included in **Appendix A** from the 2019 noise study. Additionally, for the SR 710 segment that has been realigned, an updated traffic data set has been developed using new design year forecasts and current LOS C volumes from FDOT's 2023 Multimodal Quality/Level of Service Handbook's generalized service volume tables. The updated traffic data is also provided in **Appendix A**. The traffic volumes used to predict noise levels included the least of either the traffic capacity of the roadway at LOS C or the projected traffic demand of the roadway. These traffic volumes can be expected to produce the noisiest traffic conditions likely to occur during the design year.

#### 2.2 **Noise Abatement Criteria**

**Table 2-2** presents the NAC for land use categories established by the FHWA. Maximum noise threshold levels, or criteria levels, have been established for five of the seven activity categories. These criteria determine when an impact occurs and when consideration of noise abatement is required. Noise abatement measures must be considered when predicted noise levels approach or exceed the NAC levels or when a substantial noise increase occurs. A substantial noise increase occurs when the existing noise level is predicted to be exceeded by 15 dB(A) or more as a result of the transportation improvement project. The FDOT defines "approach" as within 1.0 dB(A) of the FHWA criteria.

Noise sensitive receptor sites include properties where frequent exterior human use occurs and where a lowered noise level would be of benefit. This includes residential land use (Activity Category B); a variety of nonresidential land uses not specifically covered in Category A or B including parks and recreational areas, medical facilities, schools, and places of worship (Activity Category C); and commercial and developed properties including offices, hotels, and restaurants with exterior areas of use (Activity Category E). Noise sensitive sites also include interior use areas

where no exterior activities occur for facilities such as auditoriums, day care centers, hospitals, libraries, medical facilities, places of worship, public meeting rooms, recording studios, schools, and television studios (Activity Category D). Categories F and G, which include commercial and developed properties without exterior areas of use, do not have noise abatement criteria levels. Category F includes land uses such as industrial and retail facilities that are not considered noise sensitive. Category G includes undeveloped lands.

The noise sensitive areas being evaluated for noise impacts fall within Activity Category B, C and D. As described in the following section, noise abatement measures, in the form of noise barriers, must be considered when predicted design year noise levels approach or exceed this NAC level.

Table 2-2 Noise Abatement Criteria [Hourly A-Weighted Sound Level-decibels (dB(A))]

Activity	Activity	Leq(h) <sup>1</sup>	Evaluation	Description of Activity Catagony
Category	FHWA	FDOT	Location	Description of Activity Category
А	57	56	Exterior	Lands on which serenity and quiet are of extraordinary significance and serve an important public need and where the preservation of those qualities is essential if the area is to continue to serve its intended purpose.
B <sup>2</sup>	67	66	Exterior	Residential
C <sup>2</sup>	67	66	Exterior	Active sports areas, amphitheaters, auditoriums, campgrounds, cemeteries, day care centers, hospitals, libraries, medical facilities, parks, picnic areas, places of worship, playgrounds, public meeting rooms, public or nonprofit institutional structures, radio studios, recording studios, recreational areas, Section 4(f) sites, schools, television studios, trails, and trail crossings.
D	52	51	Interior	Auditoriums, day care centers, hospitals, libraries, medical facilities, places of worship, public meeting rooms, public or nonprofit institutional structures, radio studios, recording studios, schools, and television studios.
E <sup>2</sup>	72	71	Exterior	Hotels, motels, offices, restaurants/bars, and other developed lands, properties or activities not included in A-D or F.
F	-	-	-	Agriculture, airports, bus yards, emergency services, industrial, logging, maintenance facilities, manufacturing, mining, rail yards, retail facilities, shipyards, utilities (water resources, water treatment, electrical), and warehousing.
G	_	_	_	Undeveloped lands that are not permitted.

(Based on Table 1 of 23 CFR Part 772)

*Note*: FDOT defines that a substantial noise increase occurs when the existing noise level is predicted to be exceeded by 15 decibels or more as a result of the transportation improvement project. When this occurs, the requirement for abatement consideration will be followed.

<sup>&</sup>lt;sup>1</sup> The Leq(h) Activity Criteria values are for impact determination only, and are not a design standard for noise abatement measures. <sup>2</sup> Includes undeveloped lands permitted for this activity category.

#### 2.3 **Noise Abatement Measures**

Per federal regulation (23 CFR 772) and FDOT's *PD&E Manual*, when traffic noise associated with a proposed project is predicted to approach [i.e., 1 dB(A)] or exceed the NAC at a noise sensitive site, noise abatement must be considered. The most common and effective noise abatement measure is the construction of a noise barrier. Noise barriers reduce noise by blocking the sound path between a roadway and a noise sensitive area. To be effective, noise barriers must be long, continuous (i.e., with no intermittent openings), and have sufficient height to block the path between the noise source and the receptor site. Evaluated noise abatement measures, in the form of a noise barrier, must be considered reasonable and feasible to be recommended for design and construction.

For noise abatement measures to be recommended for further consideration in the design phase of the project, they must be determined to be both feasible and reasonable. A range of factors is used to evaluate the feasibility and reasonableness of noise abatement measures. Feasibility deals with engineering considerations, including the ability to construct a noise barrier using standard construction methods and techniques as well as the ability of the noise barrier to provide a reduction of at least 5 dB(A) to the impacted receptor sites. For example, given the topography of a particular location, can the minimum noise reduction [5 dB(A)] be achieved given certain access, drainage, utility, safety, and maintenance requirements? In addition, for a noise barrier to be considered acoustically feasible, at least two impacted receptor sites must achieve at least a 5 dB(A) reduction.

Once a noise barrier is determined to be feasible, the reasonableness of a noise barrier is evaluated. Reasonableness implies that common sense and good judgment were applied in a decision related to noise abatement. The following three reasonable factors must be collectively achieved in order for the noise barrier to be deemed reasonable: the cost effectiveness of the noise barrier, the achievement of the noise reduction design goal, and the consideration of the viewpoints of the benefited property owners and residents. To be deemed reasonable, the noise barrier needs to be below FDOT's reasonable cost criterion which is described below, must attain FDOT's noise reduction design goal of 7 dB(A) at one or more impacted receptor sites, and be supported by a majority of the benefited receptor sites.

When determining the cost reasonableness of a conceptual noise barrier design for a residential area, \$42,000 per benefited receptor is looked upon as the limit using the standard construction cost of \$30.00 per square foot. A benefited receptor site is defined as a noise sensitive site that will obtain a minimum of 5.0 dB(A) of noise reduction as a result of the construction of a noise barrier regardless of whether or not they are identified as impacted. Only benefited receptor sites are included in the calculation of reasonable cost for a noise barrier.

If the noise abatement measure has been determined to be reasonable and feasible, the viewpoint of the impacted and benefited property owners must be considered. During the Final Design phase of the project, a more detailed process is implemented to include noise abatement workshops and/or public surveys, to determine the wishes of the benefited receptor sites. Each benefited receptor, including both the owner and resident, is given the opportunity to provide input regarding their desires to have the proposed noise abatement measure constructed. The goal of this process is to obtain a response for or against the noise barrier from a majority of benefited receptors (property owners and tenants) that respond to the survey. If not supported by a majority of the survey respondents, a noise barrier or abatement measure will not be deemed reasonable.

#### 3.0 **Traffic Noise Analysis**

#### 3.1 **Predicted Noise Levels and Abatement Analysis**

The following section describes the traffic noise analysis performed and the potential impacts associated with the proposed design changes and to fulfill the prior commitments related to 1) the evaluation of new noise sensitive land uses that might be impacted by design year (2051) traffic noise associated with the project, as well as 2) the re-evaluation of previously evaluated noise sensitive land uses that might be potentially impacted by traffic noise due to design changes to the horizontal and vertical geometry of the project. In addition, it describes the analysis performed to determine if land uses and traffic noise impacts have changed along the project corridor and to determine if any additional locations warrant consideration of noise abatement measures.

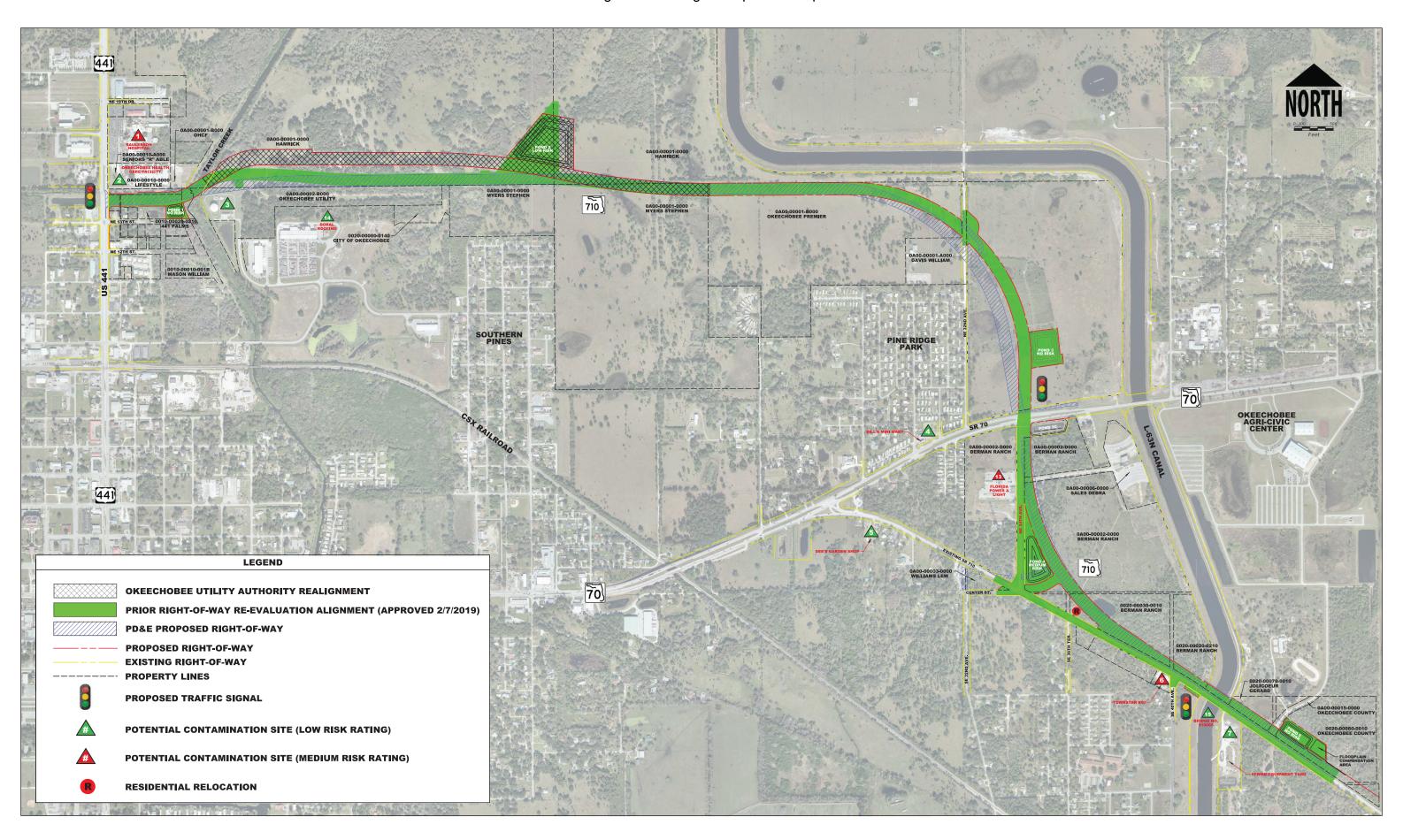
A review of the project area did not identify any additional noise sensitive sites from the 2019 Design Noise Study. A field review was conducted in January 2018 to identify all noise sensitive sites located along the corridor. Additionally, a search for Okeechobee County building permits was conducted for properties along the corridor to identify any new permitted developments that are potentially noise sensitive. No new building permits were issued after the DOPK, except for the Okeechobee Health Care Facility (see **Figure 3-2**). This facility appeared to be under construction for expansion during the most recent field review, in January 2018. Since the permit was issued prior to the DOPK, this facility was assessed for noise impacts as part of the 2019 design phase traffic noise study. Site plans for the facility expansion are provided in **Appendix C**. Multiple outdoor seating areas were identified at the Okeechobee Health Care Facility and were evaluated as noise sensitive receptor sites in this study. For informational purposes only, interior noise levels were also evaluated. FDOT policy does not require the assessment of interior noise levels when exterior noise levels are evaluated.

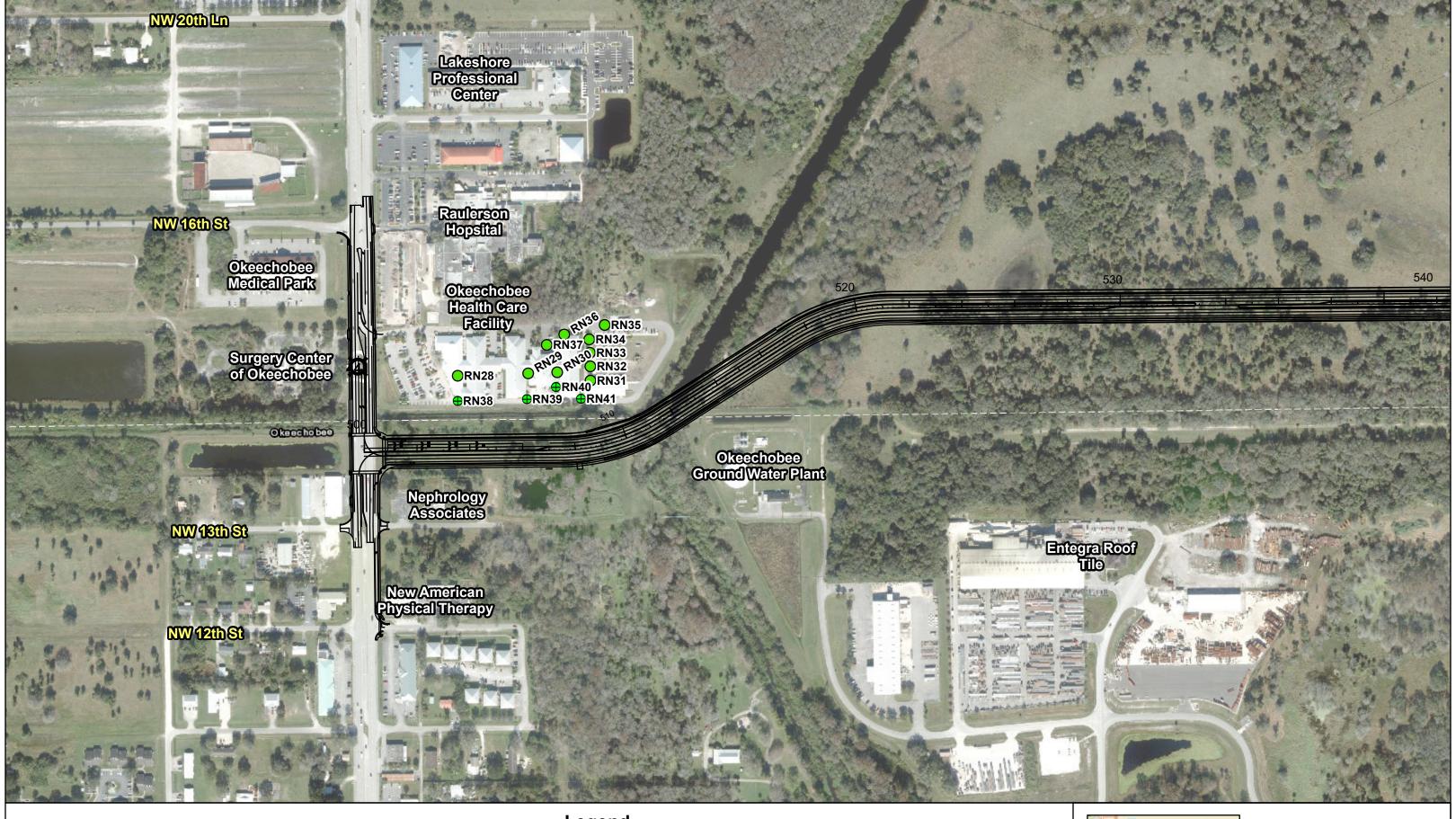
The results table, **Table 3-1**, identifies the representative noise sensitive sites assessed for traffic noise impacts, the type and number of noise sensitive sites represented, and general location. Station numbers are also provided in **Table 3-1** to facilitate locating receptor sites on **Figure 3-2**. **Table 3-1** includes the predicted future design year (2051) noise levels associated with the latest design concept. Each representative receptor site was given a unique designation (e.g., RN28) to represent the name of the noise sensitive area/site in which the receptor is located. The same noise sensitive sites evaluated in the 2019 Noise Studies were also evaluated in the latest design phase.

With the latest design concept, the predicted design year (2051) noise levels at the Okeechobee Health Care Facility exterior areas of use range from 48.8 dB(A) to 60.0 dB(A), below the NAC of 66 dB(A) for Activity Category C; therefore, are not considered impacted by the project and do

not require the consideration of noise abatement measures. Additionally, consistent with the PD&E phase noise study results, no sites are predicted to experience a substantial increase in noise levels (i.e.: increase more than 15 dB(A) over existing) as a result of the project. The predicted interior noise levels at the facility range from 34.1 dB(A) to 38.3 dB(A), below the NAC of 51 dB(A) for Activity Category D; therefore, are not considered impacted by the project and do not require the consideration of noise abatement measures. The predicted design year (2051) noise levels at the remaining single-family residences along the project corridor range from 49.0 dB(A) to 62.3 dB(A), below the NAC of 66.0 dB(A) for Activity Category B; therefore, are not considered impacted by the project and do not require the consideration of noise abatement measures.

Figure 3-1 Design Comparison Map





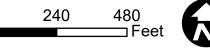


## SR 710 Extension Okeechobee County, Florida

Project Number 41934433201 Date: October 2023

### Legend

- Exterior Noise Receptor
- Interior Noise Receptor







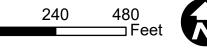


# **SR 710 Extension Okeechobee County, Florida**

Project Number 41934433201 Date: October 2023

## Legend

- Exterior Noise Receptor
- Interior Noise Receptor



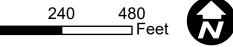






## Legend

- Exterior Noise Receptor
- Interior Noise Receptor







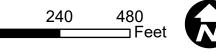


## **SR 710 Extension** Okeechobee County, Florida

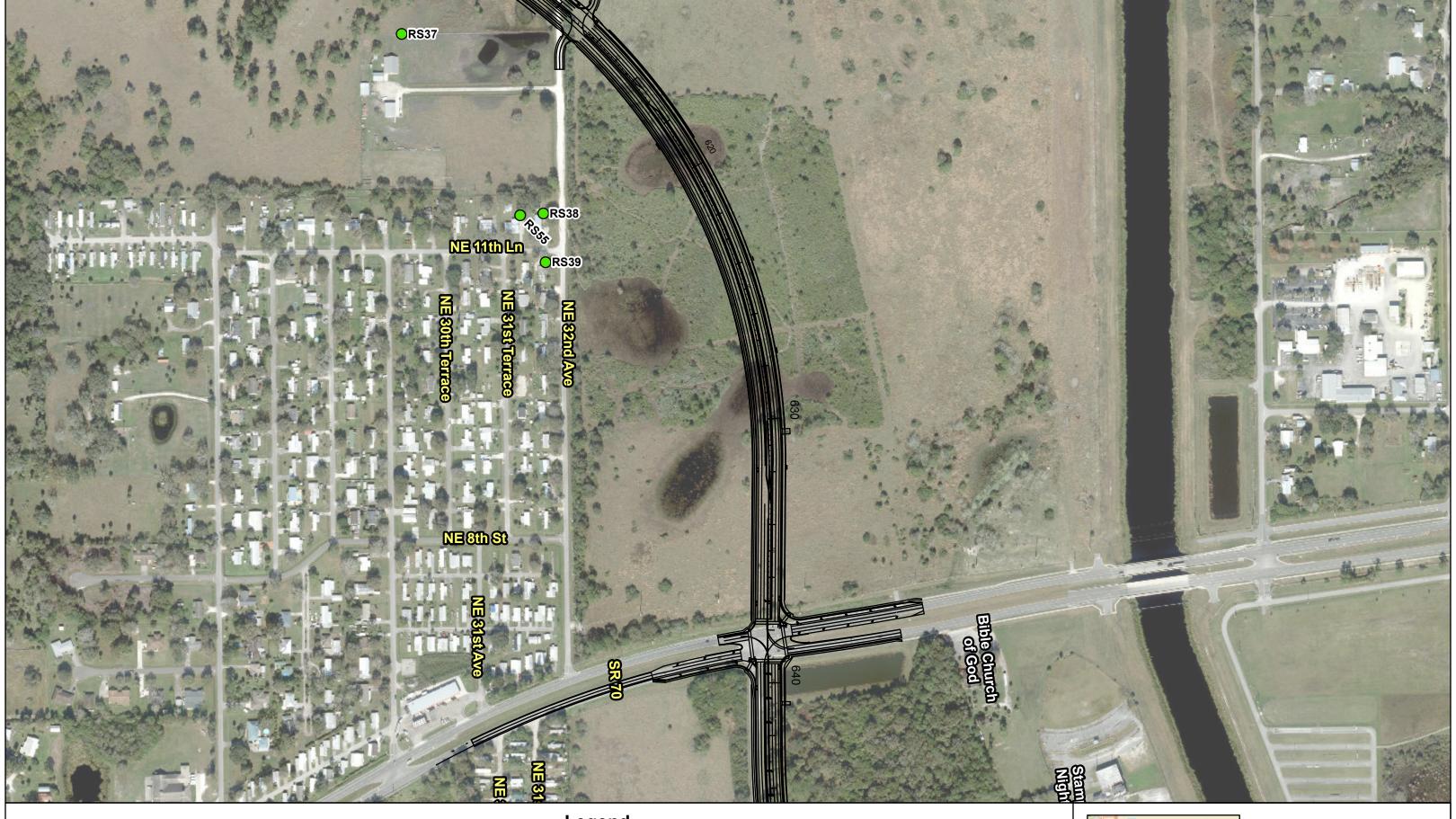
Project Number 41934433201 Date: October 2023

## Legend

- Exterior Noise Receptor
- Interior Noise Receptor



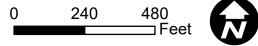




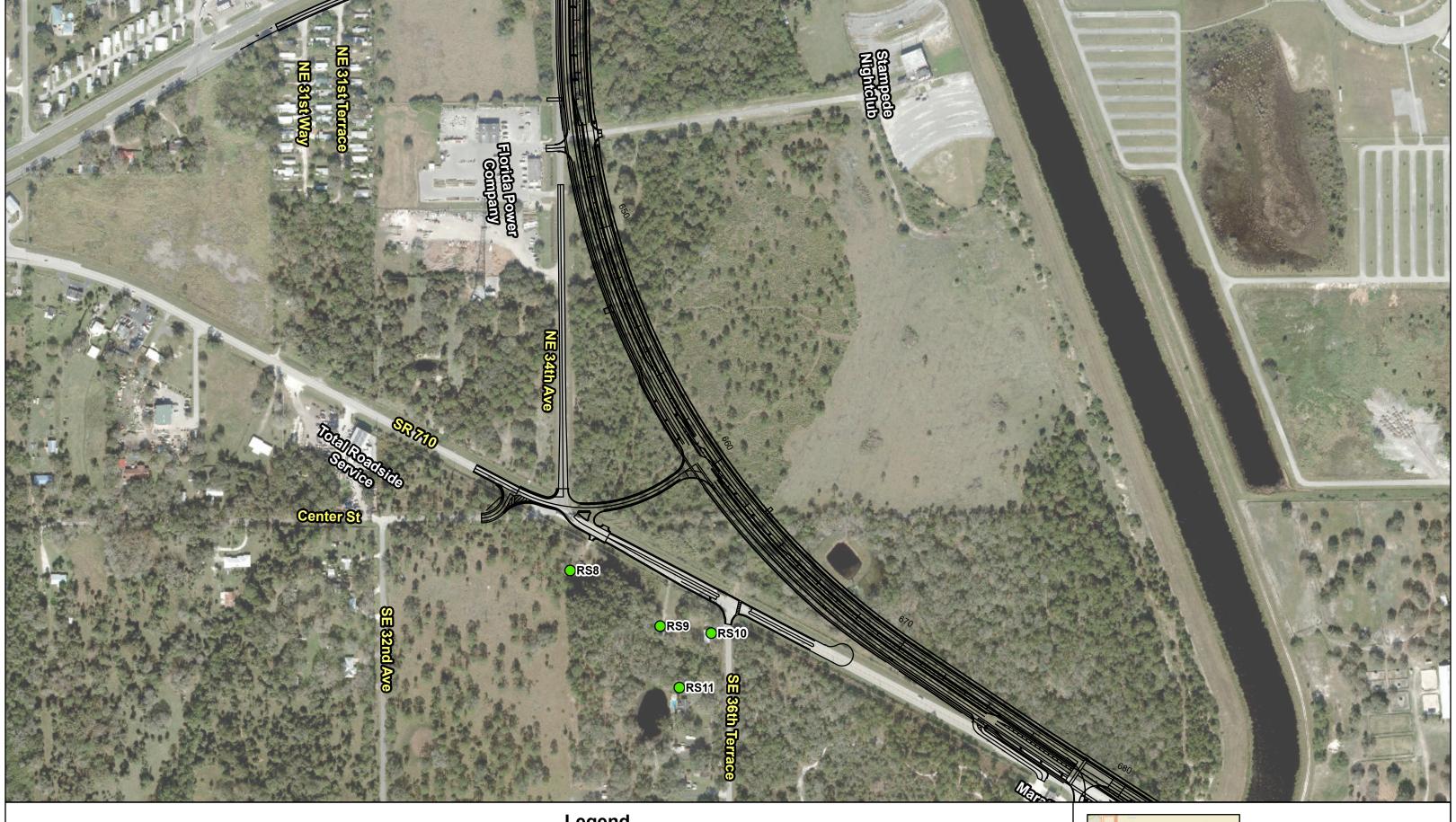


## Legend

- Exterior Noise Receptor
- Interior Noise Receptor



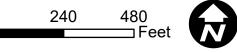






## Legend

- Exterior Noise Receptor
- Interior Noise Receptor









## Legend

- Exterior Noise Receptor
- Interior Noise Receptor

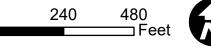




Table 3-1 Representative Noise Sensitive Receptor Sites and Noise Impact Analysis Results - Based on Revised 2023 Design Concept

			Τ		
Name of Noise Sensitive Site/Area [Noise Abatement Activity Category - FDOT's Noise Abatement Criteria Category dB(A)]	Representative Noise Receptor Site Designation	Number of Noise Sensitive Sites Represented (Description)	Approximate Station Number	TNM Predicted Design Year (2051) Noise Levels dB(A) with Proposed Roadway Design Concept	Noise Abatement Criteria Status
	RN28	1 (Medical Facility - Outdoor Area of Use)	503+00	60.0	Below
	RN29	1 (Medical Facility - Outdoor Area of Use)	506+00	50.2	Below
	RN30	1 (Medical Facility - Outdoor Area of Use)	509+00	51.6	Below
	RN31	1 (Medical Facility - Outdoor Area of Use)	509+00	58.5	Below
Okeechobee Health Care	RN32	1 (Medical Facility - Outdoor Area of Use)	509+00	56.8	Below
Facility - [NAC C - 66 dB(A)]	RN33	1 (Medical Facility - Outdoor Area of Use)	509+00	55.6	Below
	RN34	1 (Medical Facility - Outdoor Area of Use)	509+00	54.7	Below
	RN35	1 (Medical Facility - Outdoor Area of Use)	510+00	54.7	Below
	RN36	1 (Medical Facility - Outdoor Area of Use)	508+00	48.8	Below
	RN37	1 (Medical Facility - Outdoor Area of Use)	507+00	49.5	Below
		Alea of ose)	Minimum	48.8	
			Maximum	60.0	
			Average	54.0	
	Total Number of	Receptor Sites Equal to or Greater th	nan 66.0 dB(A)	0	
	RN38	1 (Medical Facility -Interior)	507+00	38.3	Below
Okeechobee Health Care	RN39	1 (Medical Facility -Interior)	509+00	37.0	Below
Facility - [NAC D - 51 dB(A)]	RN40	1 (Medical Facility -Interior)	508+00	34.1	Below
	RN41	1 (Medical Facility -Interior)	508+00	36.8	Below
			Minimum	34.1	
			Maximum	38.3	
			Average	36.6	
	Total Number of	Receptor Sites Equal to or Greater th	nan 51.0 dB(A)	0	
	RS36	1 (Single Family Residential)	558+00	49.0	Below
	RS37	1 (Single Family Residential)	607+00	55.7	Below
	RS38	1 (Single Family Residential)	618+00	53.9	Below
	RS39	1 (Single Family Residential)	622+00	52.9	Below
Single Family Residences -	RS55	1 (Single Family Residential)	618+00	53.0	Below
[NAC B - 66 dB(A)]	RS8	1 (Single Family Residential)	660+00	55.4	Below
	RS9	1 (Single Family Residential)	663+00	56.9	Below
	RS10	1 (Single Family Residential)	665+00	59.7	Below
	RS11	1 (Single Family Residential)	665+00	54.7	Below
	RS13	1 (Single Family Residential)	679+00	62.3	Below
			Minimum	49.0	
Maximum 62.3					
Average 55.4					
	Total Number of	Receptor Sites Equal to or Greater th	nan 66.0 dB(A)	0	

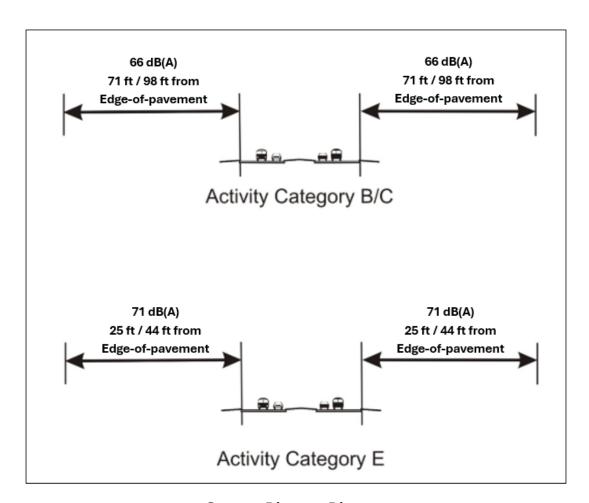
X\P\Noise\_Studies\SR 710\Report\Tables\[Table\_3-1\_Results table\_May 2024.xisx]Table3-1\_ReceiverTable

#### 3.2 **Noise Impact Contours**

To aid in promoting land use compatibility, generalized future noise impact contours for the properties in the immediate vicinity of the project have been developed for NAC B/C and E (i.e., residential, other sensitive land uses, and sensitive commercial land uses, respectively). Updated noise impact contours were developed since the proposed typical section was modified after the PD&E noise analysis. These contours represent the approximate distance from the edge of the nearest proposed travel lane of SR 710 to the limits of the area predicted to approach [i.e., within 1 dB(A)] or exceed the NAC in the design year 2051. These contours do not consider any shielding of noise provided by structures between the receptor site and the proposed travel lanes. Two typical sections are proposed along the project limits, and both were evaluated. Typical Section No. 1, proposed from Station 500+00 to Station 523+00, has a posted speed of 40 mph, while Typical Section No. 2, proposed from Station 523+00 to Station 684+01, has a posted speed of 50 mph. Both typical sections provide four 12-foot-wide travel lanes and a 30-foot-wide median. The distances between the proposed edge of the outside travel lane and the noise contour line for both typical sections are presented in **Table 3-2** and the following contour distance diagram. To minimize the potential for incompatible land use, noise sensitive land uses should be located beyond this distance.

**Table 3-2 Design Year Noise Abatement Criteria Contour Distances** 

Distance from Nearest Proposed SR 710 Travel Lane to		
Noise Contour Line		
(Typical Section No. 1/Typical Section No. 2)		
66 dB(A) – Activity Category B/C	71 dB(A) – Activity Category E	
71 ft/ 98 ft	25 ft/ 44 ft	



**Contour Distance Diagram** 

#### 4.0 **Conclusions**

A traffic noise study was performed to update the 2019 Design Noise Study, as part of the commitments made during the SR 710 PD&E study, during the final design phase of the SR 710 Extension project from US 441 to the L-63 Canal. The potential for traffic noise impacts were reevaluated to reflect the 2023 design concept and the proposed design changes. The noise study was performed in accordance with 23 CFR 772, *Procedures for Abatement of Highway Traffic Noise and Construction Noise* (July 13, 2010) and the FDOT PD&E Manual. Design year (2051) traffic noise levels for the Build Alternative are not predicted to approach or exceed the NAC at any noise sensitive sites along the project corridor. Therefore, the consideration of noise abatement measures was not warranted for any properties along the project limits. While the predicted traffic noise levels have changed due to the alignment changes, the findings presented in the PD&E NSR remain valid.

#### 5.0 **Construction Noise and Vibration**

During construction of the project, there is the potential for noise impacts to be substantially greater than impacts resulting from normal traffic operations due to the use of heavy equipment to build roadways. Vibration impacts may also be a result of project construction. The early identification of potential noise/vibration sensitive sites along the project corridor is important in minimizing noise and vibration impacts during the construction phase. A list of typical construction noise and vibration sensitive sites can be found in the FDOT PD&E Manual.

There are noise and vibration sensitive sites along the project corridor as shown in **Figure 3-2**. The majority of the noise sensitive locations represent residential areas bordering the roadway. The Okeechobee Health Care Facility is also potentially sensitive to vibration impacts. Construction noise and vibration impacts to these sites will be minimized by adherence to the controls listed in the latest edition of the FDOT *Standard Specifications for Road and Bridge Construction*.

If unanticipated noise and/or vibration issues arise during the construction process, the Project Engineer, in concert with the District Noise Specialist and the Contractor, will investigate additional methods of controlling these impacts.

#### 6.0 **References**

23 CFR Part 772, "Procedures for Abatement of Highway Traffic Noise and Construction Noise", Federal Register, Vol. 75, No. 133, Tuesday, July 13, 2010; pages 39834-39839.

Federal Highway Administration Report FHWA-HEP-10-025, "Highway Traffic Noise: Analysis and Abatement Guidance", June 2010 (revised December 2010); 76 pages.

Federal Highway Administration Report FHWA-PD-96-009, "FHWA Traffic Noise Model, Version 1.0 User's Guide", January 1998; 192 pages + supplements.

Federal Highway Administration Report FHWA-HEP-06-015, "FHWA Highway Construction Noise Handbook: Final Report". August 2006; 185 pages.

"A Method to Determine Reasonableness and Feasibility of Noise Abatement at Special Use Locations", Roger L. Wayson and John M. MacDonald, University of Central Florida; Updated July 22, 2009; 64 pp. Available from: Florida Department of Transportation, Environmental Management Office, 605 Suwannee Street, M.S. 37, Tallahassee, FL 32399-0450.

Florida Department of Transportation. "Highway Traffic Noise", Part 2, Chapter 18. Project Development and Environment Manual, Florida Department of Transportation, Tallahassee, July 1, 2023.

Florida Department of Transportation Plans Preparation (Topic No. 625-000-007) Manual Volume 1, Chapter 32, "Noise Walls and Perimeter Walls", January 2017; 16 pages.

Florida Department of Transportation "Standard Specifications for Road and Bridge Construction," FY 2024-25.

# **APPENDIX A Approved Traffic Data for Noise Analysis**

## TRAFFIC DATA FOR NOISE STUDIES - SUMMARY OUTPUT FDOT DISTRICT 1

419344-2-22-01

Federal Aid Number(s):

FPID Number(s):

Dood Names	CD 7	SR 710	
Road Name: Project Description:	SR / .	10 Extension	
Segment Description:	SR 710 East of US 441	(STA 500+00 to STA 523+00)	
Section Number:		<u> </u>	
Mile Post To/From:	(new	v alignment)	
existing Facility:		D = <b>58.10%</b> %	
Voor.	2024	T24 = <b>22.69%</b> % of 24 Hour Volume	
ear:	2024	Tpeak = 11.34% % of Design Hour Volume  MT = 4.51% % of Design Hour Volume	
OS C Peak Hour Directional Volume:	0	HT = 6.55% % of Design Hour Volume	
emand Peak Hour Volume:	1	B = <b>0.28%</b> % of Design Hour Volume	
Posted Speed:	0	MC = <b>0.30%</b> % of Design Hour Volume	
No Build Alternative (Design Year):		D = <b>58.10%</b> %	
	2054	T24 = <b>22.69%</b> % of 24 Hour Volume	
ear:	2051	Tpeak = 11.34% % of Design Hour Volume  MT = 4.51% % of Design Hour Volume	
OS C Peak Hour Directional Volume:	0	HT = <b>6.55%</b> % of Design Hour Volume	
Demand Peak Hour Volume:	1	B = 0.28% % of Design Hour Volume	
Posted Speed:	0	MC = <b>0.30%</b> % of Design Hour Volume	
Build Alternative (Design Year):	2051	D = <b>58.10%</b> %	
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Federal Aid Number(s):			
FPID Number(s):	419344-2-22-01		
State/Federal Route No.:	10071000		
Road Name:	SR 710 Extension		
Project Description:	Build		
	50110		
Segment Description:	US 441 to SR 70		
Section Number:	1		
Mile Post To/From:	(new alignment)		
	(new anginating)		
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State/Federal Route No.:			_	
Road Name:	SR 710 Exter	nsion	-	
Project Description:	Bulld		_	
Segment Description:	SR 70 to Old S	iR 710		
Section Number:	1		_	
Mile Post To/From:	(new alignm	ent)	_	
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Existing Facility:		D=	55.00% %	
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FPID Number(s):	419344-2-22-01		
State/Federal Route No.:			
Road Name:	SR 710 Extension		
Project Description:	Build		
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Segment Description:	Old SR 710 to SE 40th Ave		
Section Number:	1		
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FPID Number(s):	419344-2-22-01	
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Road Name:	SR 710 Extension	
Project Description:	Build	
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Federal Aid Number(s):				
FPID Number(s):	419344-2-2	2-01	_	
State/Federal Route No.:			_	
Road Name:	SR 710 Exter	rsion	-	
Project Description:	Build			
Segment Description:	SR 70 East of S	5R 710		
Section Number:	1		-	
Mile Post To/From:	(new alignm	ent)	_	
Existing Facility:		D=	55.00%	10
		T24 =	13.00%	% of 24 Hour Volume
Year:	2013	Tpeak =	6.50%	% of Design Hour Volume
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# TRAFFIC DATA FOR NOISE STUDIES - SUMMARY OUTPUT FDOT DISTRICT 1

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State/Federal Route No.: Road Name:			_	
Project Description:	SR 710 Exten	sion		
Project Description:	Build		_	
Segment Description:	Old SR 710 East of N	E 34th Ave		
Section Number:	1		-	
Mile Post To/From:	(new alignme	ent)	_	
Existing Facility:		D=		%
Year:	2013	T24 =		% of 24 Hour Volume
	2013	Tpeak =		% of Design Hour Volume
LOS C Peak Hour Directional Volume:		MT =		% of Design Hour Volume
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FPID Number(s):	419344-2	2-22-01		
State/Federal Route No.:			_	
Road Name:	SR 710 Ex	tension	_	
Project Description:	Buil	đ	_	
Segment Description:	US 441 North o	f SR 710 Ext.		
Section Number:	1		_	
Mile Post To/From:	(new align	ment)	_	
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Year:	2013	Tpeak =	4.86%	
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	0	Tpeak =	0.00%	% of Design Hour Volume
LOS C Peak Hour Directional Volume:		MT=	0.00%	% of Design Hour Volume
	0	HT=	0.00%	% of Design Hour Volume
Demand Peak Hour Volume: Posted Speed:	0	B=	0.00%	% of Design Hour Volume
rosteu speed:	0	MC=	0.00%	% of Design Hour Volume
	AND			
Euild Alternative (Design Year):		Da	55.00%	96
	Marie Control of the	T24 =	9.72%	% of 24 Hour Volume
Year	2040	Tpeak=		% of Design Hour Volume
		MT=		% of Design Hour Volume
OS C Peak Hour Directional Volume:	1740	HT=		% of Design Hour Volume
Demand Peak Hour Volume:	1311	B=	0.31%	% of Design Hour Volume
Posted Speed:	35	MC=		% of Design Hour Volume
certify that the above information is ac		use with the traffic noise and	lysis.	
Prepared By: Sou K	Myztrick a	Sau Into	1	Date: 04/25/2018
Prin	it Name	Signature		- 1 and annu
have reviewed and concur that the abo	7	te for use with the traffic noi	se analysis	
DOT Reviewer:	0111111	AM		Date: 6/18/298
Prin	t Name	Signature		

# TRAFFIC DATA FOR NOISE STUDIES - SUMMARY OUTPUT FOOT DISTRICT 1

419344-2-22	-01	_	
		_	
SR 710 Extens	sion	_	
Build		_	
US 441 South of SR	710 Fyt		
1	720 LAL	-	
(new alignment	nt)	-	
		_	
	0-		1
	_	-	
2013			% of 24 Hour Volume % of Design Hour Volume
	•		% of Design Hour Volume
0			% of Design Hour Volume
1	B =	0.24%	% of Design Hour Volume
35	MC =	0.51%	% of Design Hour Volume
	D=	0.00%	1%
	T24 =	0.00%	% of 24 Hour Volume
0	Tpeak =	0.00%	% of Design Hour Volume
	MT=	0.00%	% of Design Hour Volume
0	HT=	0.00%	% of Design Hour Volume
	8=	0.00%	% of Design Hour Volume
0	MC=	0.00%	% of Design Hour Volume
	D=		%
F 2000			% of 24 Hour Volume
2040			% of Design Hour Volume
1700			% of Design Hour Volume
		2.29%	% of Design Hour Volume
		0.24%	% of Design Hour Volume
33	MC= L	0.51%	% of Design Hour Volume
	US 441 South of SR 1 (new alignme) 2013 0 1 35	Build  US 441 South of SR 710 Ext.  1 (new alignment)  D = T24 = Tpeak = MT = HT = B = MC =  D = T24 = Tpeak = MT = HT = HT = B = MC =  D = T24 = Tpeak = MT = HT = HT = B = MC =  D = T24 = Tpeak = MT = HT = HT = B = MC =  D = T24 = Tpeak = MT = HT	Build  US 441 South of SR 710 Ext.  1 (new alignment)  D =

# TRAFFIC DATA FOR NOISE STUDIES - SUMMARY OUTPUT FOOT DISTRICT 1

Federal Aid Number(s):			_		
FPID Number(s):  State/Federal Route No.:	419344-2-22	2-01	_		
Road Name:	SR 710 Exten	alea.	_		
Project Description:	Build	ISION	_		
	Duild		_		
Segment Description:	SE 40th Ave (South of	F SR 710 Ext.)	_		
Section Number:	1				
Mile Post To/From:	(new alignme	ent)	_		
				<u></u>	
Existing Facility:		D=	EE 00W	7.	
		T24 =	55.00% 16.04%	% % of 24 Hour Volume	
Year:	2013	Tpeak =	8.02%	% of Design Hour Volume	
		MT =	2.23%	% of Design Hour Volume	
LOS C Peak Hour Directional Volume:	0	HT =	5.65%	% of Design Hour Volume	
Demand Peak Hour Volume:	1	B =	0.15%	% of Design Hour Volume	
Posted Speed:	35	MC =	1.20%	% of Design Hour Volume	
			LUNG		
No Build Alternative (Design Year):		D=	0.00%	%	od II.
Year:		T24 =	0.00%	% of 24 Hour Volume	
rear:	0	Tpeak =	0.00%	% of Design Hour Volume	
LOS C Peak Hour Directional Volume:		MT≖	0.00%	% of Design Hour Volume	
Demand Peak Hour Volume:	0	HT =	0.00%	% of Design Hour Volume	
Posted Speed:	0	B =	0.00%	% of Design Hour Volume	
		MC =	0.00%	% of Design Hour Volume	
Build Alternative (Design Year):		D=	55.00%	lx	
		T24 =	16.04%	% of 24 Hour Volume	
Year:	2040	Tpeak =	8.02%	% of Design Hour Volume	100
		MT=	2.23%	% of Design Hour Volume	200
LOS C Peak Hour Directional Volume:	312	HI=	5.65%	% of Design Hour Volume	100
Demand Peak Hour Volume:	493	B=	0.15%	% of Design Hour Volume	
Posted Speed:	35	MC =	1.20%	% of Design Hour Volume	
I certify that the above information is a	occurate and appropriate for a	no wish the traffic actions			
The state of the s	courate and appropriate for u	se with the traffic noise an	alvais		
Prepared By:	Hoatricle 6.	The Sou	$\omega$	Date: 06/15/2018	
· · · · · · · · · · · · · · · · · · ·	int Name	Signature		50/13/2018	
I have reviewed and concur that the ab	ove information is appropriate	o for use with the traffic or	nica analuri	ic .	
0 10 101 0	h. /.	A.A.	vine attacks	" Islanda	10/
FDOT Reviewer:	nu Jimpron	( TXW		Date: 4/18/20	18
Pri	nt Name	Signature			* U
		7			

# **APPENDIX B Referenced Roadway Design Plans 2023**

### CONTRACT PLANS COMPONENTS

ROADWAY PLANS SIGNING AND PAVEMENT MARKING PLANS SIGNALIZATION PLANS LIGHTING PLANS STRUCTURE PLANS

INDEX OF ROADWAY PLANS

SHEET DESCRIPTION

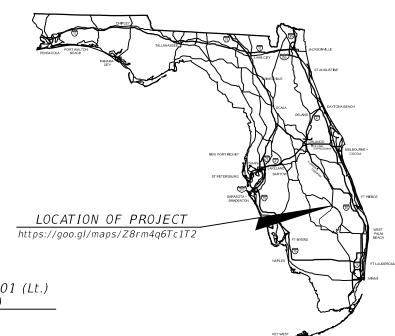
# STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

# CONTRACT PLANS

## FINANCIAL PROJECT ID 419344-3-52-01

# OKEECHOBEE COUNTY (91060)

STATE ROAD NO. 710



KEY SHEET SIGNATURE SHEET SUMMARY OF PAY ITEMS 4-12 DRAINAGE MAP PROPOSED DRAINAGE STRUCTURES 13 14 EXISTING DRAINAGE STRUCTURES TYPICAL SECTION 15-28 TYPICAL SECTION DETAILS 29-32 33-34 SUMMARY OF DRAINAGE STRUCTURES 35 OPTIONAL MATERIALS TABULATION 36-38 PROJECT LAYOUT 39 GENERAL NOTES 40-76 PLAN AND PROFILE 77-93 ROADWAY PLAN 94-98 SIDE STREET PROFILES 99-105 SPECIAL DETAILS INTERSECTION DETAIL 106-117 DRAINAGE STRUCTURES 118-210 DRAINAGE DETAILS 211-215 OUTFALL DETAIL 216 217-221 POND DETAILS 222-372 CROSS SECTIONS 373-380 DRIVEWAY CROSS SECTIONS

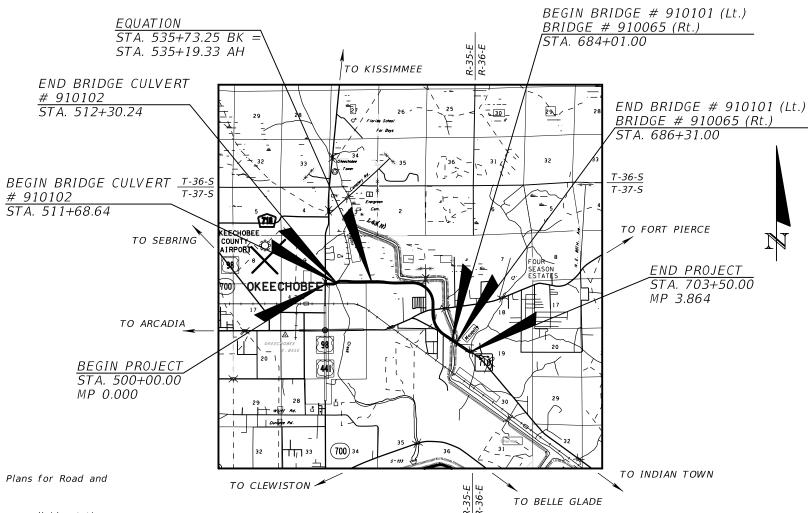
POND CROSS SECTIONS 381-400

404-424 EROSION CONTROL PLAN

425-503 504-522 UTILITY ADJUSTMENTS SQ-1 - SQ-9 SUMMARY OF QUANTITIES ROADWAY SOILS SURVEY GR-1CTL-1 - CTL-5 PROJECT CONTROL

401-403

STORMWATER POLLUTION PREVENTION PLAN TEMPORARY TRAFFIC CONTROL PLAN



# ROADWAY PLANS ENGINEER OF RECORD:

GEORGE E. KNOX, P.E. P.E. NO.: 82283 WANTMAN GROUP, INC. 2910 MAGUIRE ROAD, SUITE 2008 OCOEE, FLORIDA 34761 (407) 581-1221 PHONE (407) 581-1222 FAX VENDOR NO. 65-0271367 CONTRACT NO. C-9643 CERTIFICATE OF AUTHORIZATION NO. 6091

## FDOT PROJECT MANAGER: MICHAEL JOHNSON, P.E.

CONSTRUCTION	FISCAL	SHEET
CONTRACT NO.	YEAR	NO.
	21	1

### GOVERNING STANDARD PLANS:

Florida Department of Transportation, FY 2018-19 Standard Plans for Road and Bridge Construction and applicable Interim Revisions (IRs).

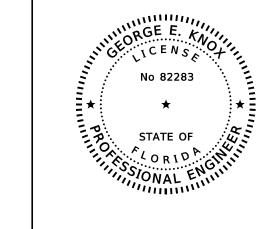
Standard Plans for Road Construction and associated IRs are available at the following website: http://www.fdot.gov/design/standardplans

APPLICABLE IRs: IR546-010-01

Standard Plans for Bridge Construction are included in the Structures Plans

#### GOVERNING STANDARD SPECIFICATIONS:

Florida Department of Transportation, July 2018 Standard Specifications for Road and Bridge Construction at the following website: http://www.fdot.gov/programmanagement/Implemented/SpecBooks



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY

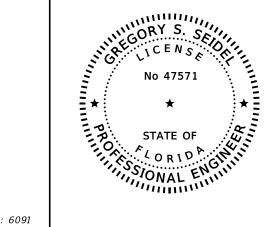
ON THE DATE ADJACENT TO THE SEAL.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

WANTMAN GROUP, INC. 2910 MAGUIRE ROAD, SUITE 2008 OCOEE, FL 34761 CERTIFICATE OF AUTHORIZATION: 6091 GEORGE E. KNOX, P.E. NO. 82283

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

SHEET NO.	SHEET DESCRIPTION
1	KEY SHEET
2	SIGNATURE SHEET
3	SUMMARY OF PAY ITEMS
15-28	TYPICAL SECTION
29-32	TYPICAL SECTION DETAILS
36-38	PROJECT LAYOUT
39	GENERAL NOTES
40-76	PLAN AND PROFILE
77-93	ROADWAY PLAN
94-98	SIDE STREET PROFILES
99-105	SPECIAL DETAILS
106-117	INTERSECTION DETAIL
222-372	CROSS SECTIONS
<i>373-380</i>	DRIVEWAY CROSS SECTIONS
425-503	TEMPORARY TRAFFIC CONTROL PLAN
504-522	UTILITY ADJUSTMENTS
SQ-1 - SQ-9	SUMMARY OF QUANTITIES
CTL-1 - CTL-5	PROJECT CONTROL



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY

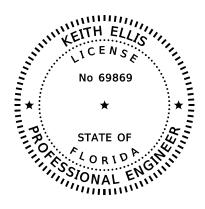
ON THE DATE ADJACENT TO THE SEAL.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

THE BALMORAL GROUP
165 LINCOLN AVENUE
WINTER PARK, FL 32789
CERTIFICATE OF AUTHORIZATION: 26123
GREGORY S. SEIDEL, P.E. NO. 47571

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

SHEET NO.	SHEET DESCRIPTION
2	SIGNATURE SHEET
4-12	DRAINAGE MAP
13	PROPOSED DRAINAGE STRUCTURES
14	EXISTING DRAINAGE STRUCTURES
216	OUTFALL DETAIL
217-221	POND DETAILS
381-400	POND CROSS SECTIONS



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY

ON THE DATE ADJACENT TO THE SEAL.

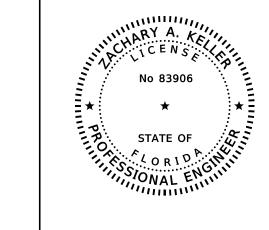
PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

FLA. DEPT. OF TRANSPORTATION MATERIALS OFFICE DISTRICT I 801 N. BROADWAY AVE. BARTOW, FL 33830 KEITH ELLIS, P.E. NO. 69869

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

SHEET NO.	SHEET DESCRIPTION
2	CICNATURE CHEET

2 SIGNATURE SHEET
GR-1 ROADWAY SOILS SURVEY



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY

ON THE DATE ADJACENT TO THE SEAL.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

WANTMAN GROUP, INC. 3111 W DOCTOR M.L.K. Jr BLVD #375 TAMPA, FL 33607 CERTIFICATE OF AUTHORIZATION: 6091 ANDREW D. ROBB, P.E. NO. 57577

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

OLLOWING	STEETS IN THE COMPTITUEE WITH MOLE OF STEET
SHEET NO.	SHEET DESCRIPTION
2	SIGNATURE SHEET
33-34	SUMMARY OF DRAINAGE STRUCTURES
35	OPTIONAL MATERIALS TABULATION
118-210	DRAINAGE STRUCTURES
211-215	DRAINAGE DETAILS
401-403	STORMWATER POLLUTION PREVENTION PL
404-424	EROSION CONTROL PLAN

'ISIONS		050005 5 44404 0 5
DATE	DESCRIPTION	GEORGE E. KNOX, P.E.
		LICENSE NUMBER: 82283
		WGI, INC.
		800 N. MAGNOLIA AVE., SUITE 1750
		ORLANDO, FL 32803
	VISIONS DATE	

STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION				
ROAD NO.	COUNTY	FINANCIAL PROJECT ID		
SR 710 OKEECHOBEE		419344-3-52-01		

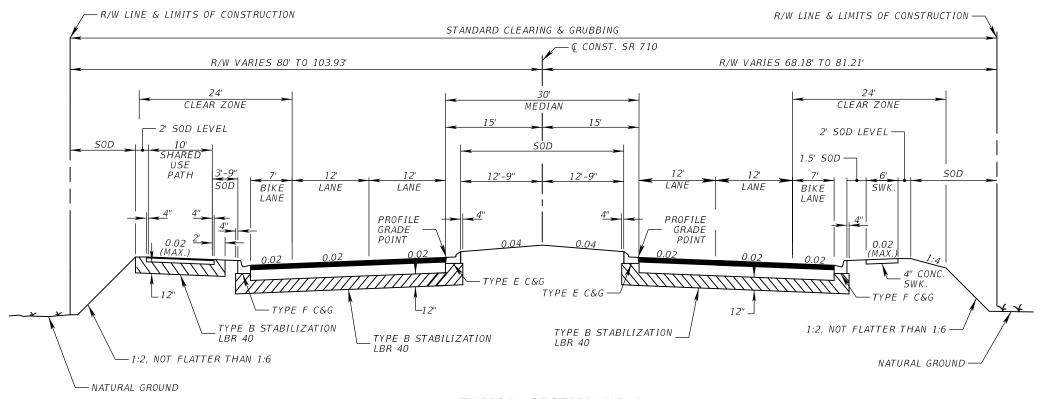
SIGNATURE SHEET (01)

SHEET NO.

IISER: George Knov

4/18/2023

c:\pwworking\wgi\george\d0586064\SIGNRD01.DGN



TYPICAL SECTION NO. 1 SR 710 STA. 500+00.00 TO STA. 523+00.00

## NEW CONSTRUCTION

OPTIONAL BASE GROUP 11 WITH

TYPE SP STRUCTURAL COURSE (TRAFFIC C) (3")

AND FRICTION COURSE FC-9.5 (1") PG 76-22

## TRAFFIC DATA

CURRENT YEAR = 2018 AADT = 8,700ESTIMATED OPENING YEAR = 2023 AADT = 10,000ESTIMATED DESIGN YEAR = 2038 AADT = 14,190 K = 9.0% D = 56.2% T = 20.9% (24 HOUR) DESIGN HOUR T = 10.5%DESIGN SPEED = 40 MPH

## SHARED USE PATH

OPTIONAL BASE GROUP 1 WITH TYPE SP STRUCTURAL COURSE (TRAFFIC A) (1  $\frac{1}{2}$ ")

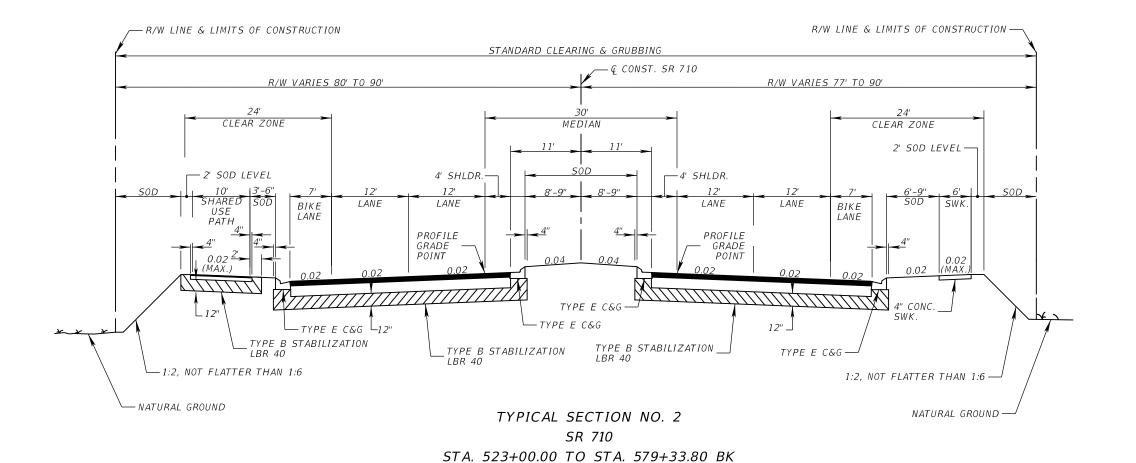
	DE	'ISIONS		T	Ī		
DATE				GEORGE E. KNOX, P.E. LICENSE NUMBER: 82283	STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION		
				WGI, INC.	ROAD NO.	COUNTY	FINANCIAL PROJECT ID
				800 N. MAGNOLIA AVE., SUITE 1750 ORLANDO, FL 32803	SR 710	OKEECHOBEE	419344-3-52-01

TYPICAL SECTION (01)

SHEET NO.

USER: George.Knox

2023 9:5



## NEW CONSTRUCTION

STA. 578+01.32 AH TO STA. 684+01.00

OPTIONAL BASE GROUP 11 WITH

TYPE SP STRUCTURAL COURSE (TRAFFIC C) (3")

AND FRICTION COURSE FC-9.5 (1") PG 76-22

## TRAFFIC DATA

CURRENT YEAR = 2018 AADT = 8,700ESTIMATED OPENING YEAR = 2023 AADT = 10,000ESTIMATED DESIGN YEAR = 2038 AADT = 14,190 K = 9.0% D = 56.2% T = 20.9% (24 HOUR) DESIGN HOUR T = 10.5%DESIGN SPEED = 50 MPH

### SHARED USE PATH

OPTIONAL BASE GROUP 1 WITH TYPE SP STRUCTURAL COURSE (TRAFFIC A) (1  $\frac{1}{2}$ ")

	RF\	/ISIONS			1	am.imp. op. p	T OPER 4
DATE	DESCRIPTION	DATE	DESCRIPTION	GEORGE E. KNOX, P.E. LICENSE NUMBER: 82283	DEP.	STATE OF FLORIDA DEPARTMENT OF TRANSPORT	
				WGI, INC.	ROAD NO.	COUNTY	FINANCIAL PROJECT ID
				800 N. MAGNOLIA AVE., SUITE 1750 ORLANDO, FL 32803	SR 710	OKEECHOBEE	419344-3-52-01

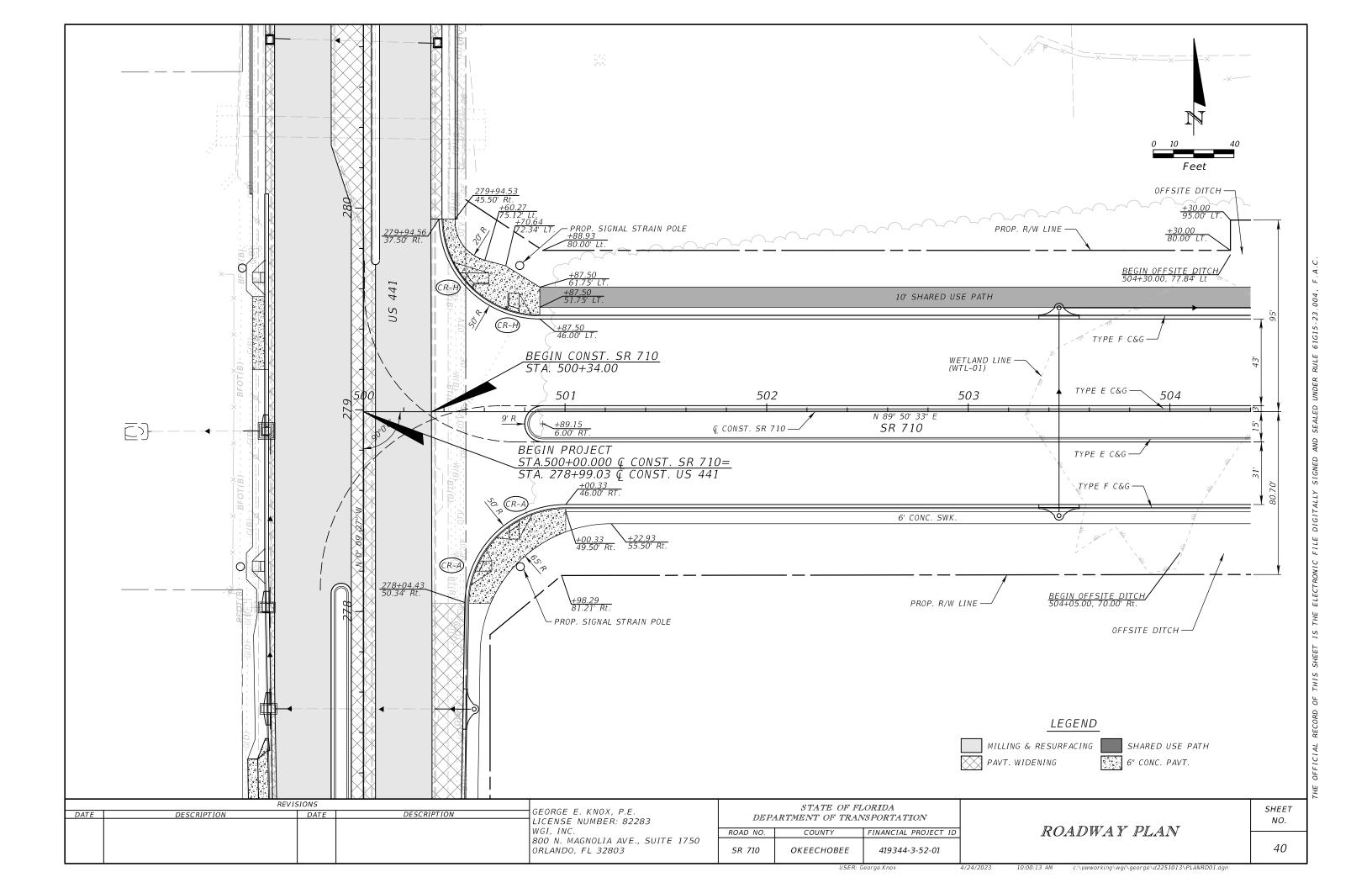
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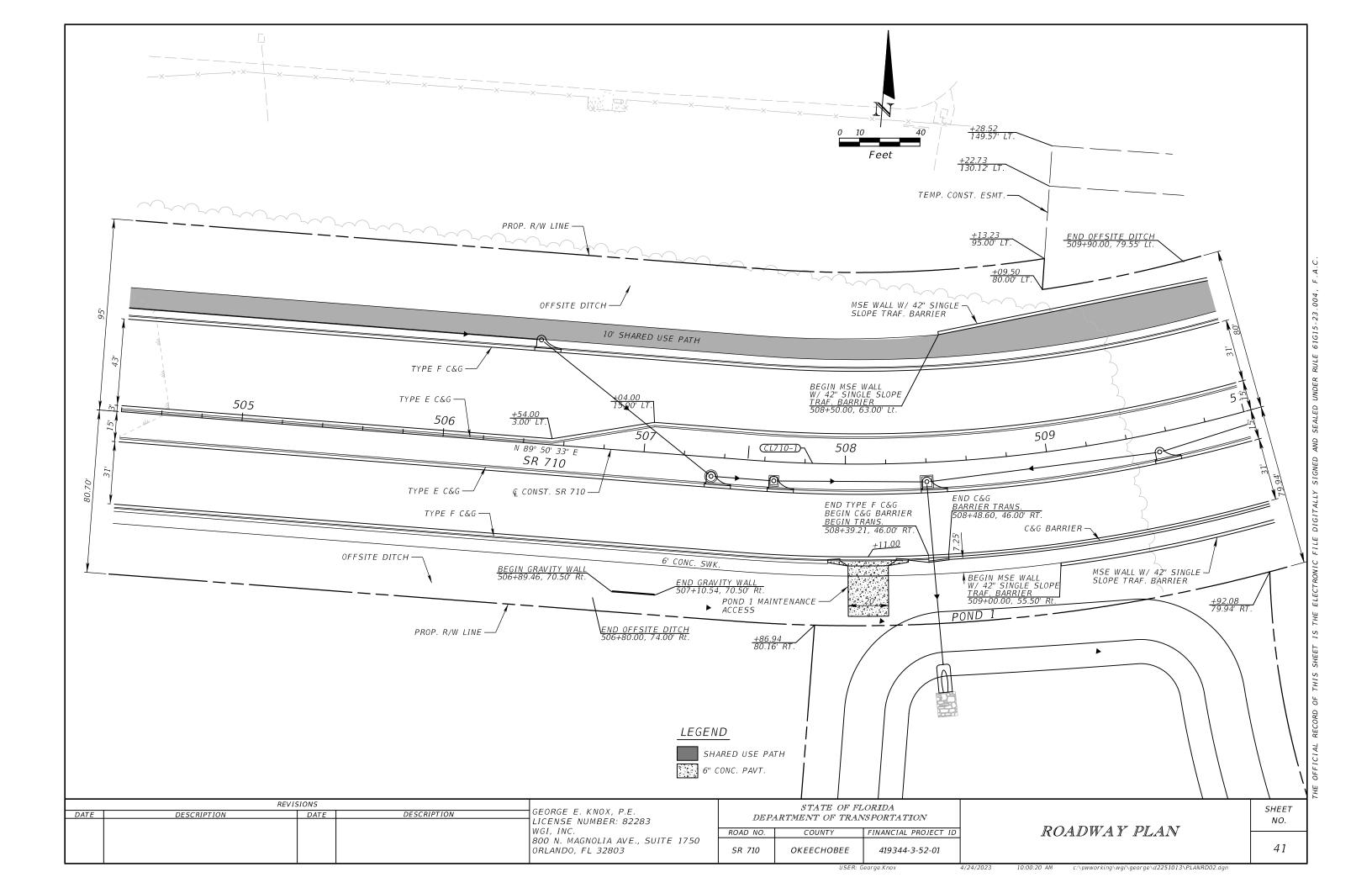
SHEET NO.

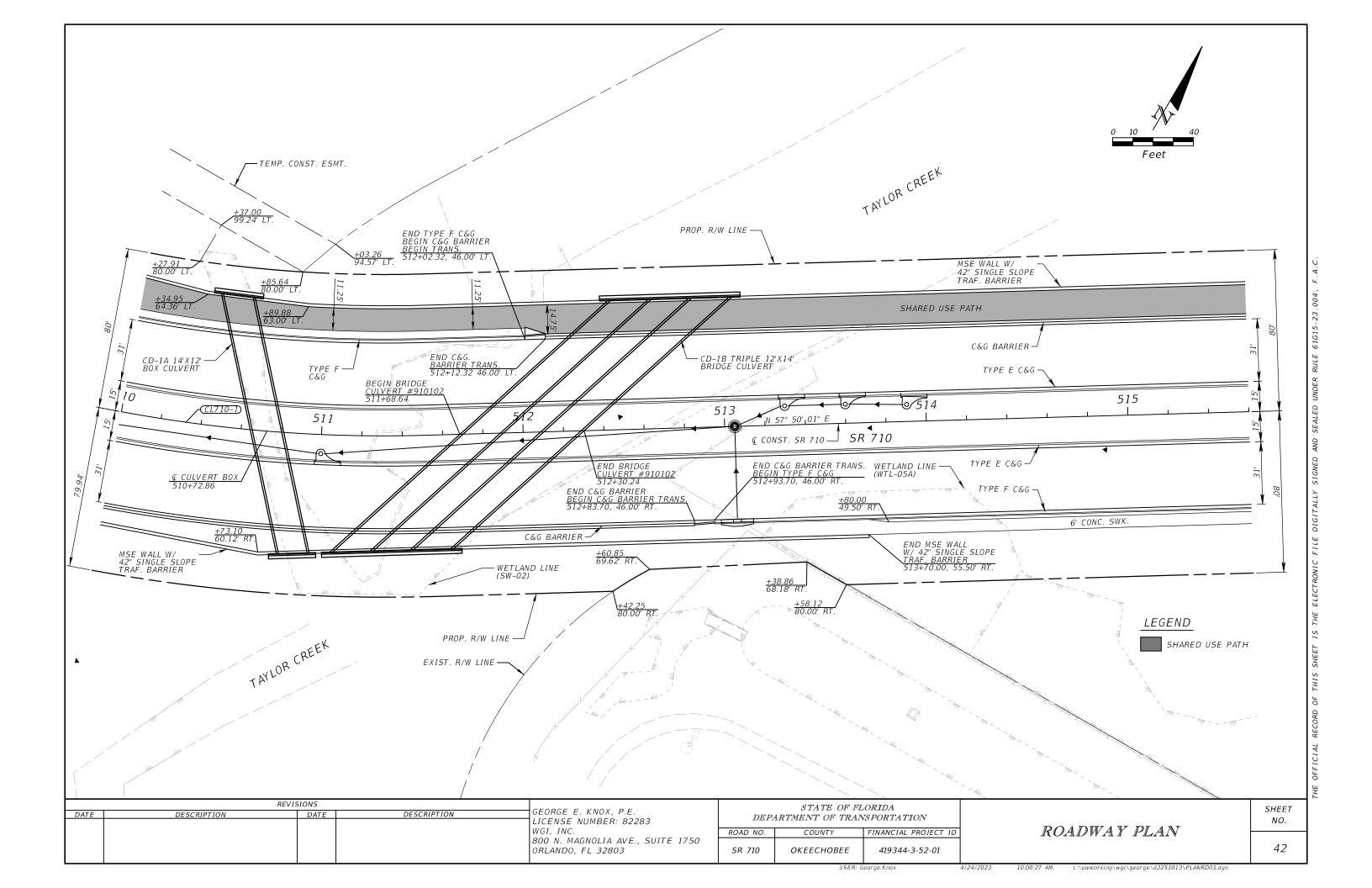
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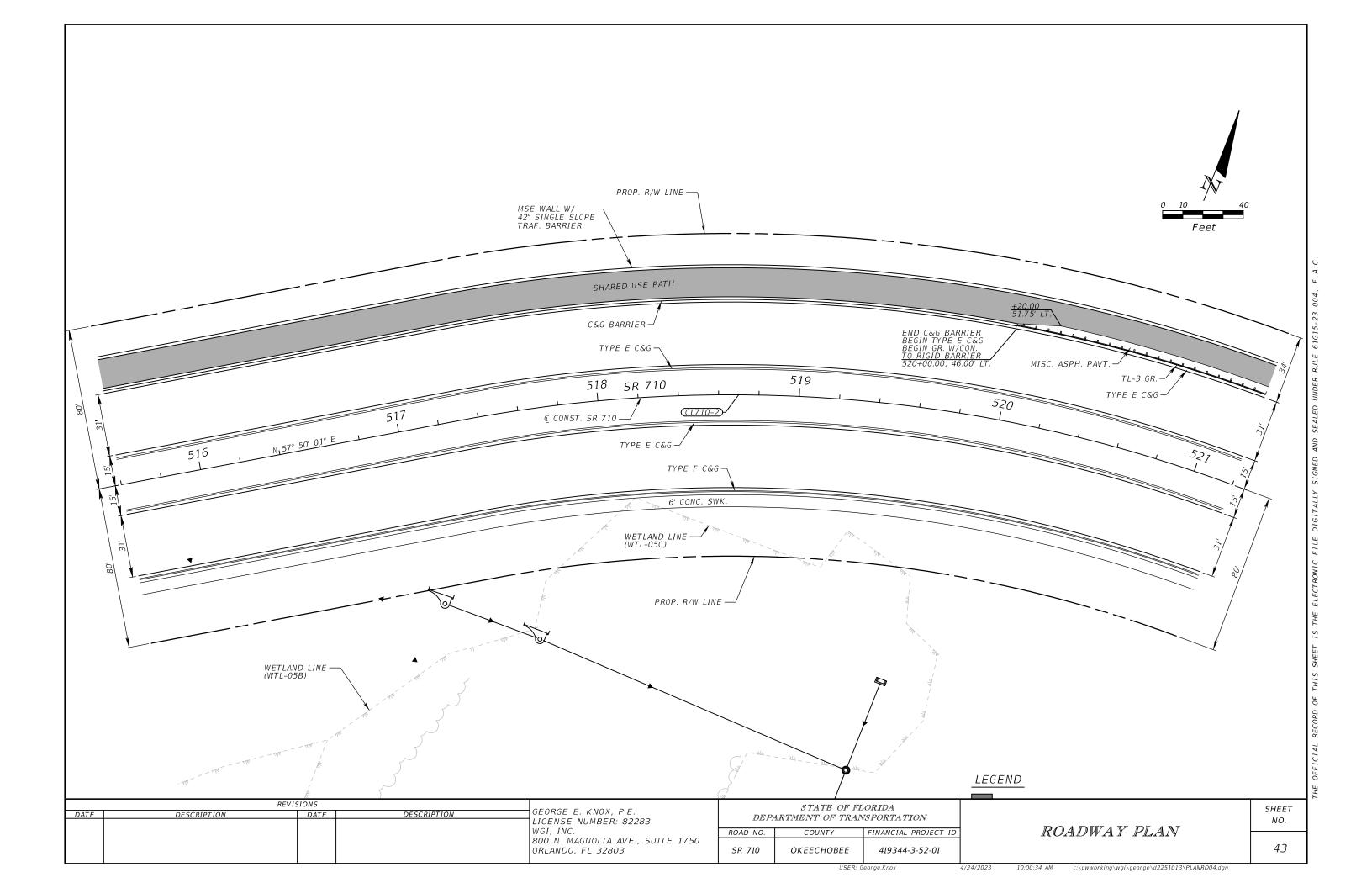
1/24/2023

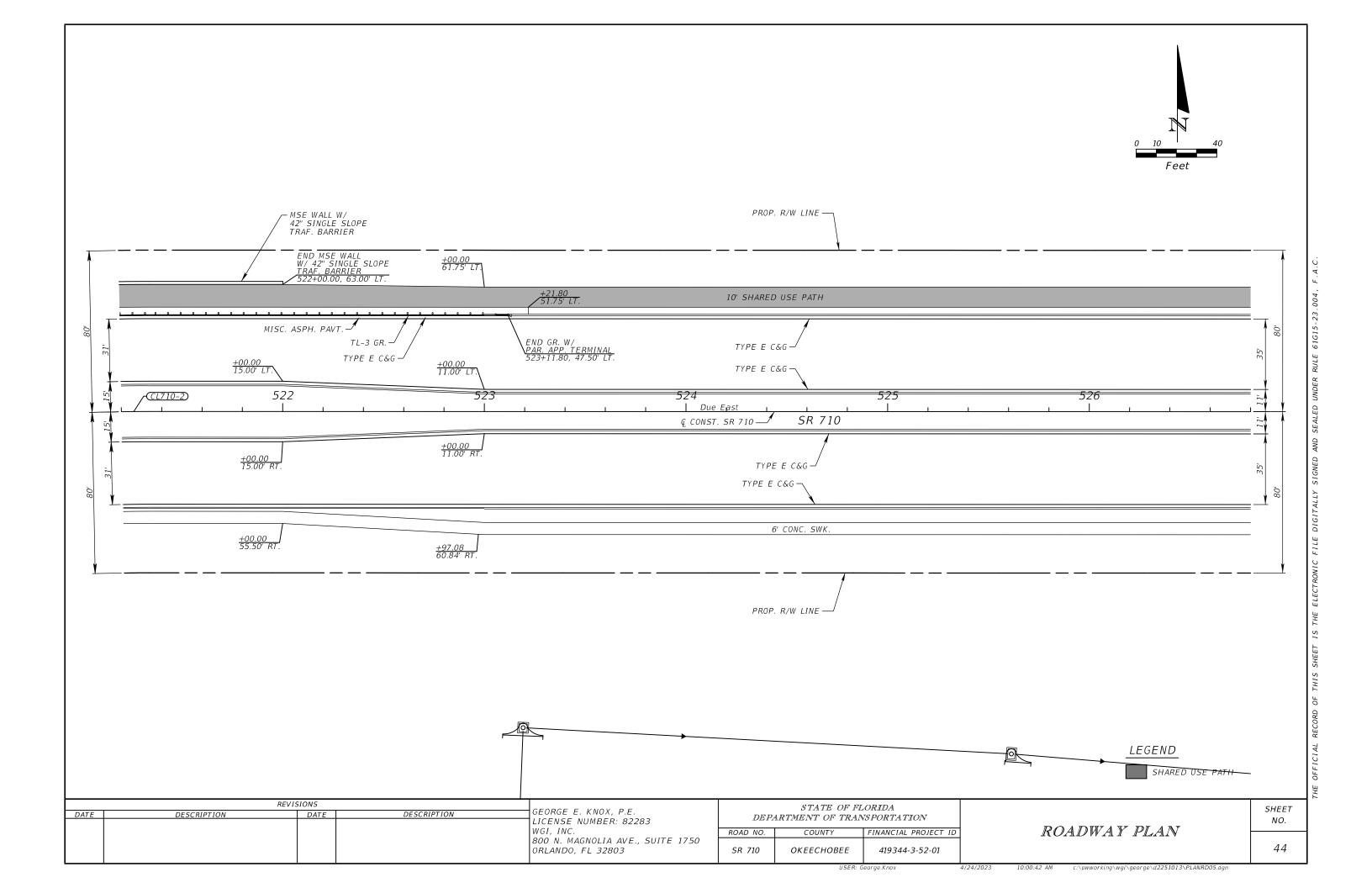
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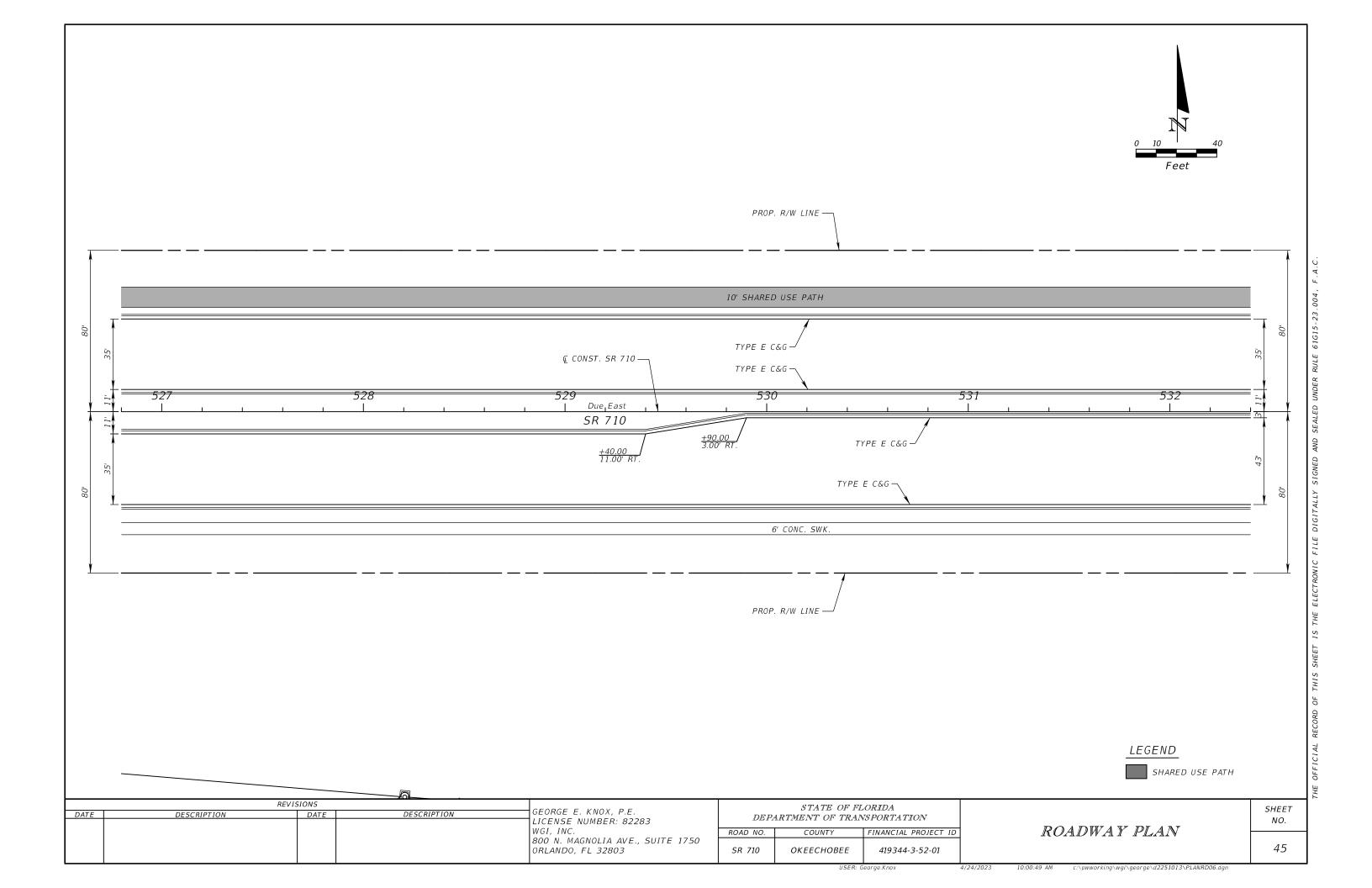


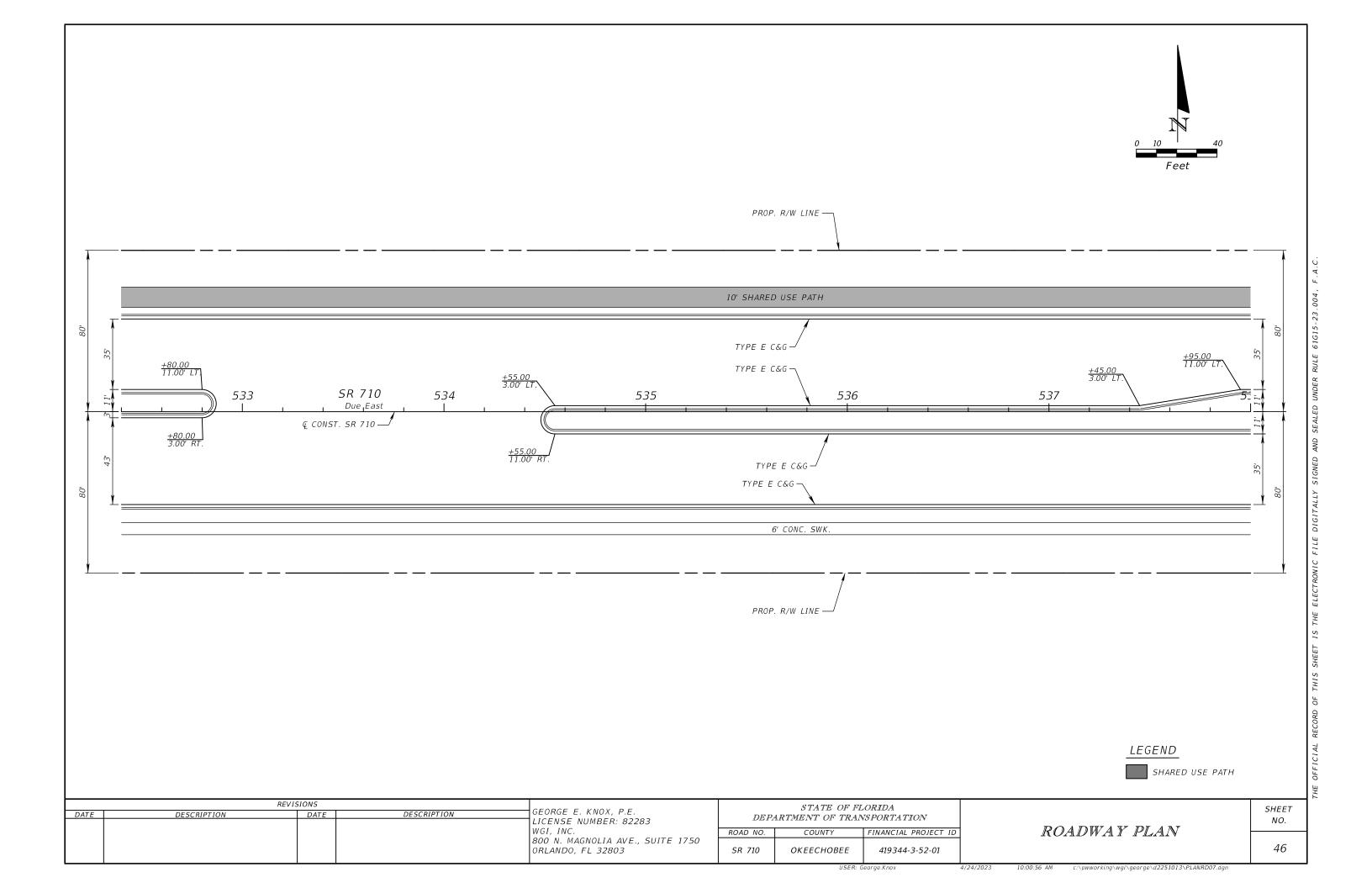


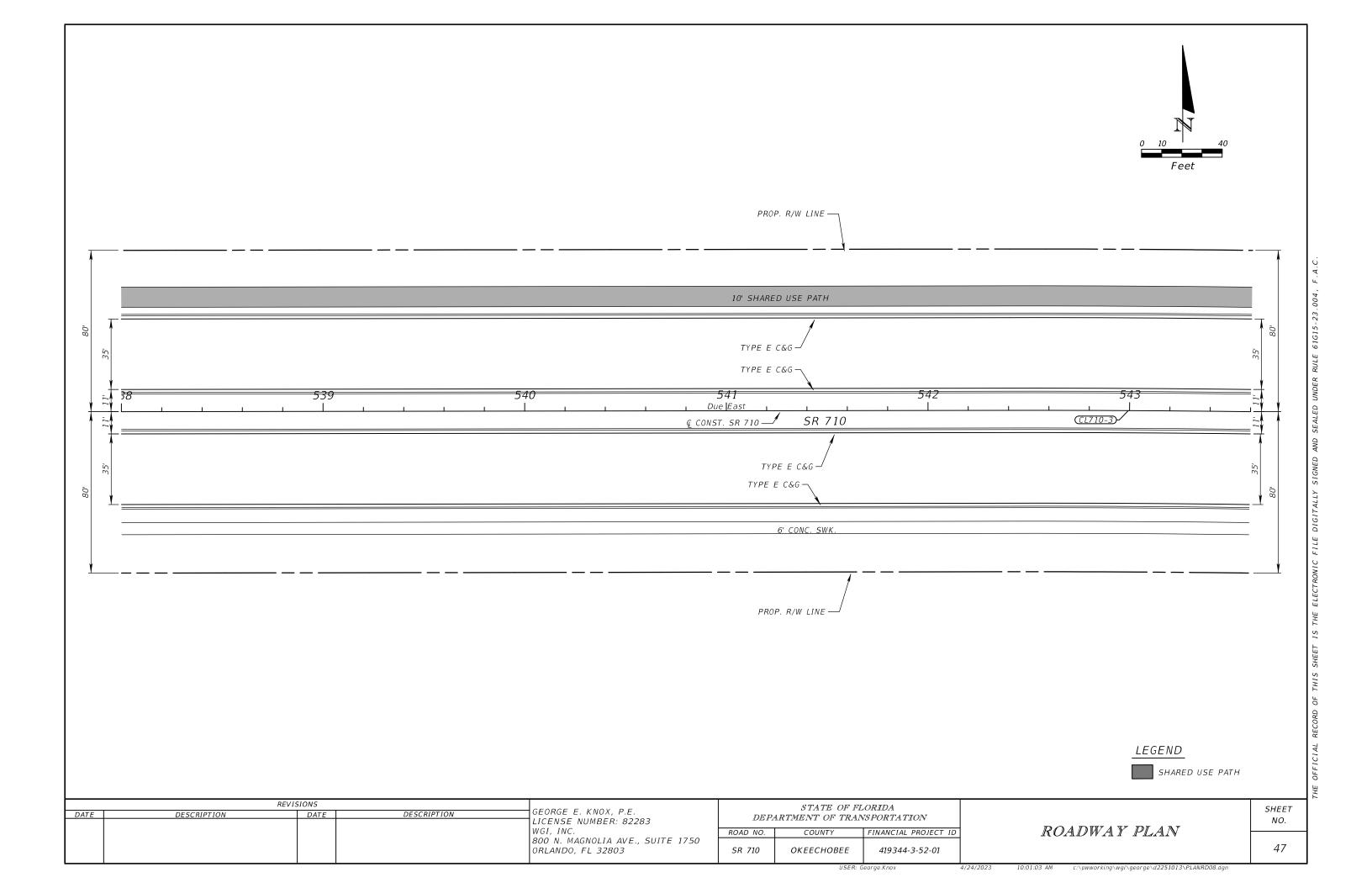


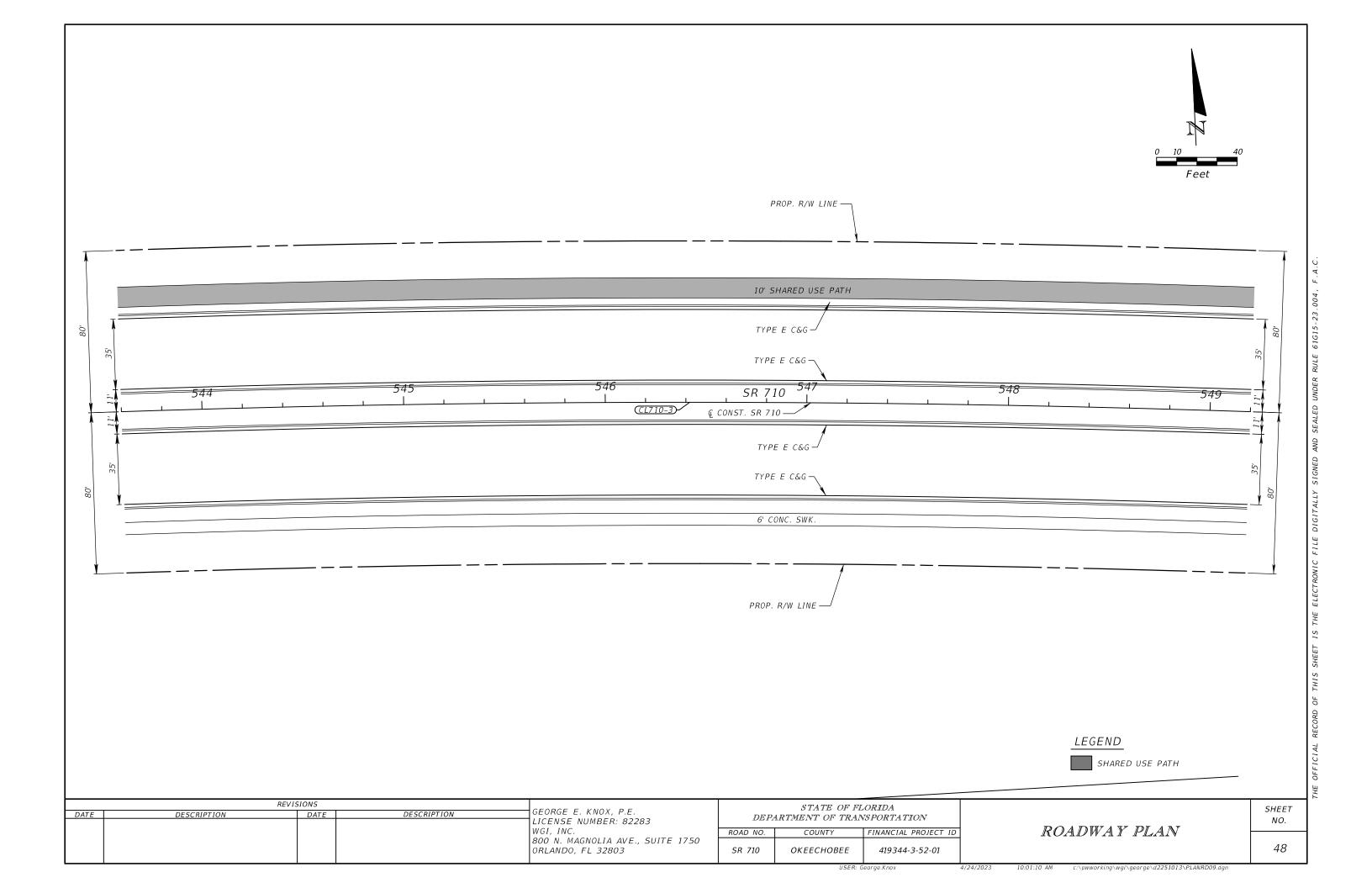


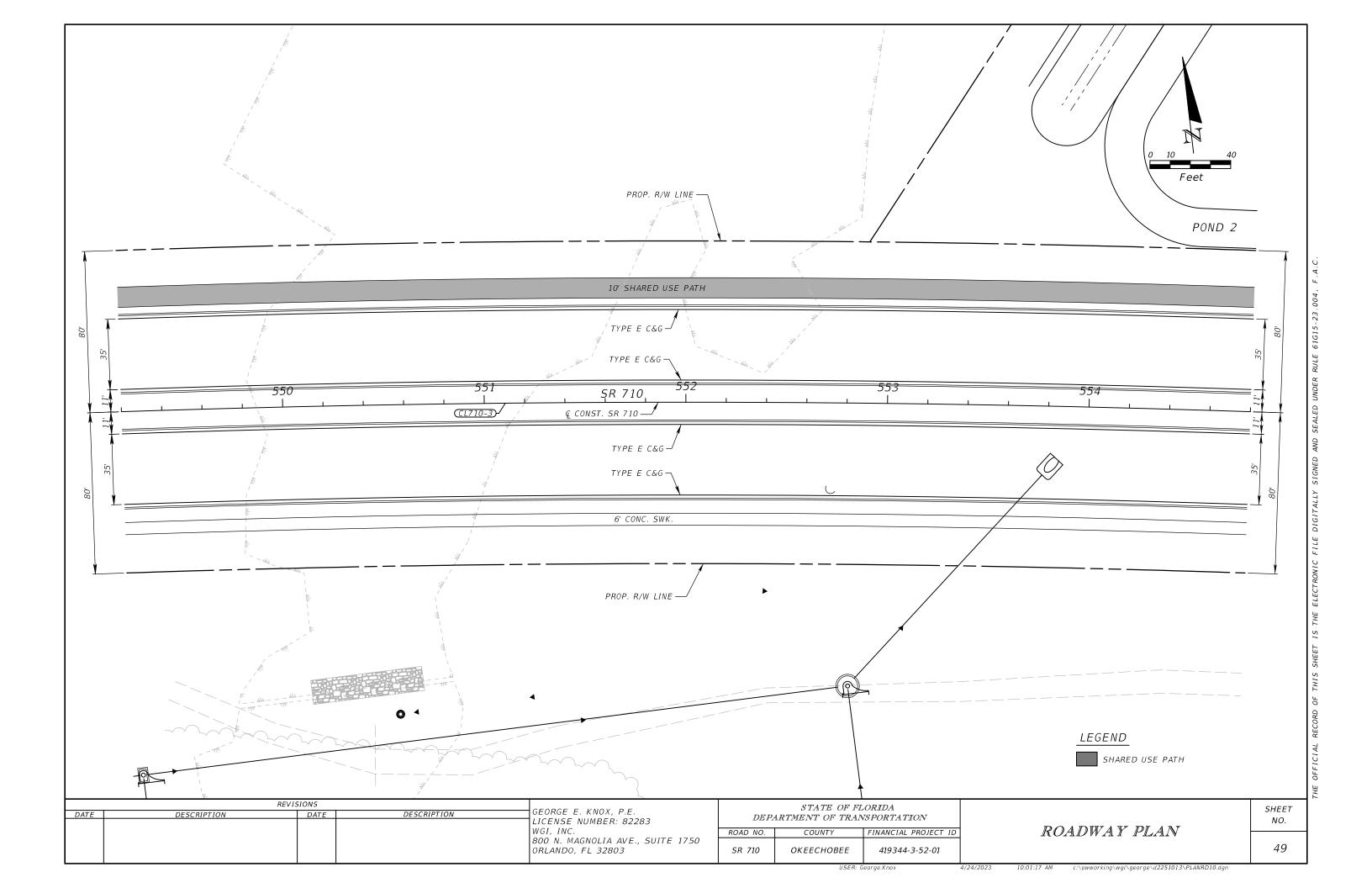


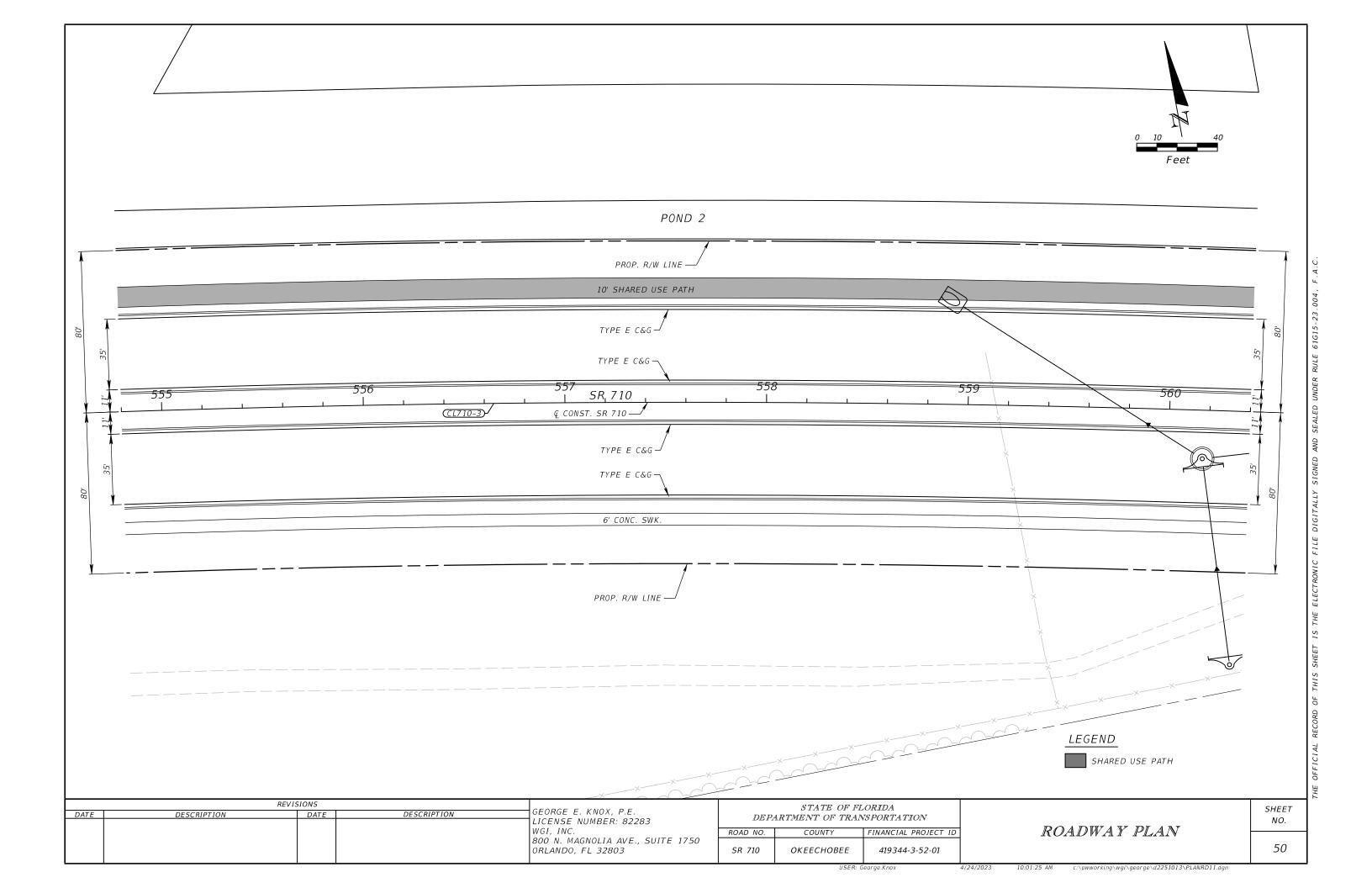


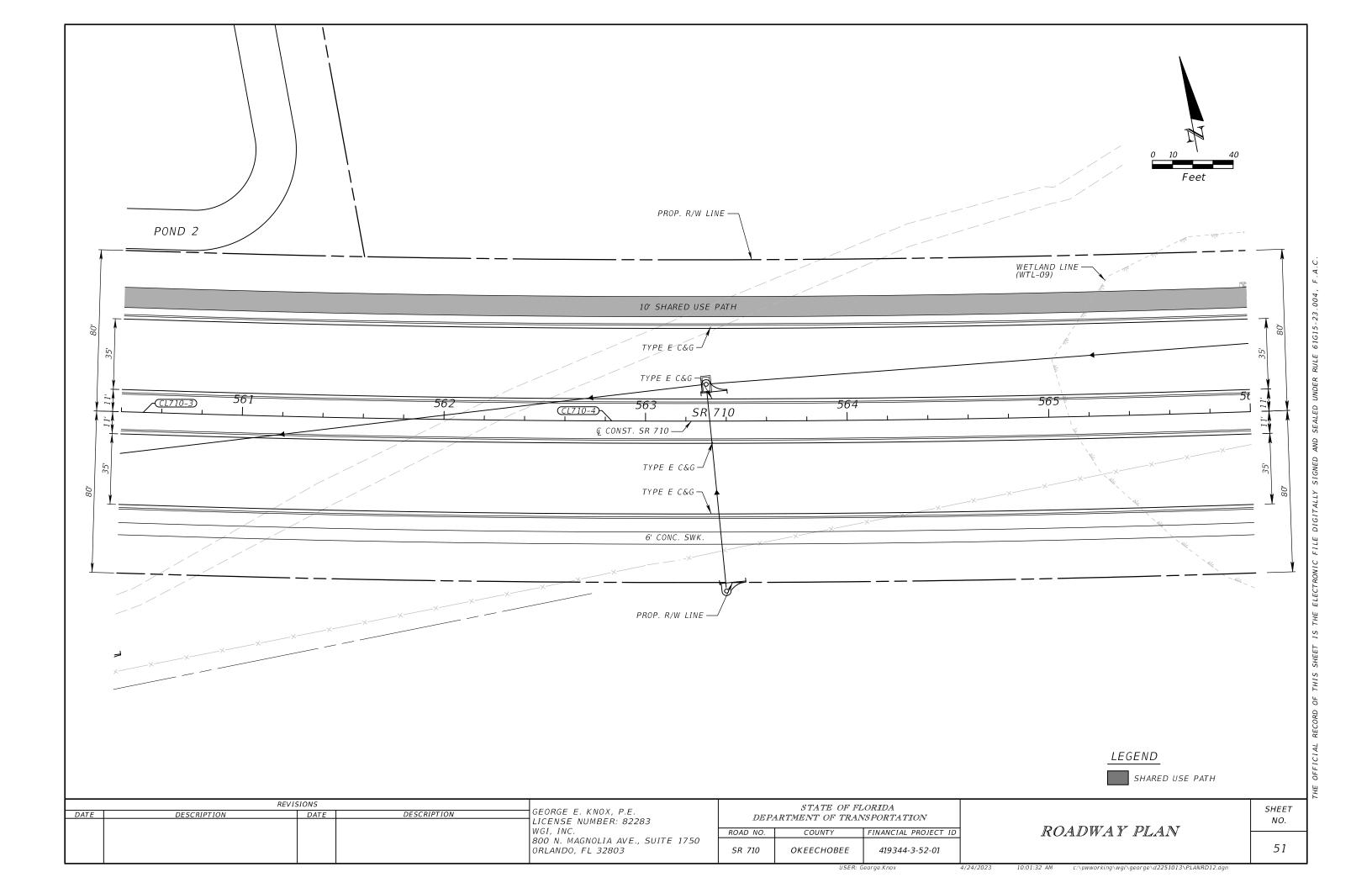


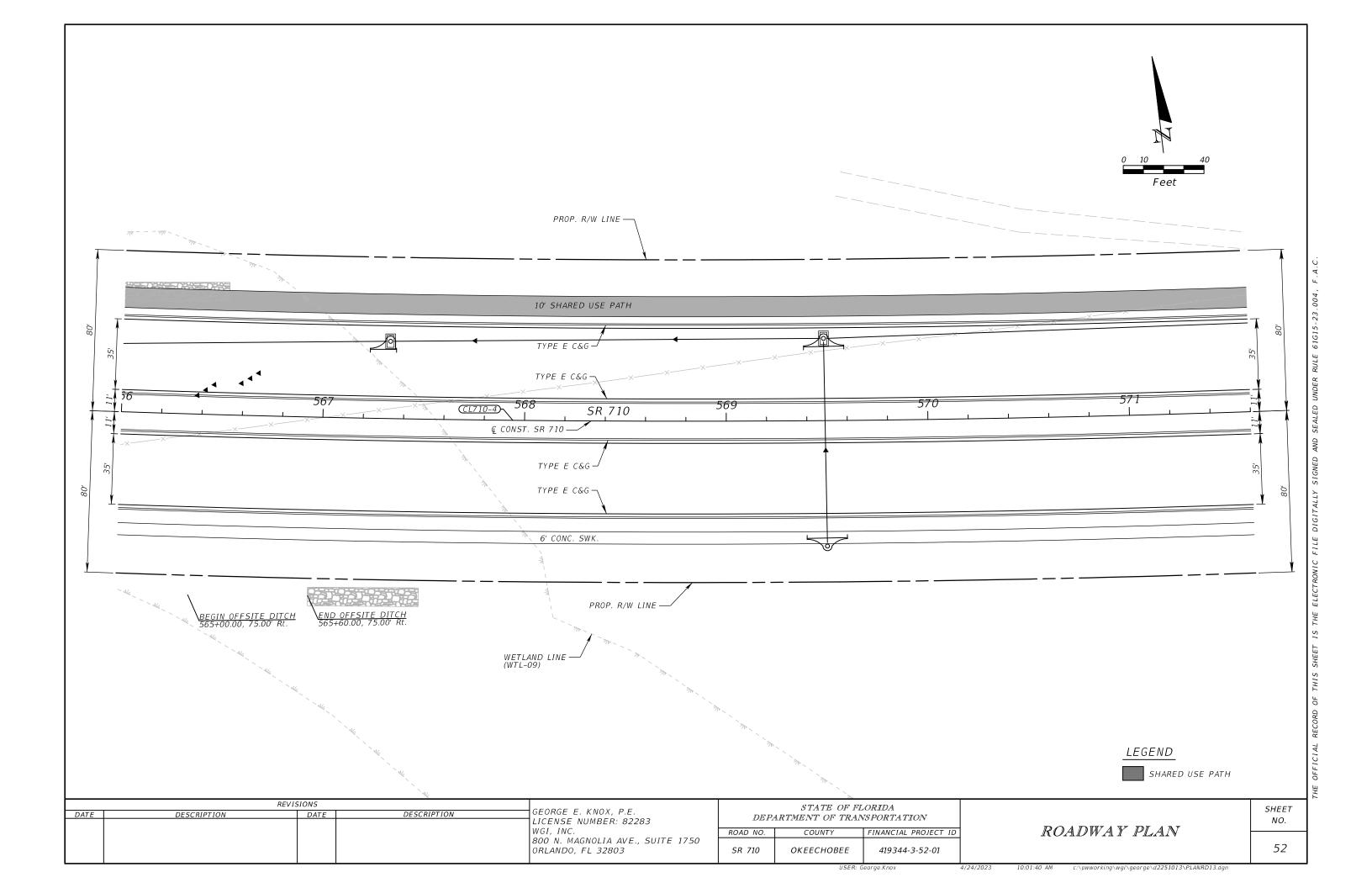


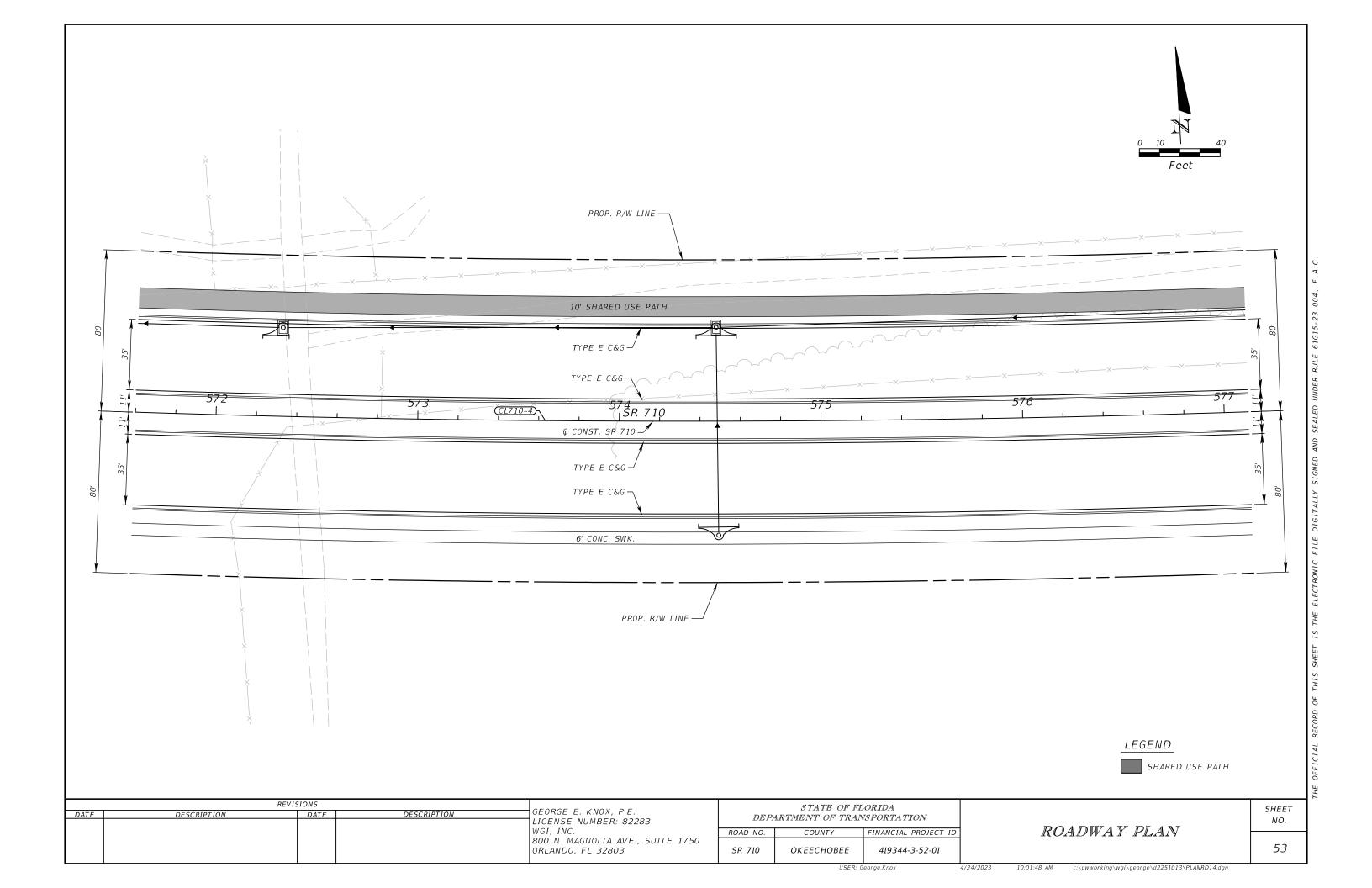


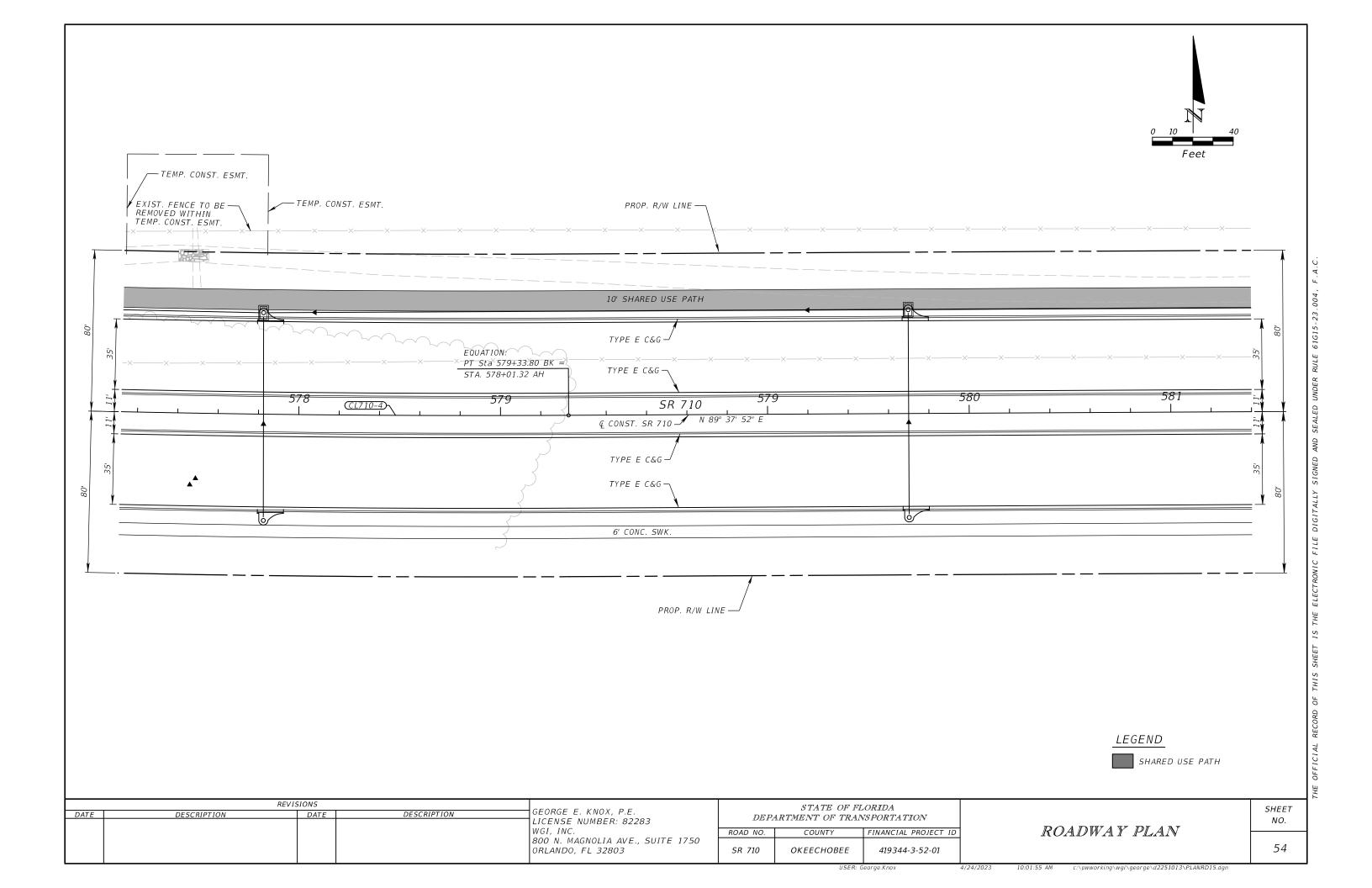


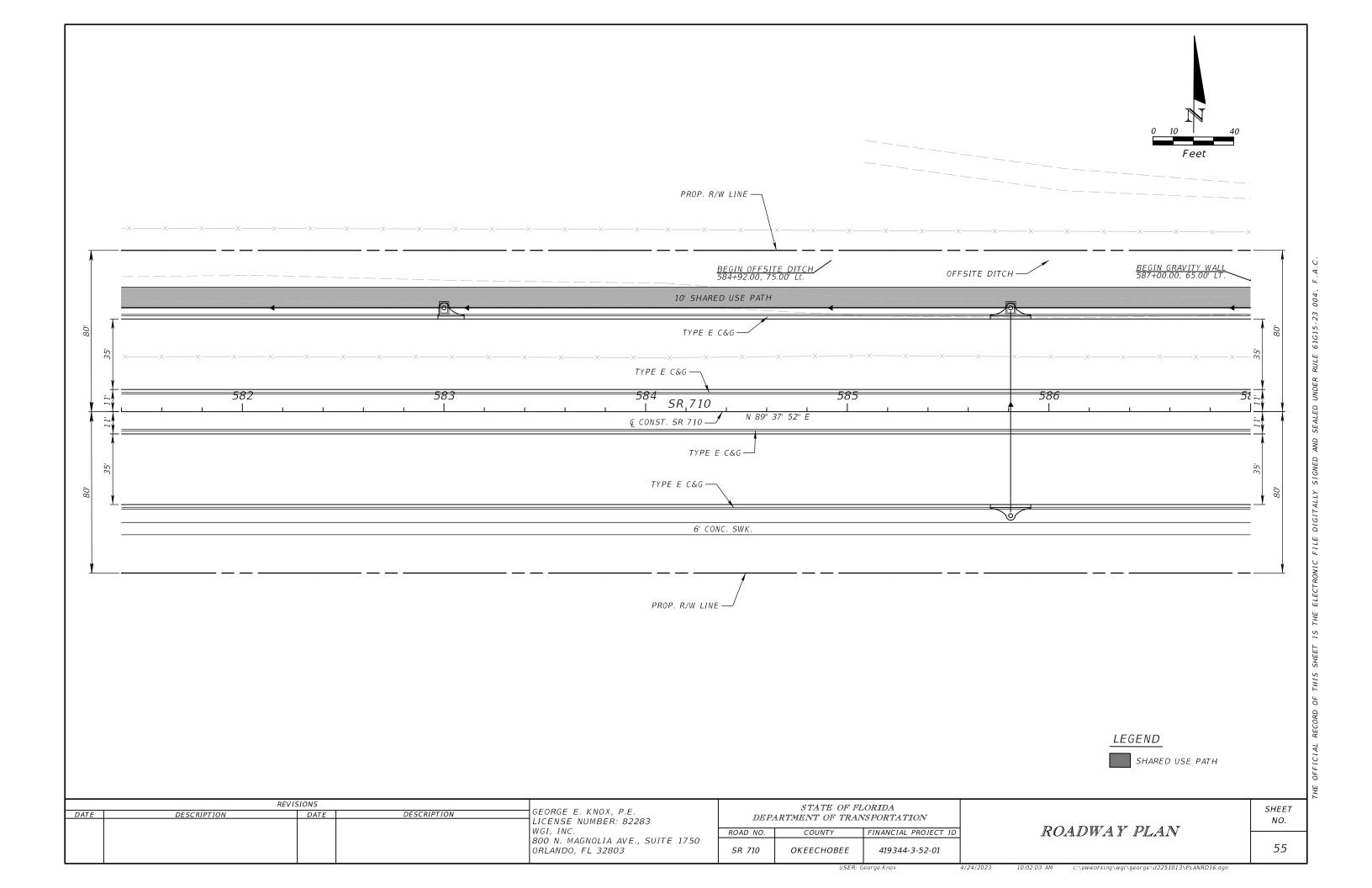


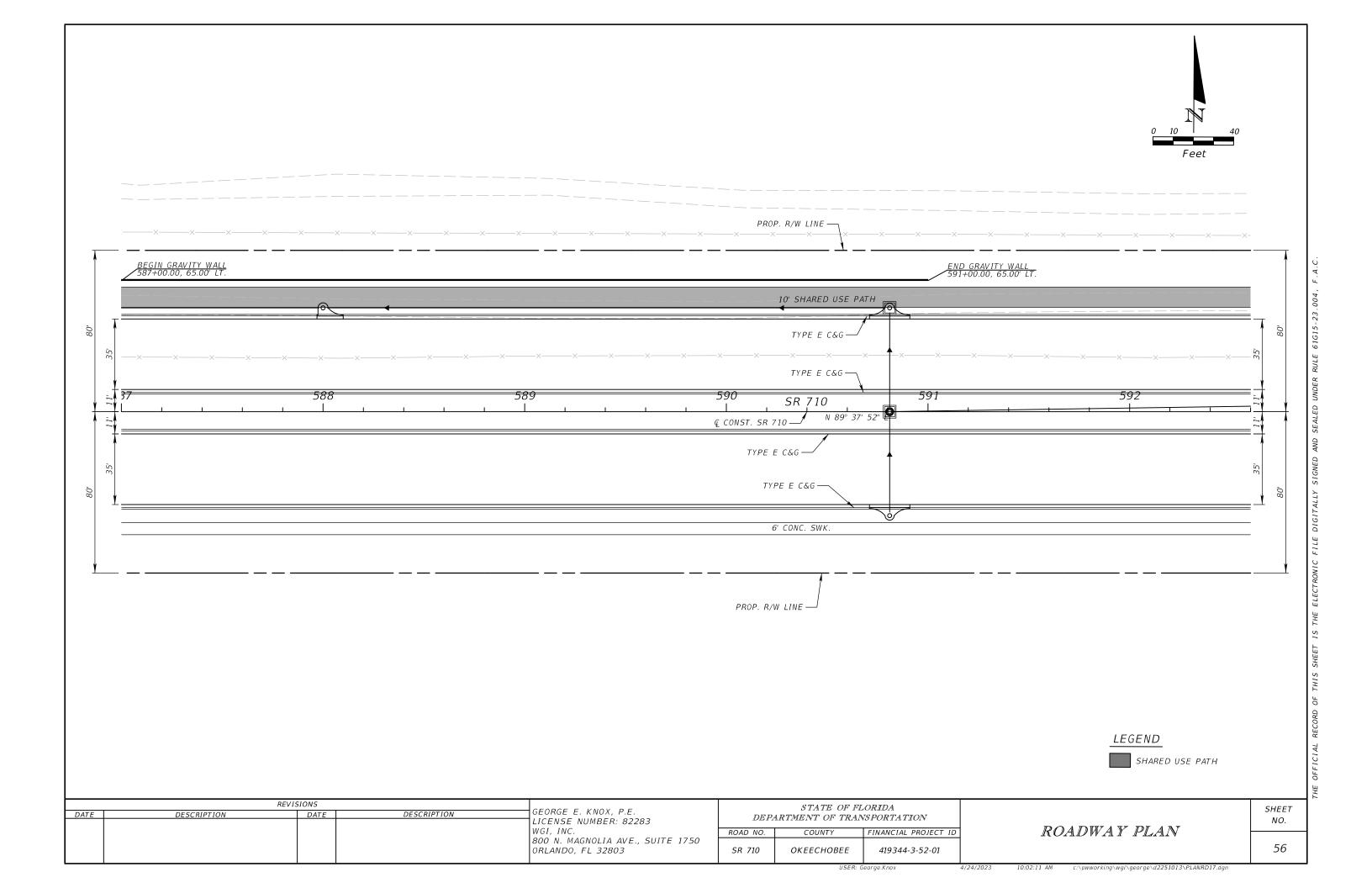


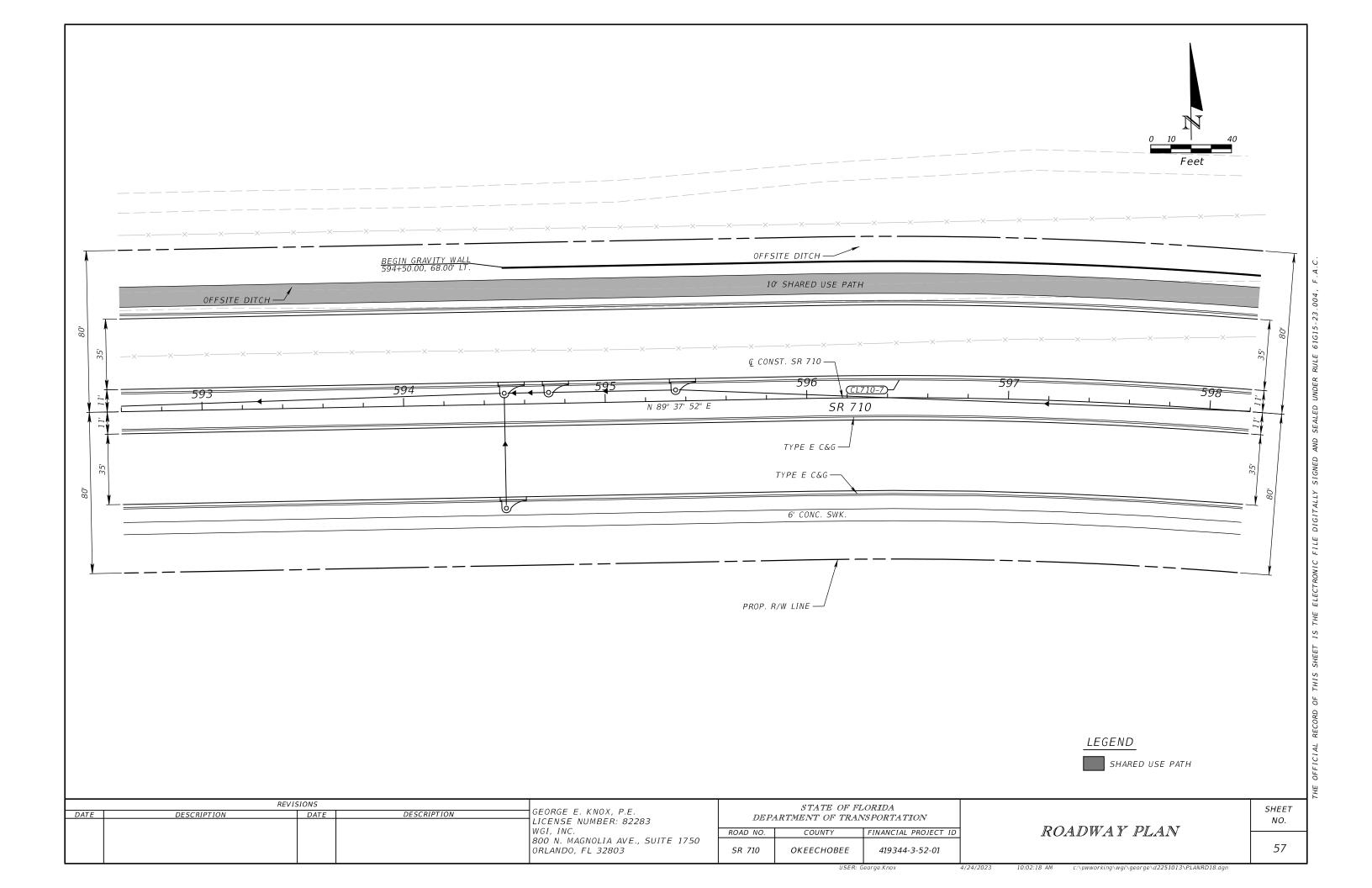


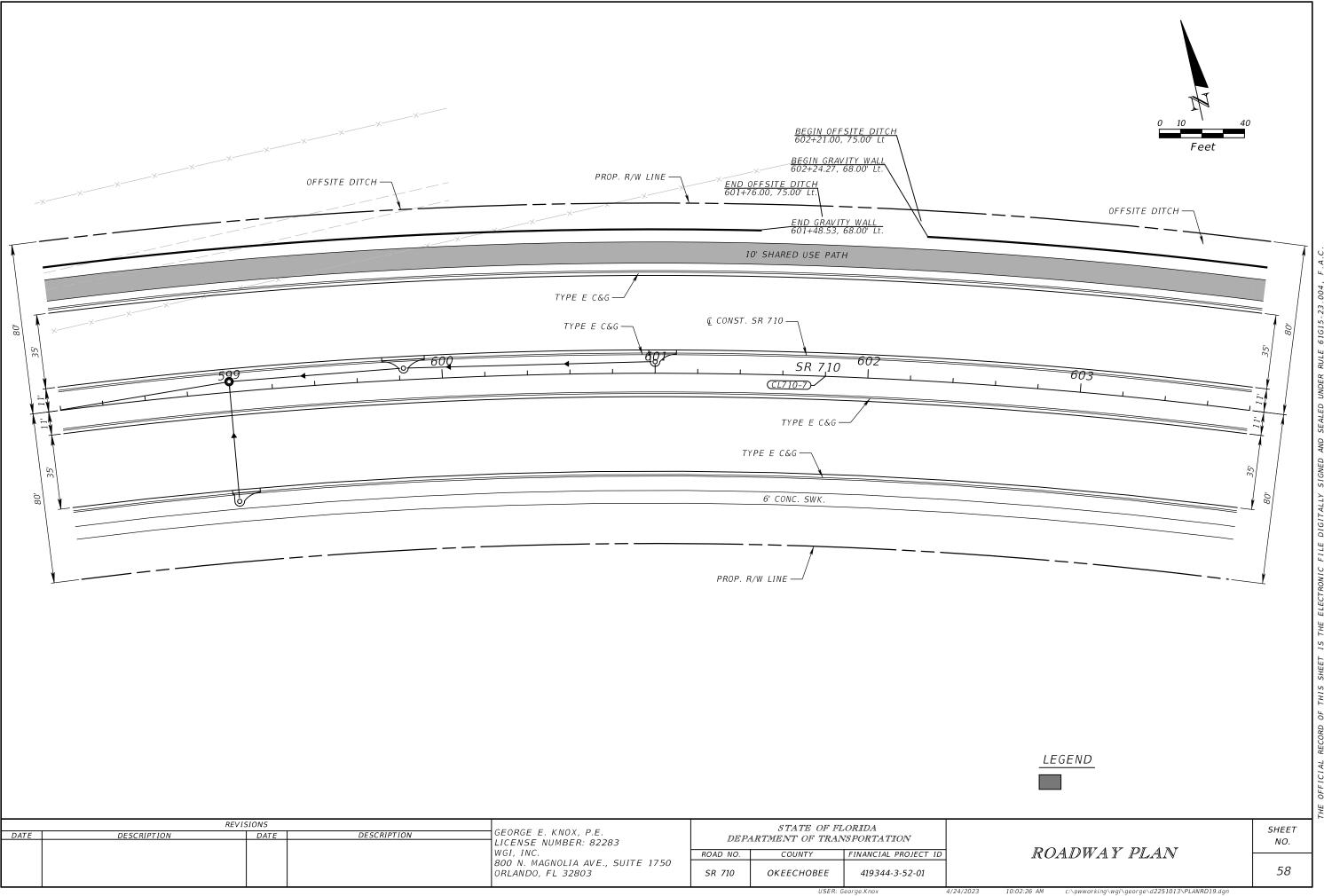


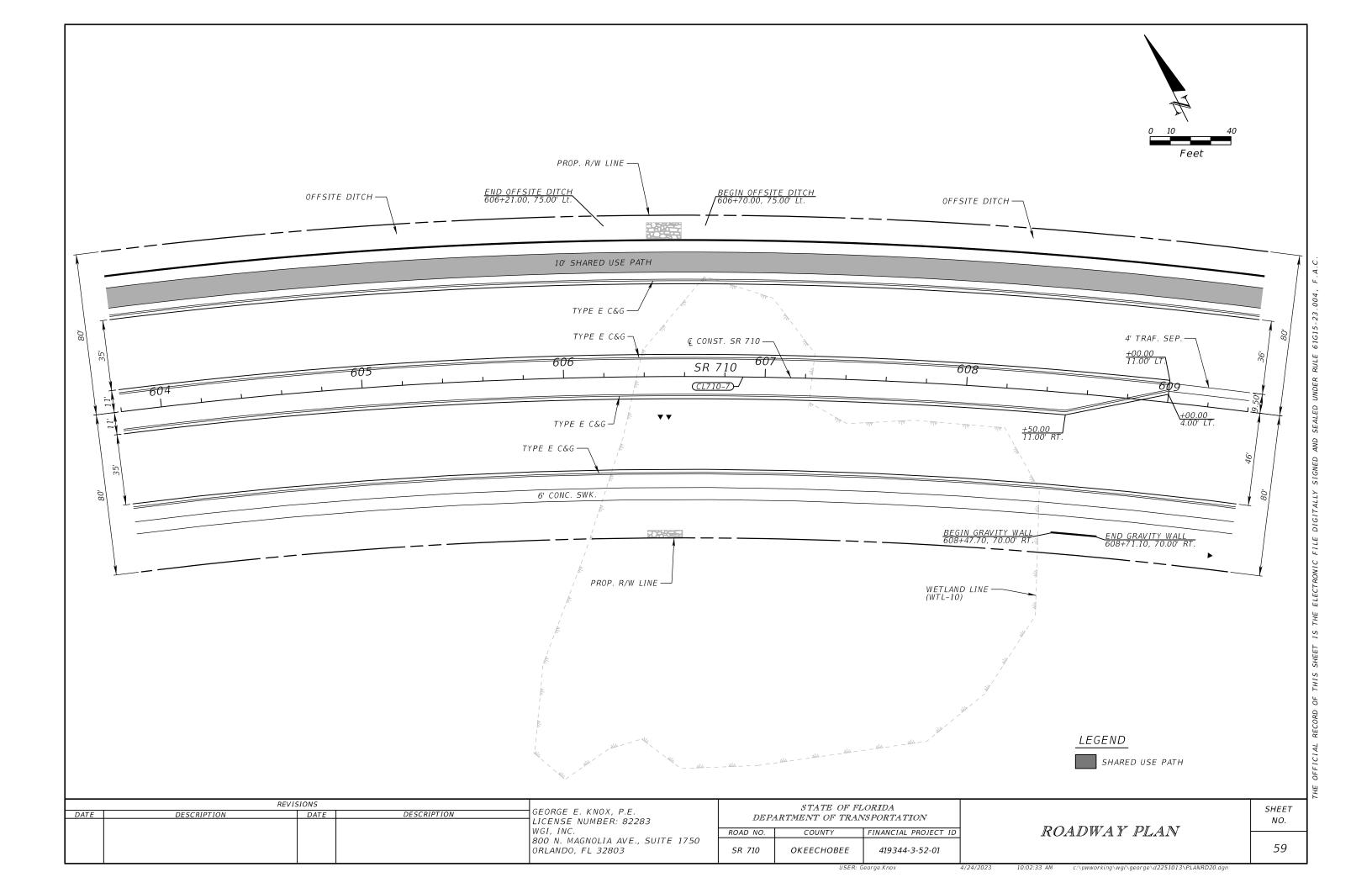


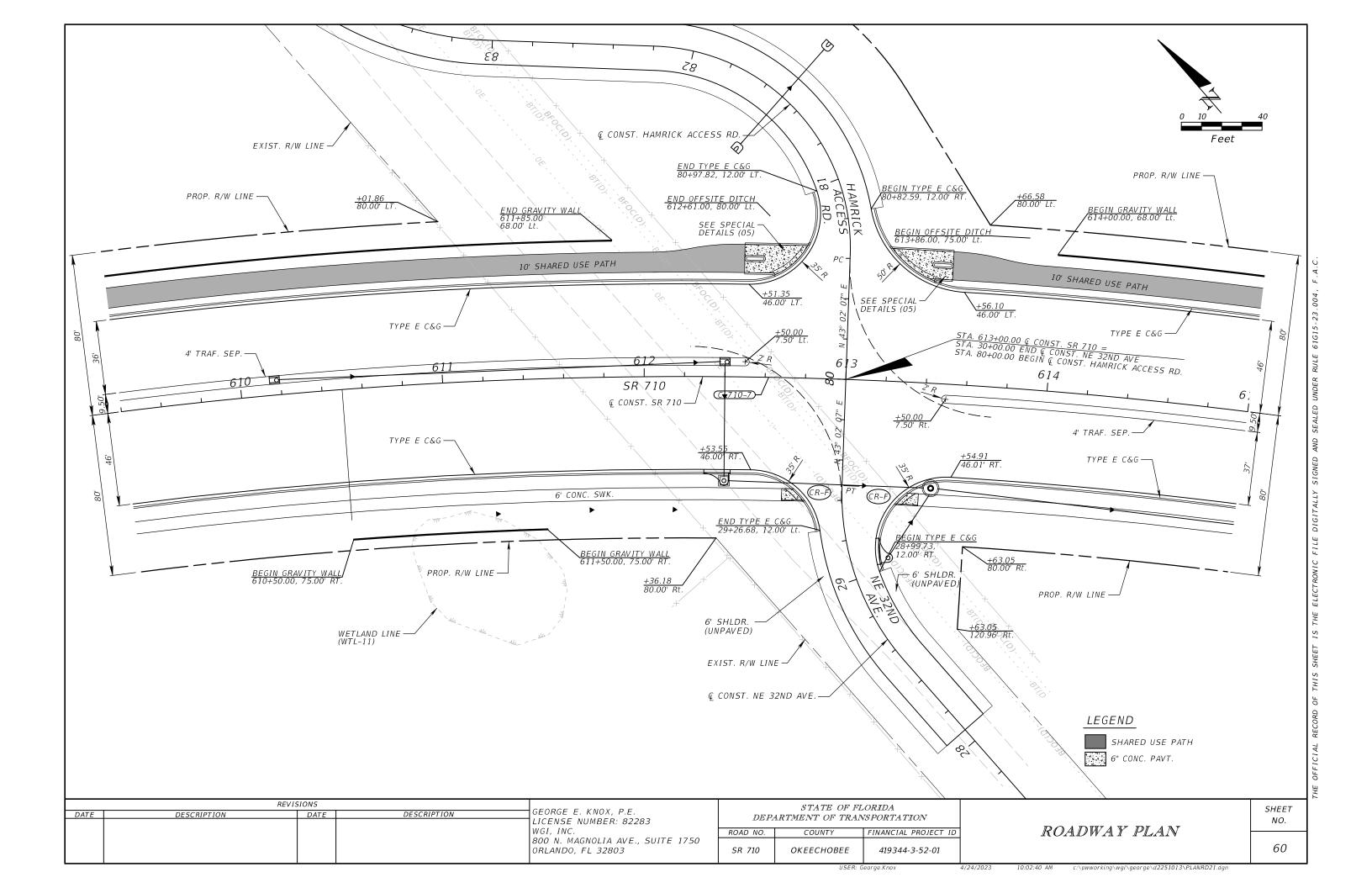


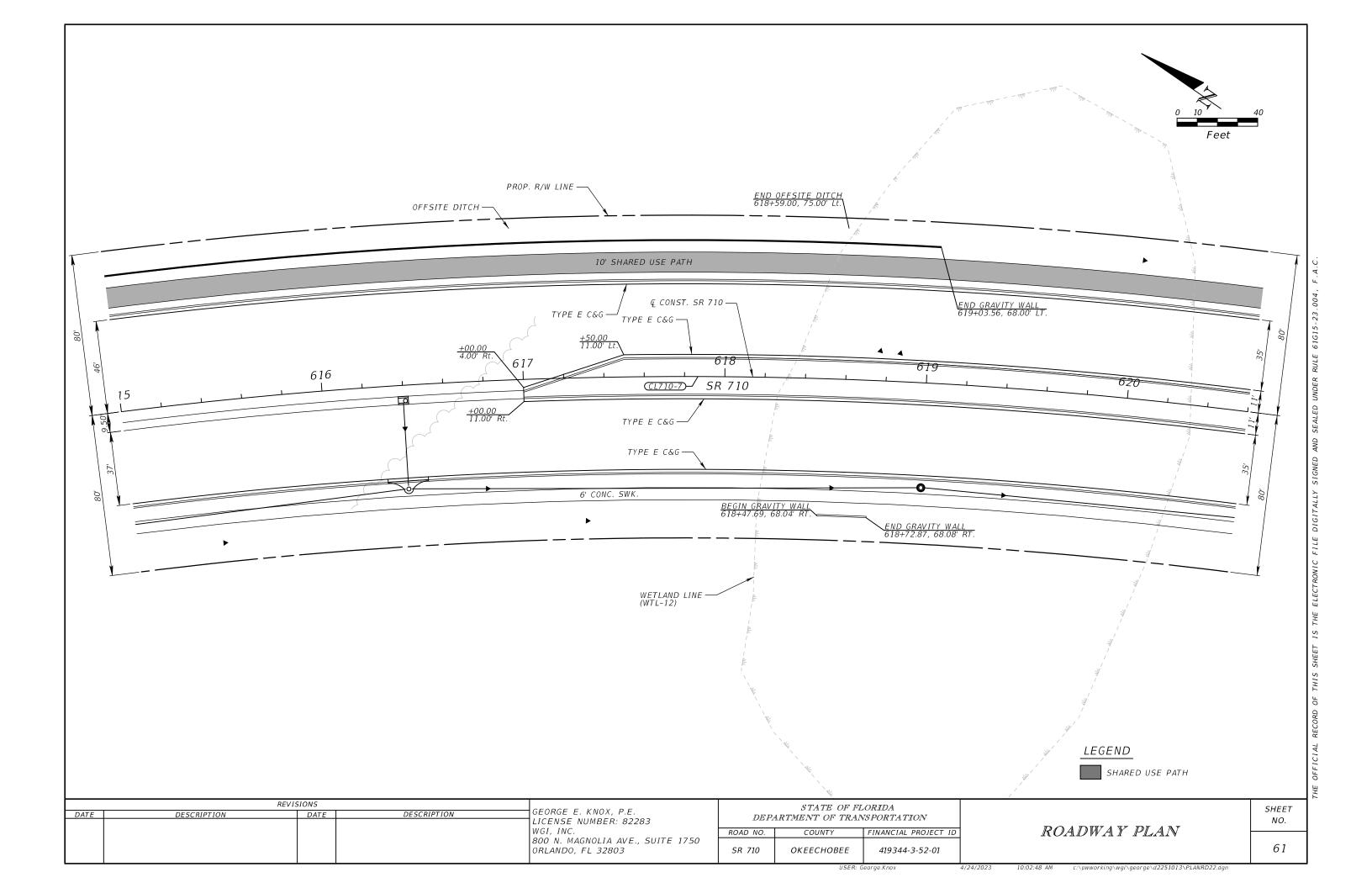


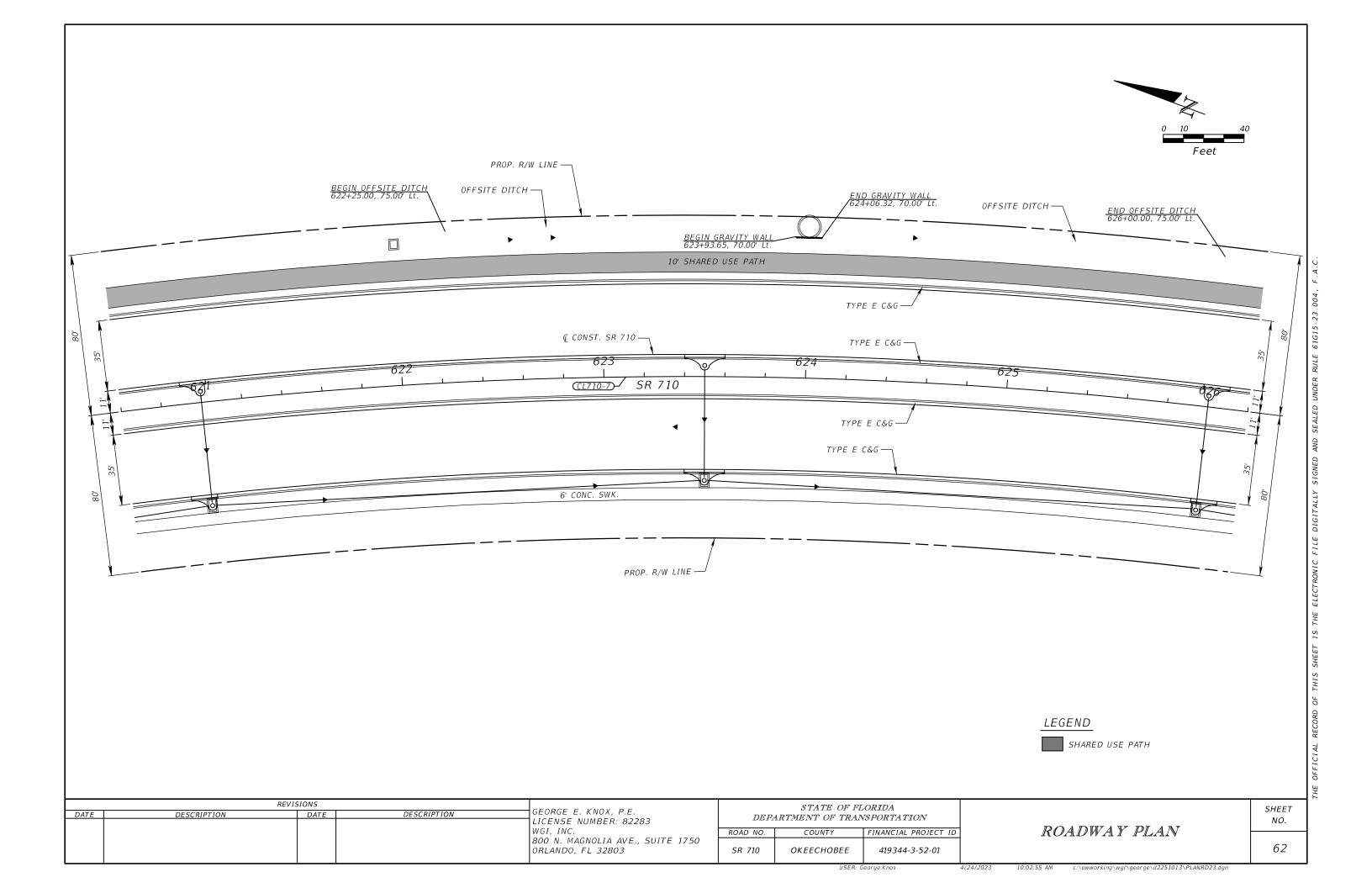


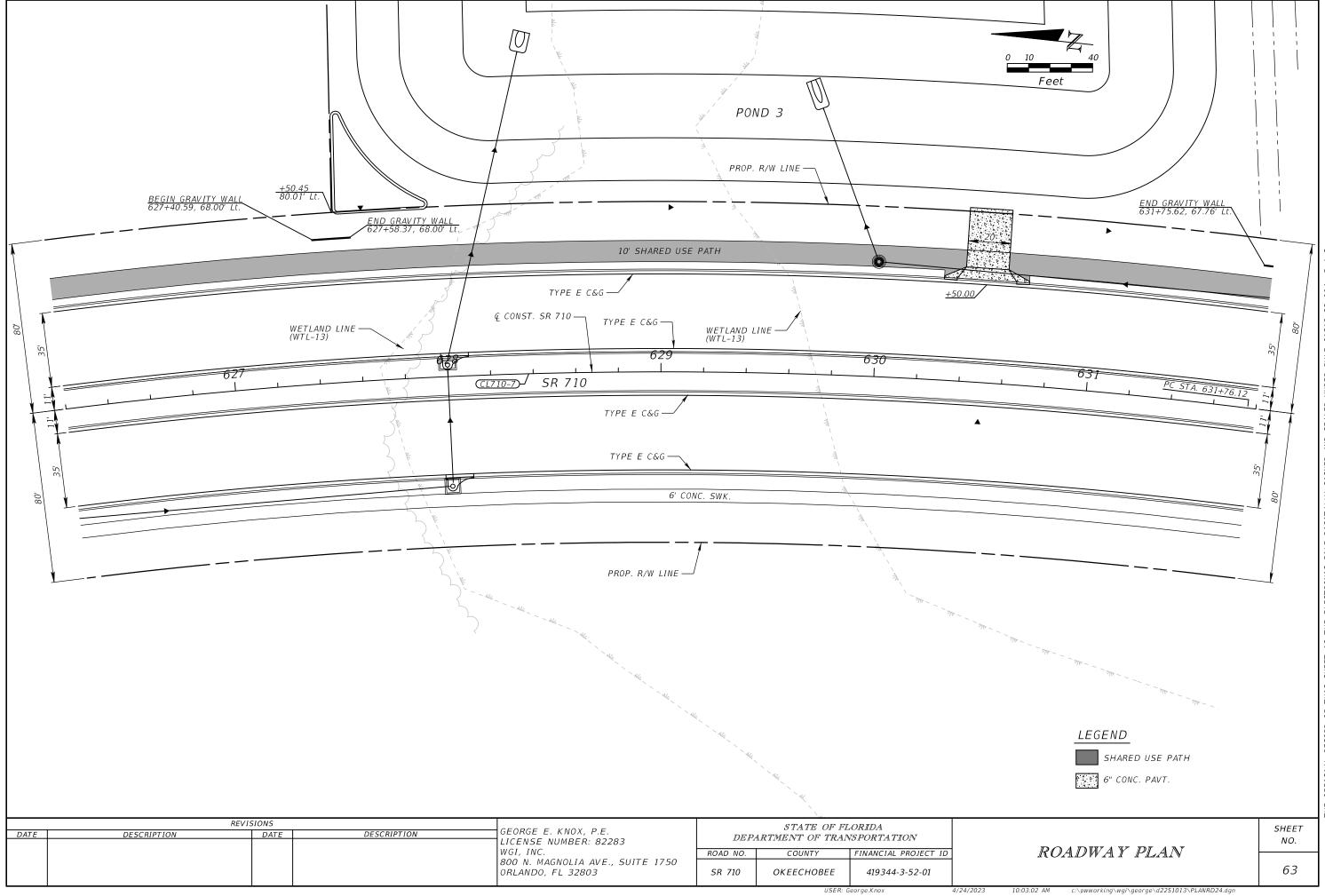


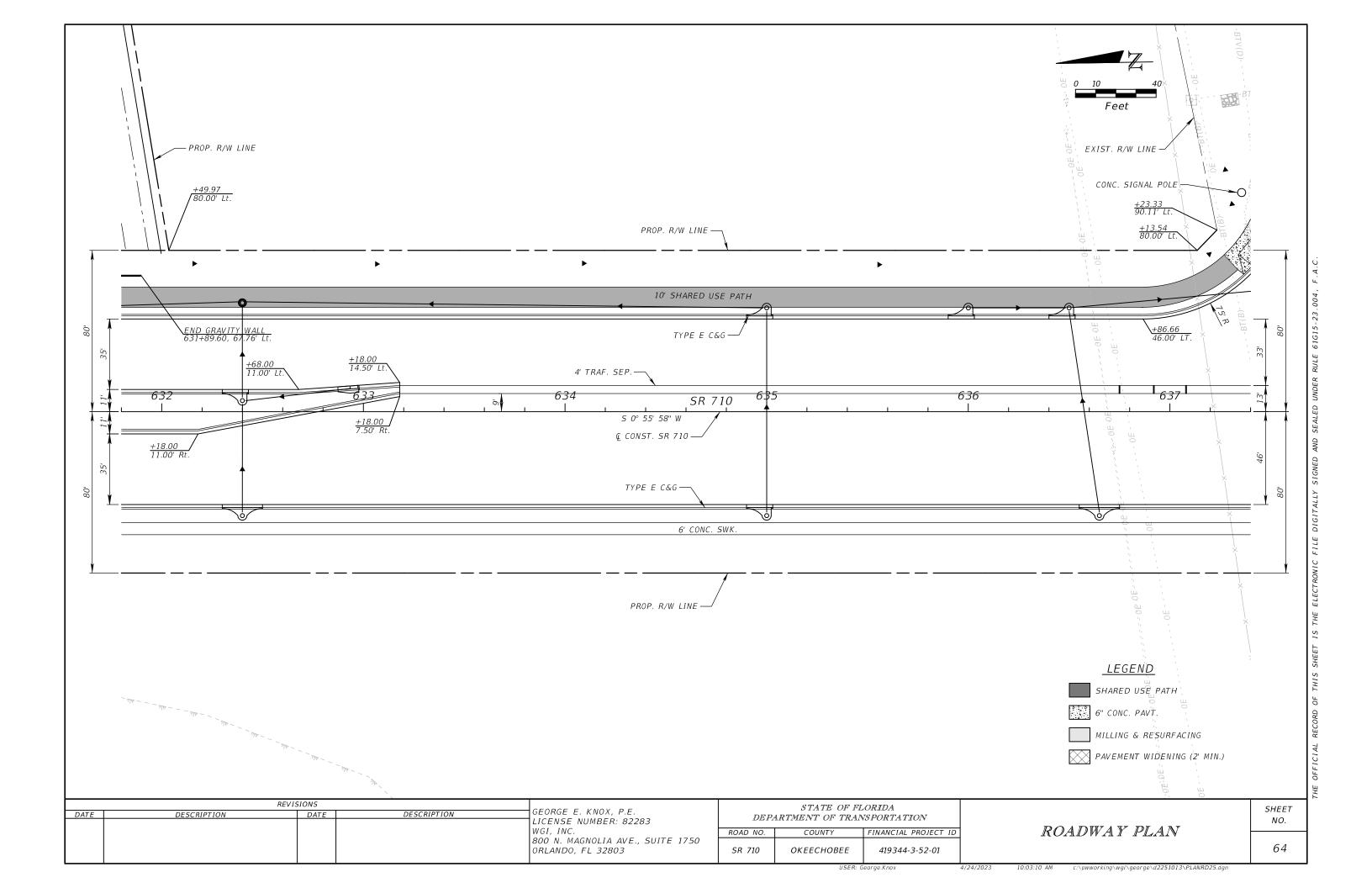


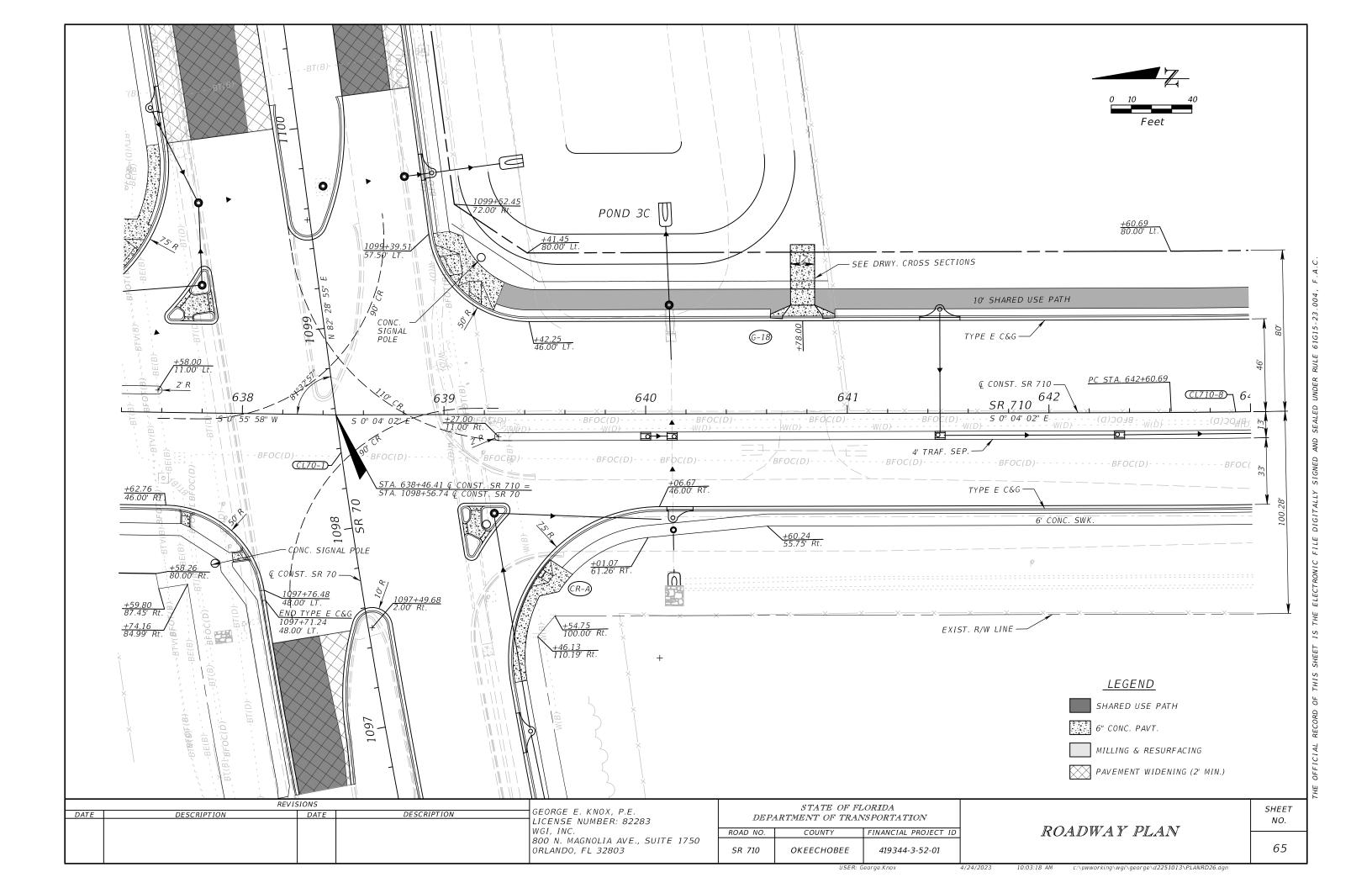


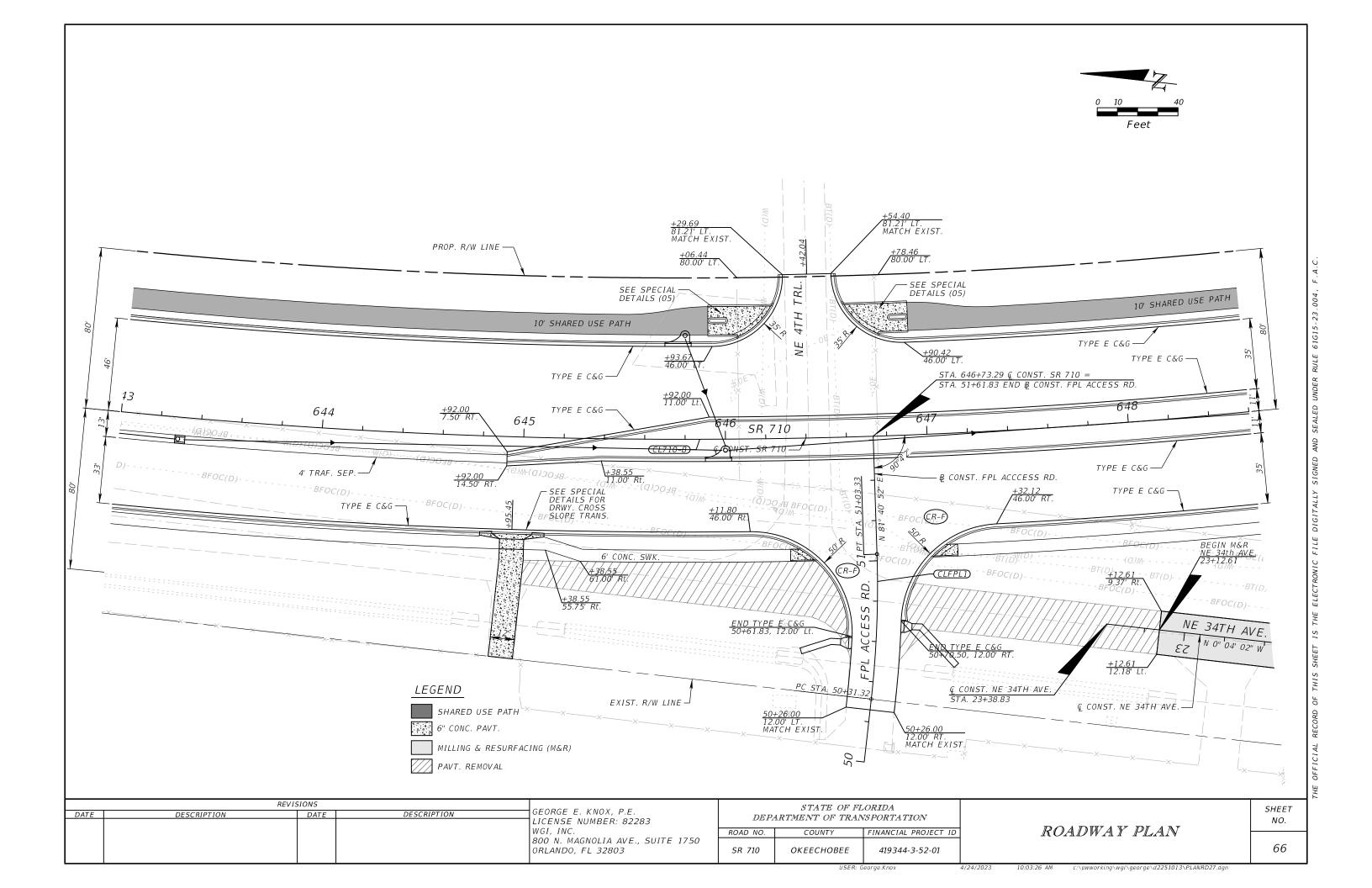


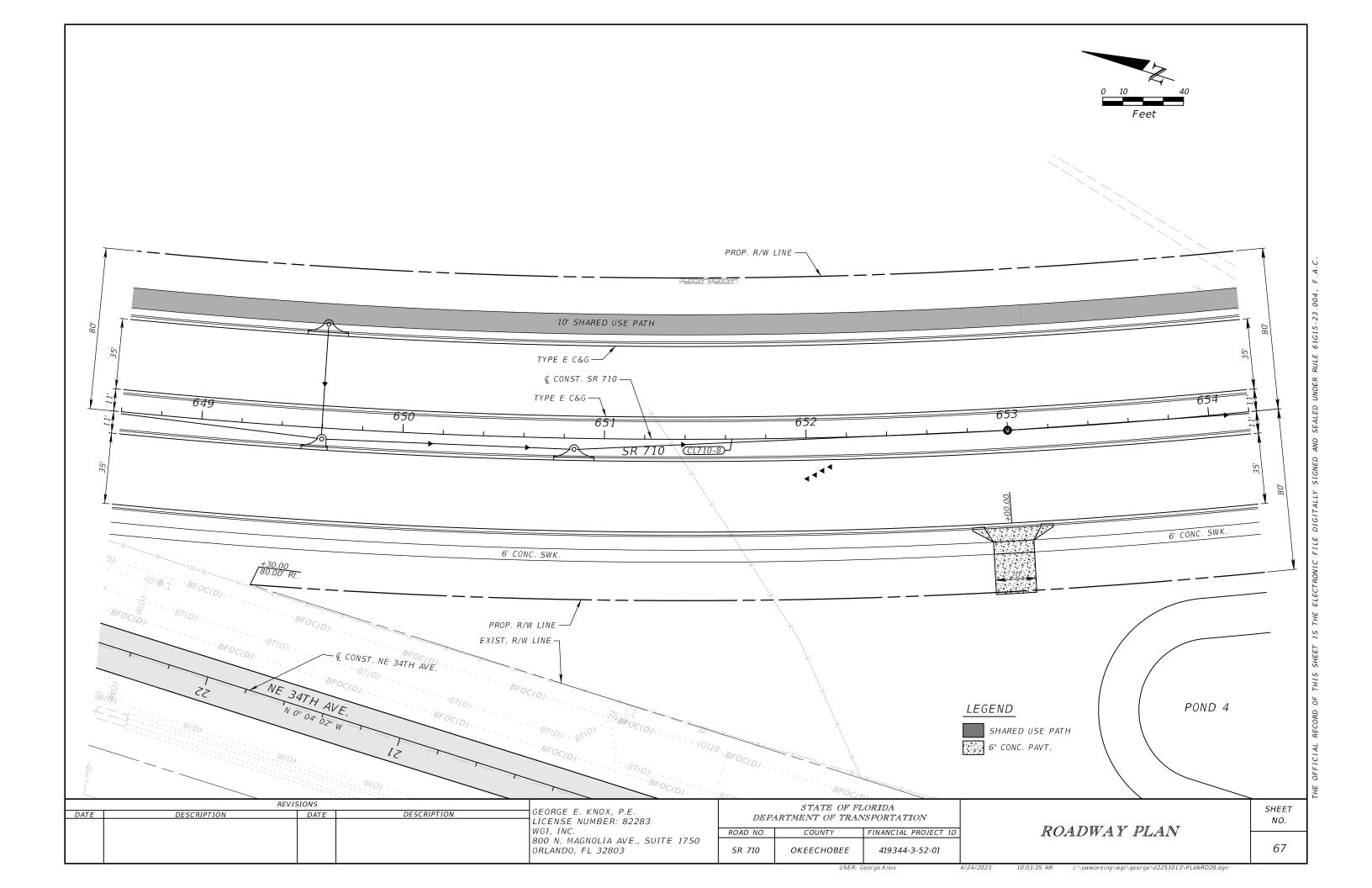


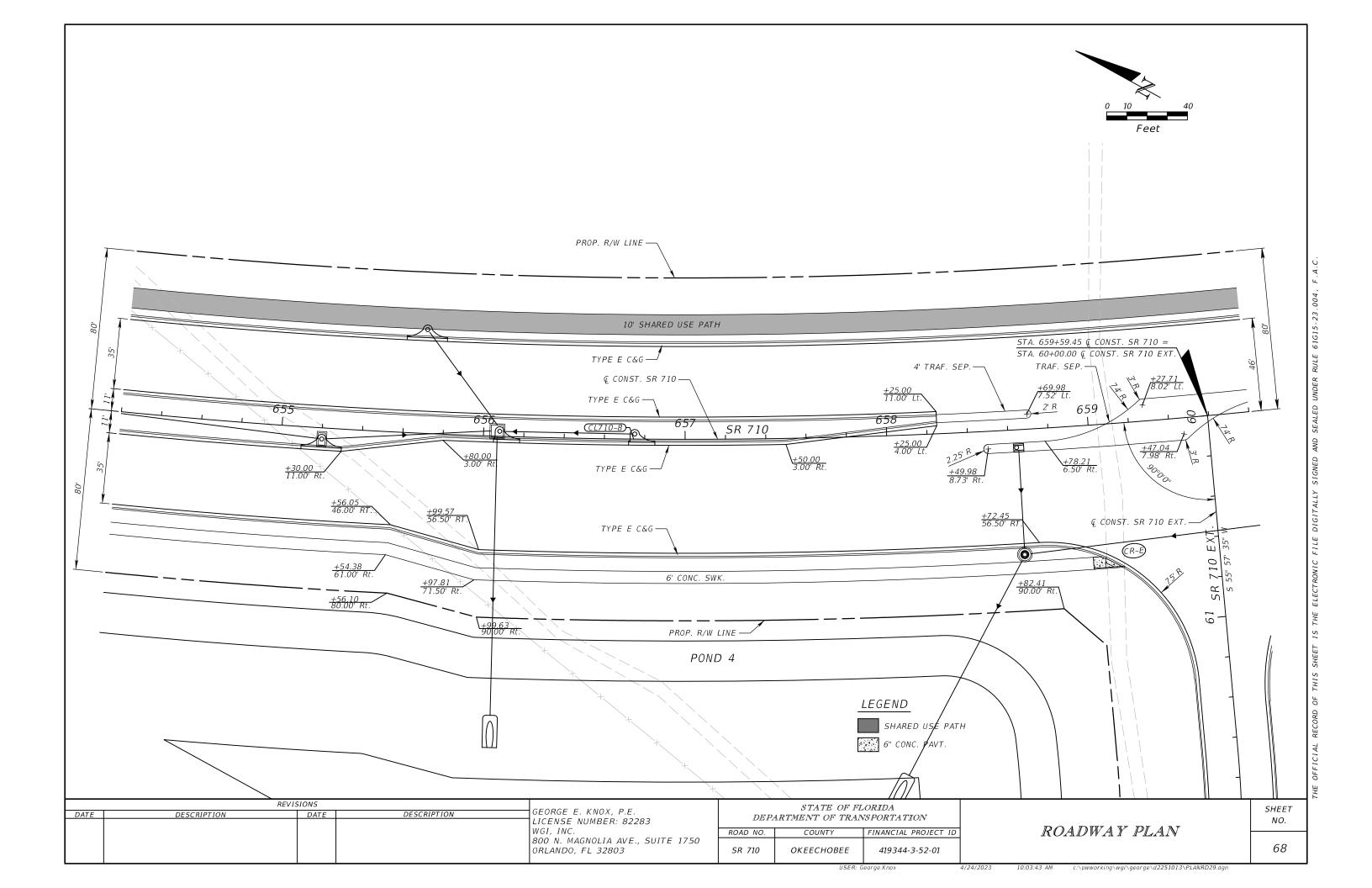


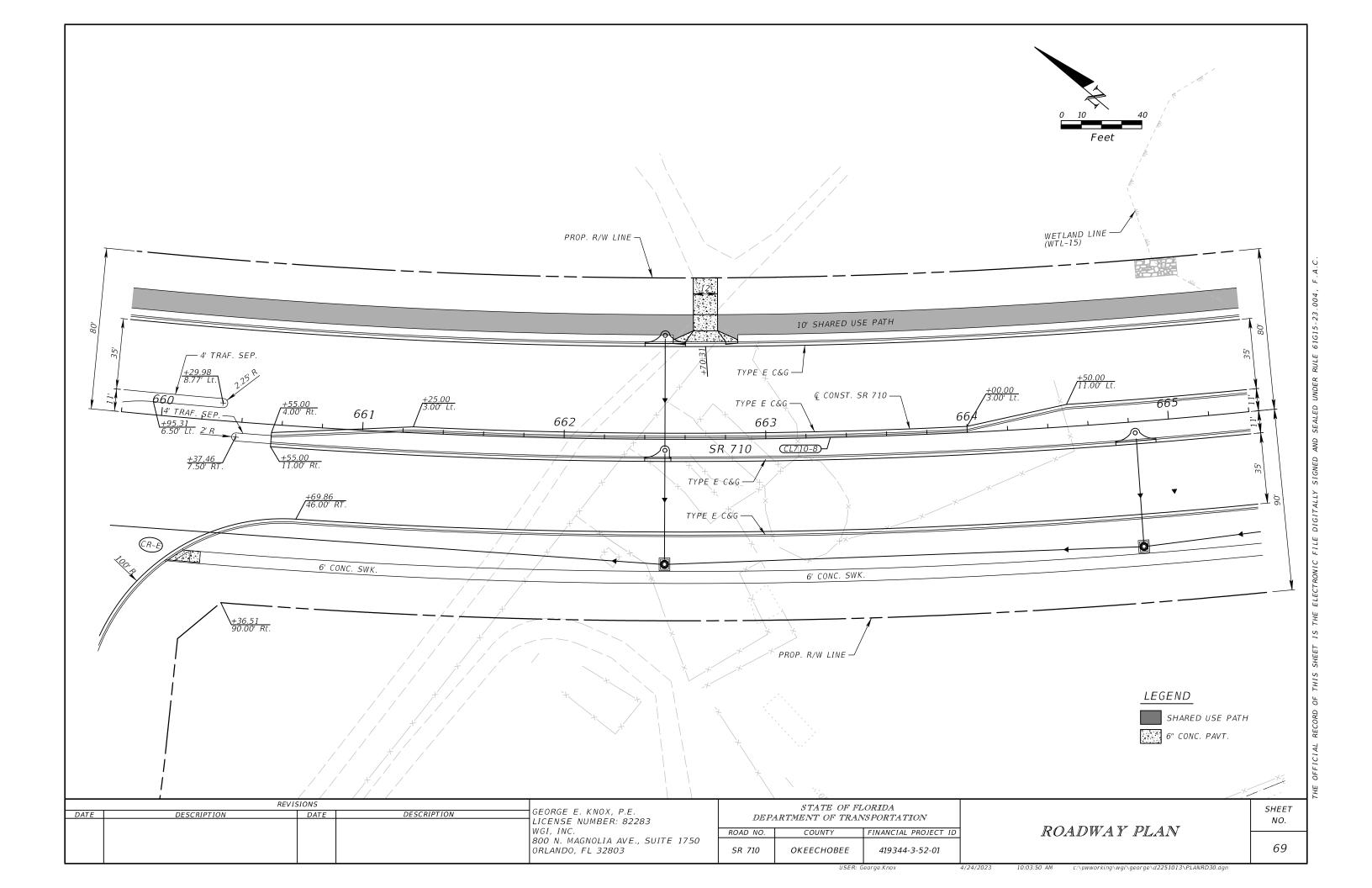


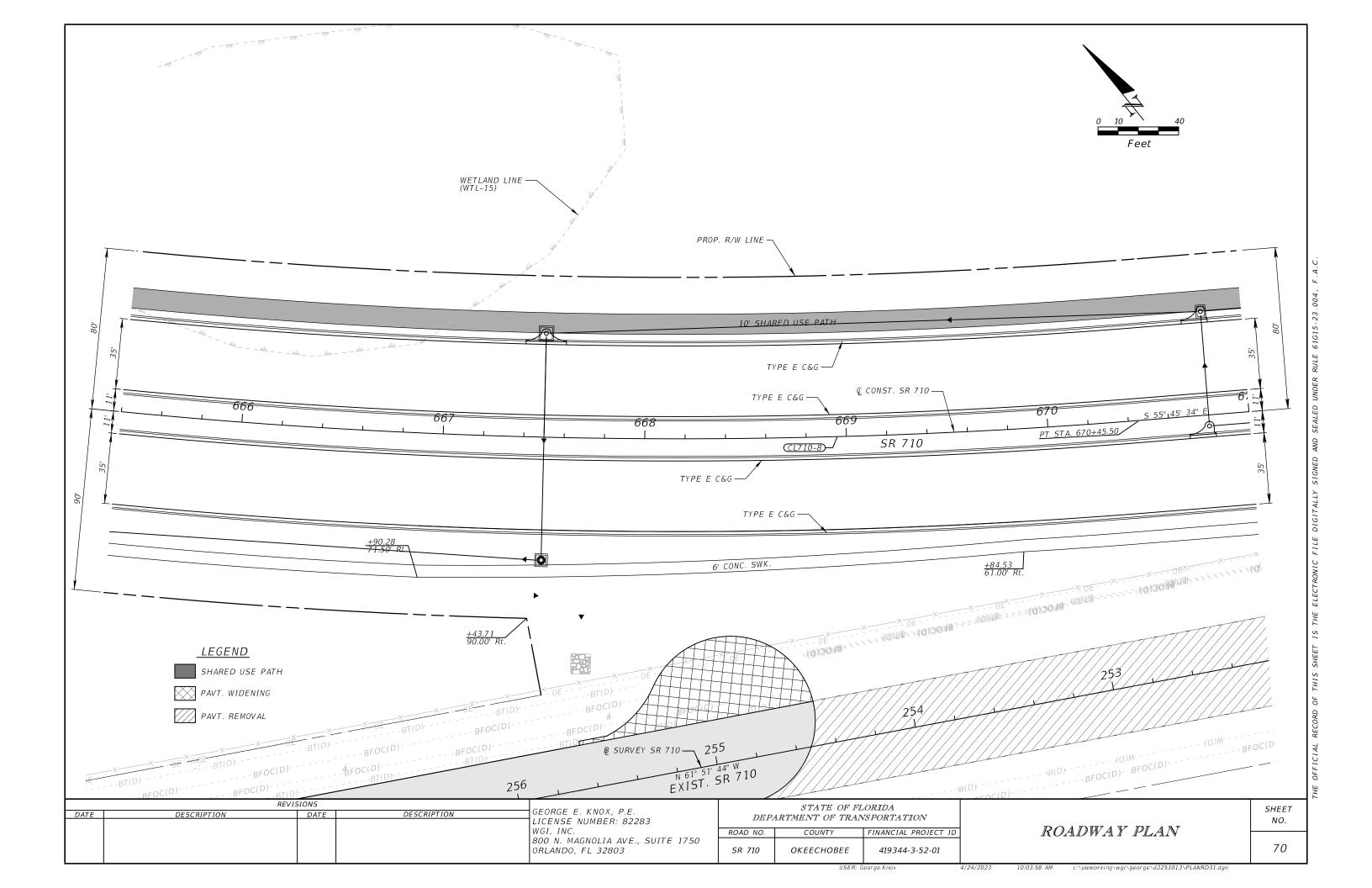


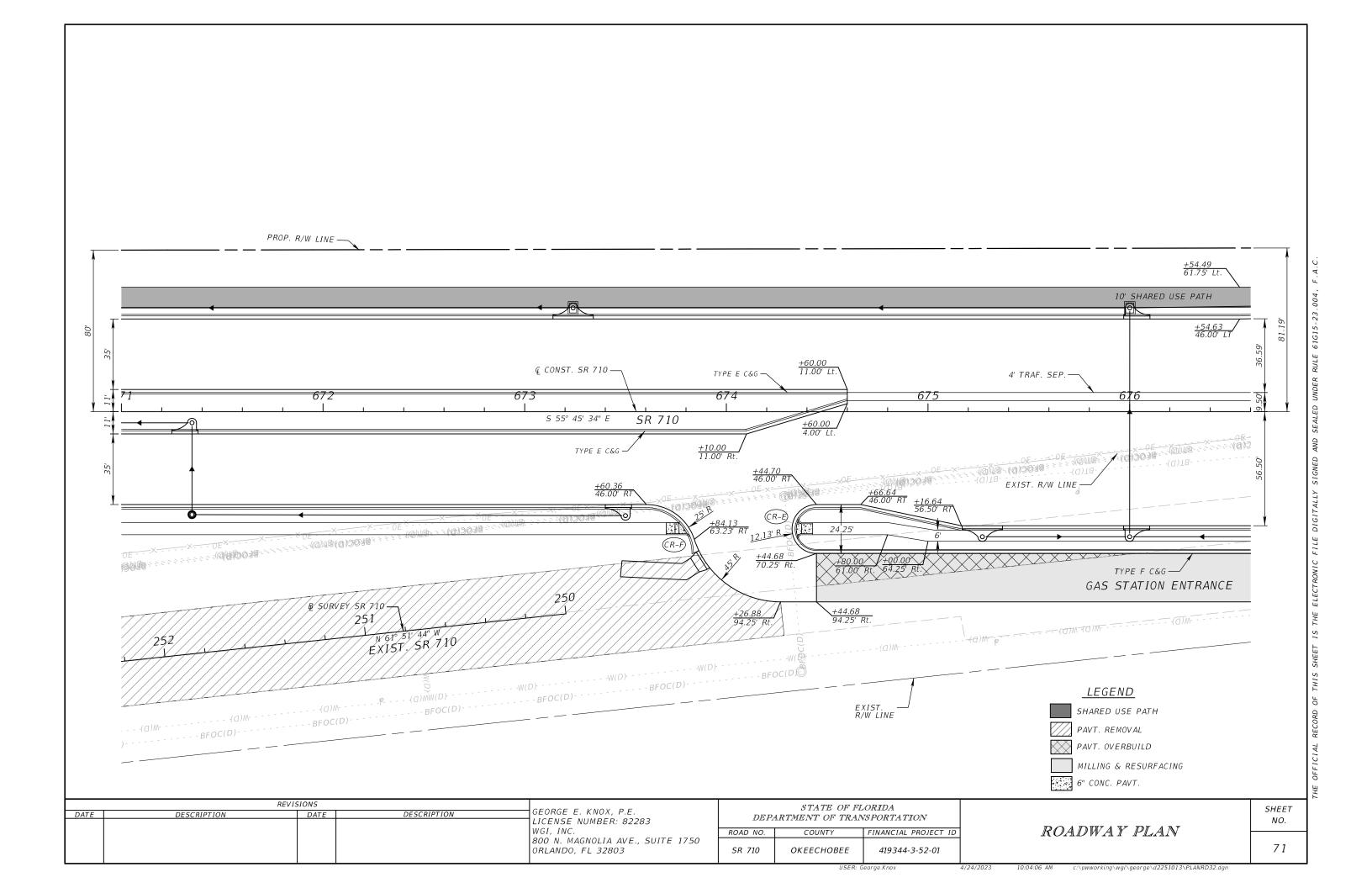


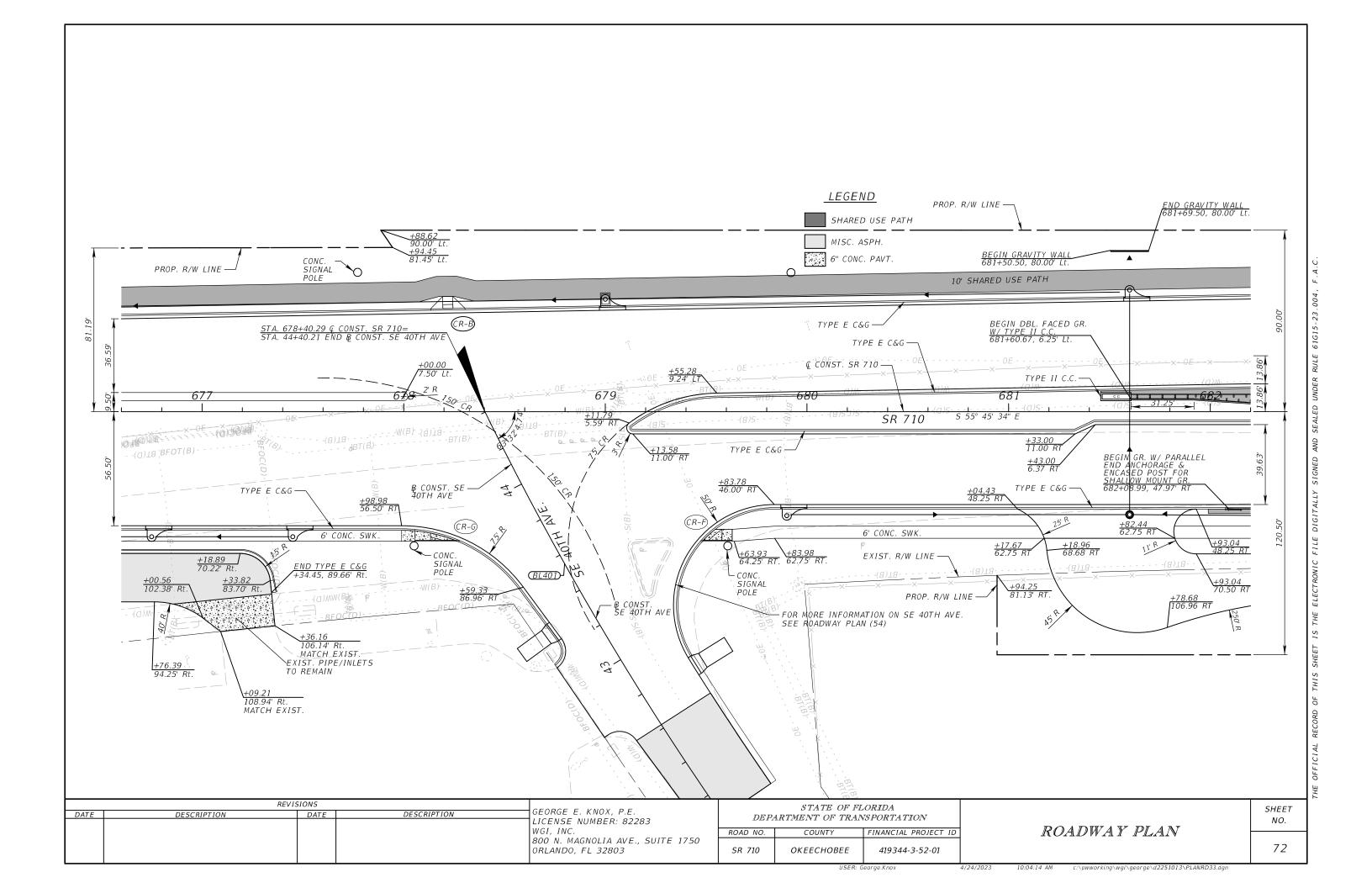


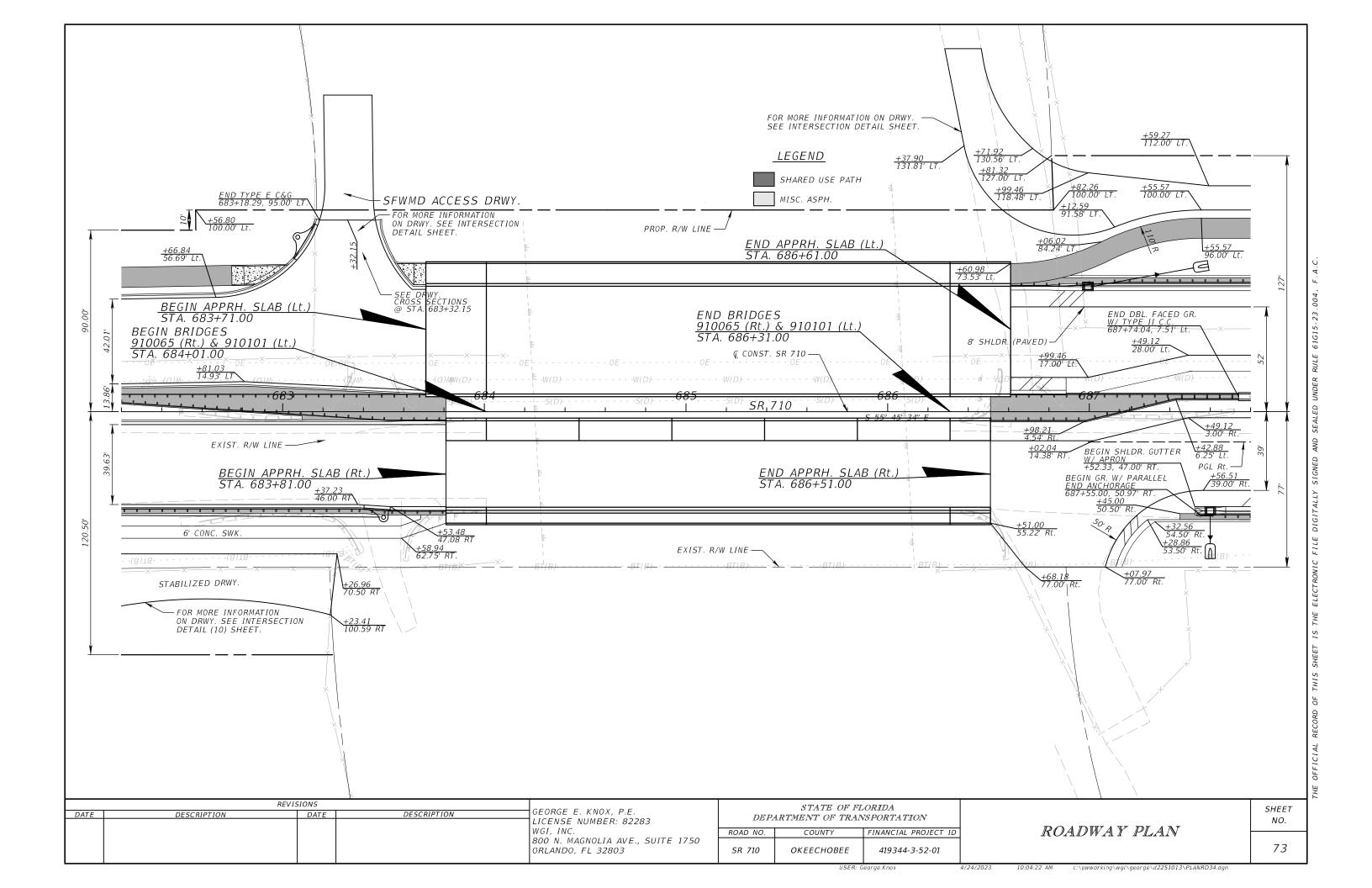


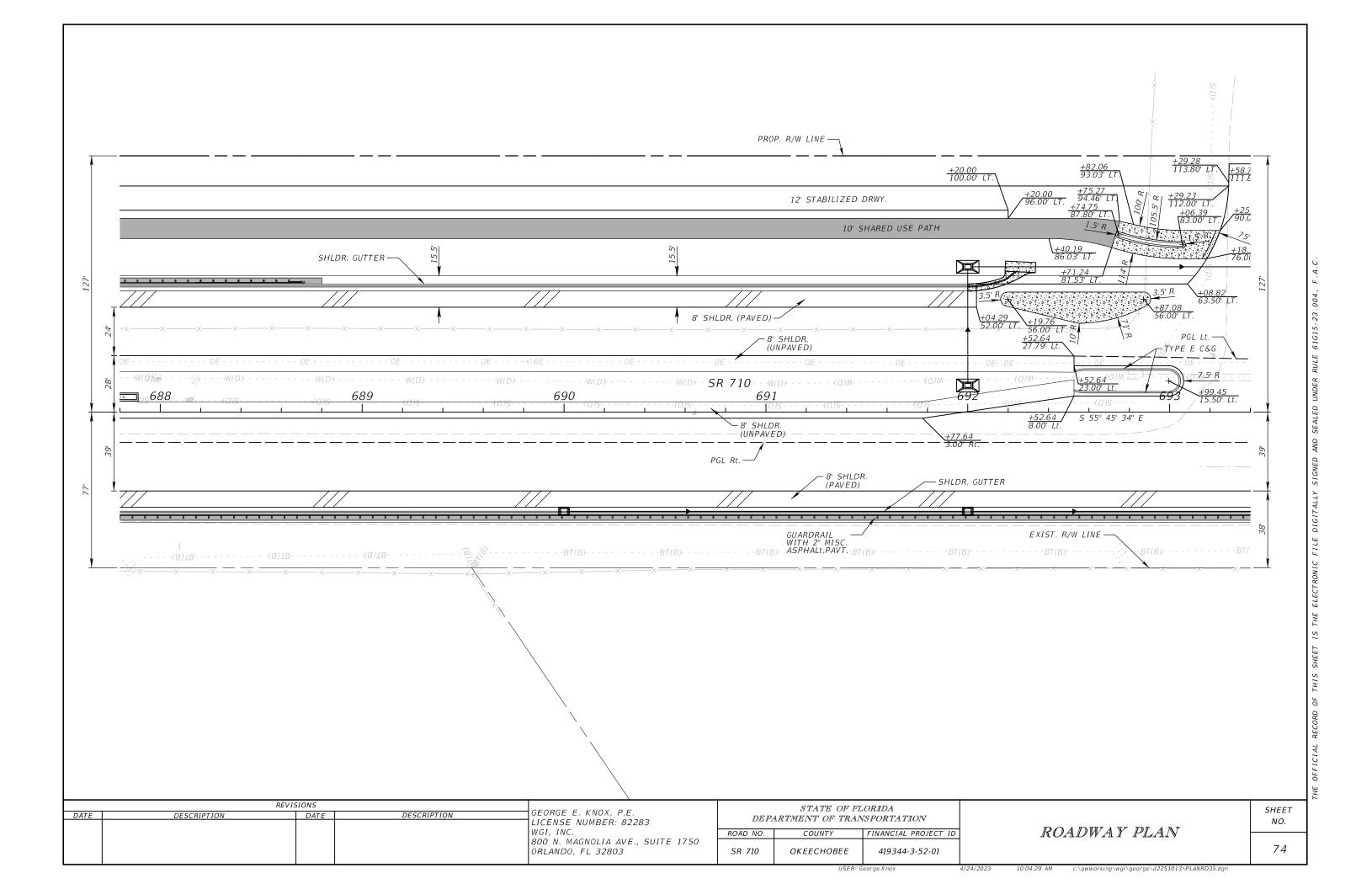


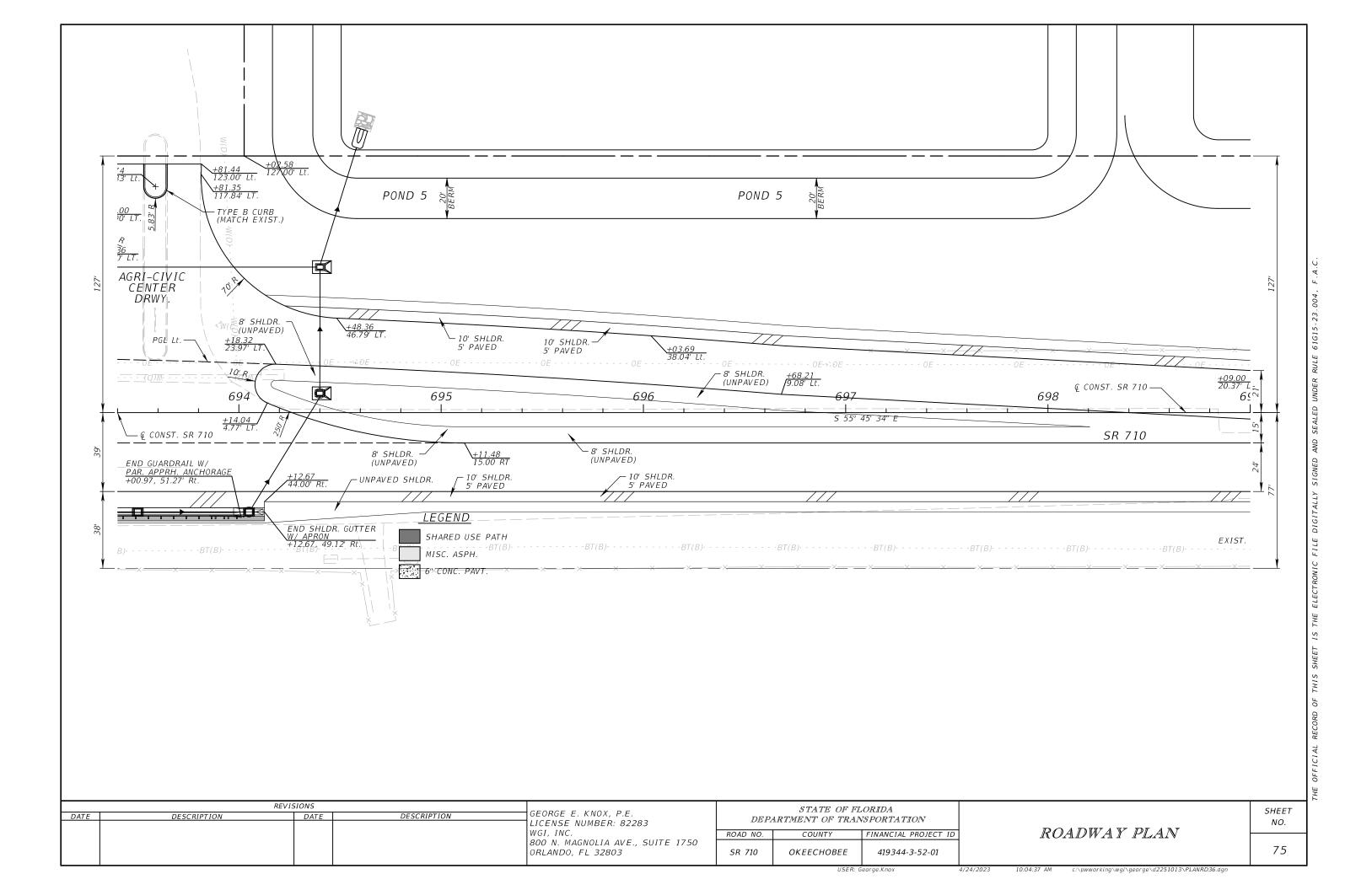


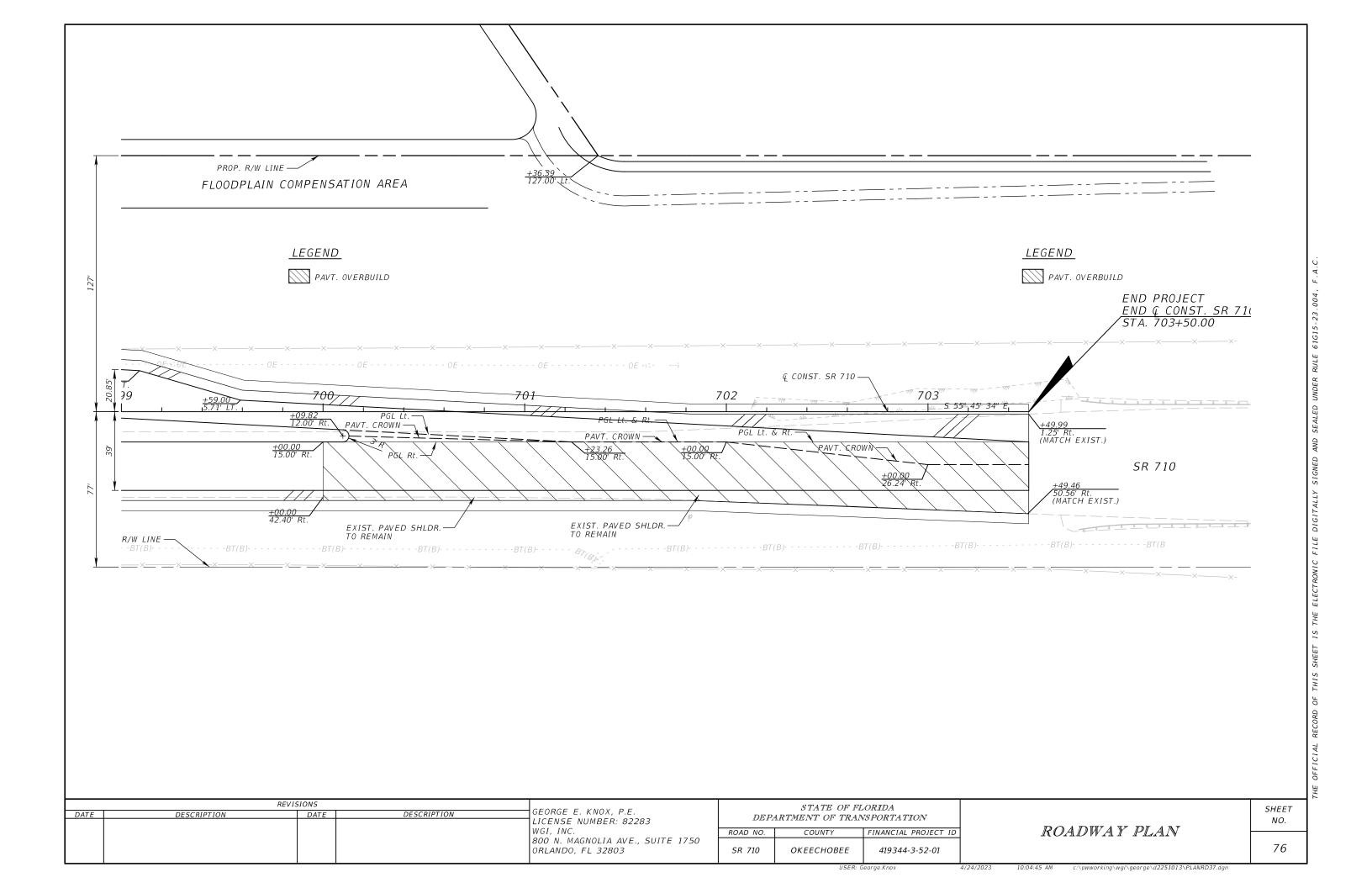


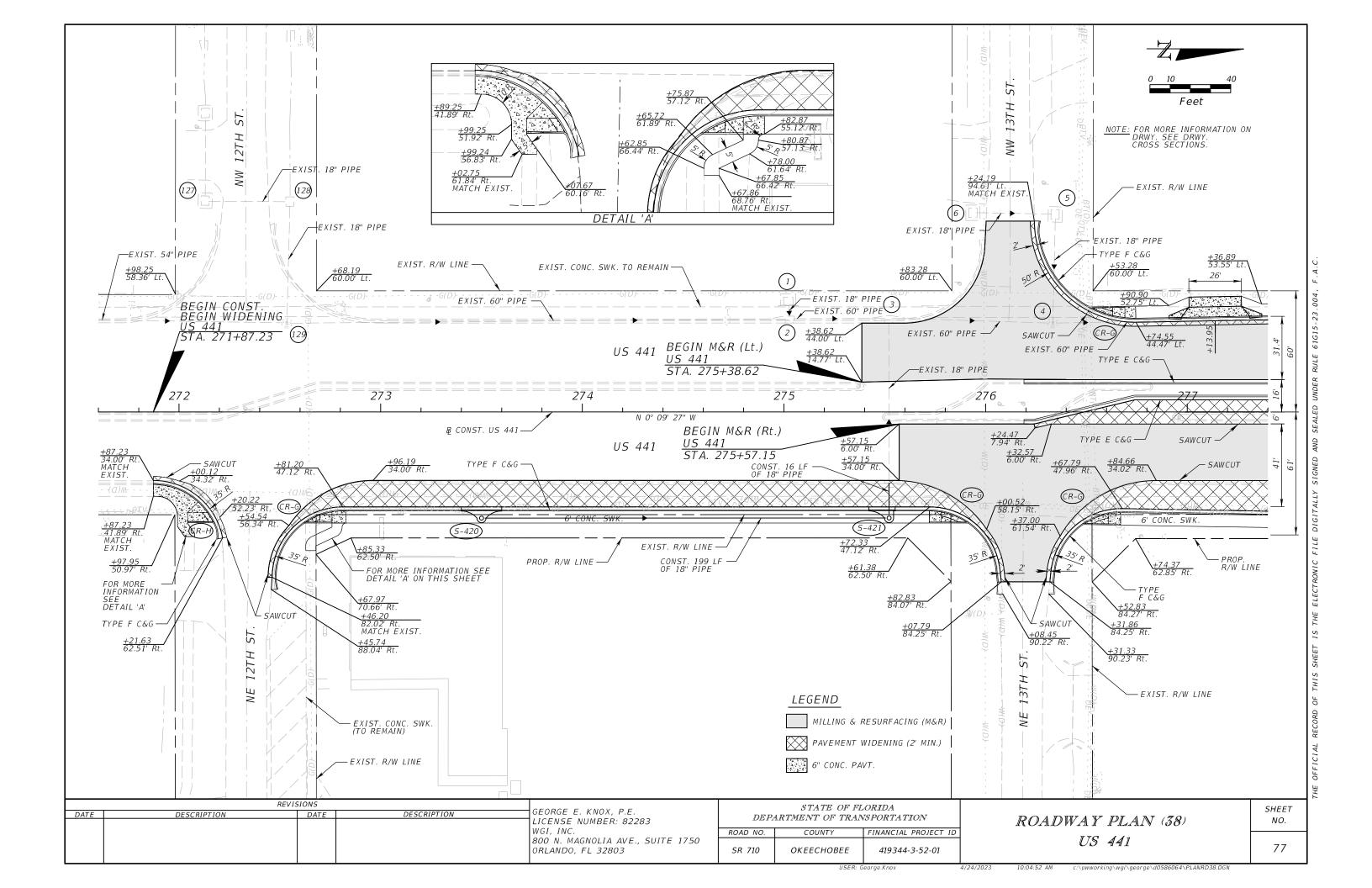


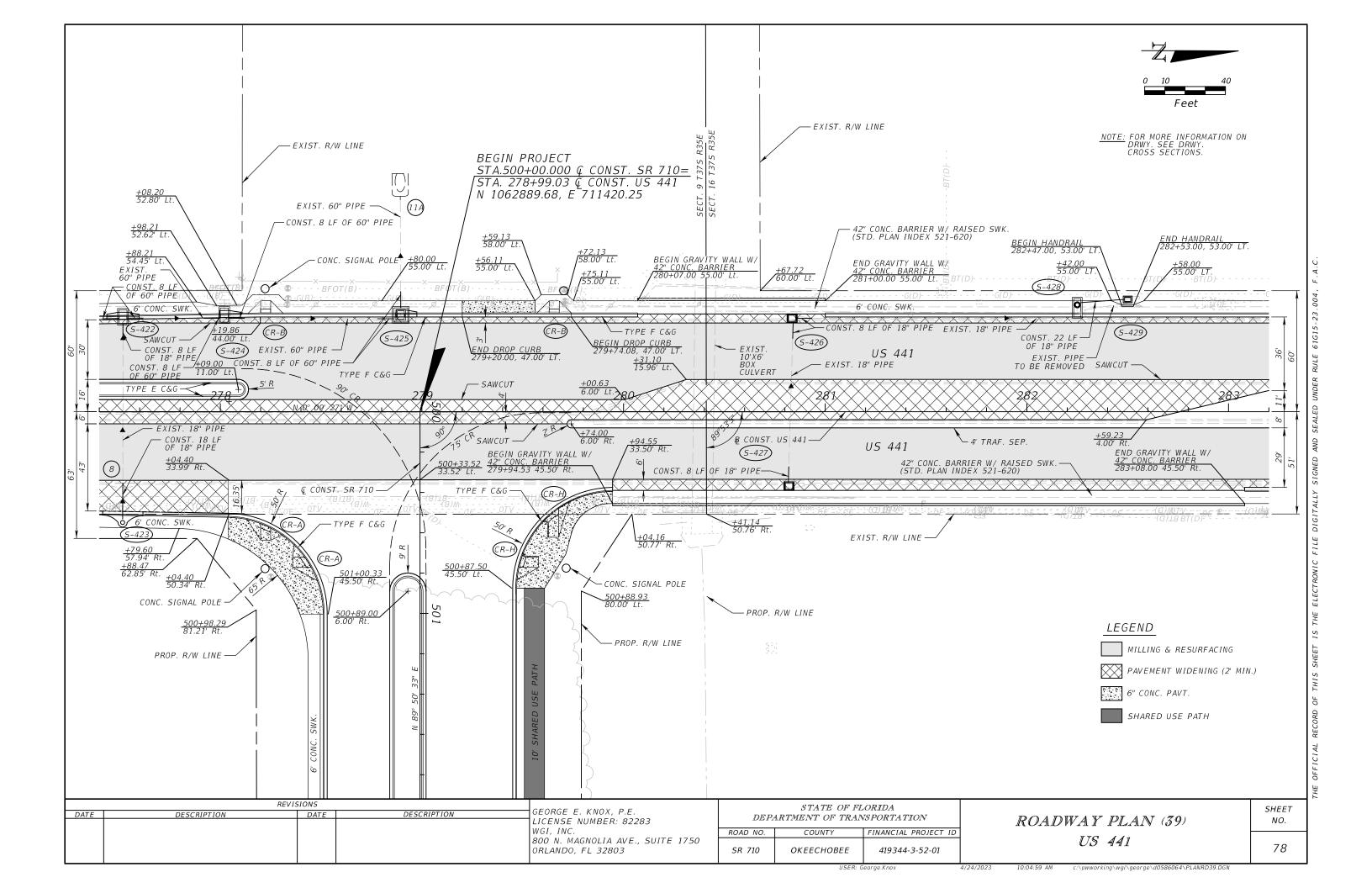


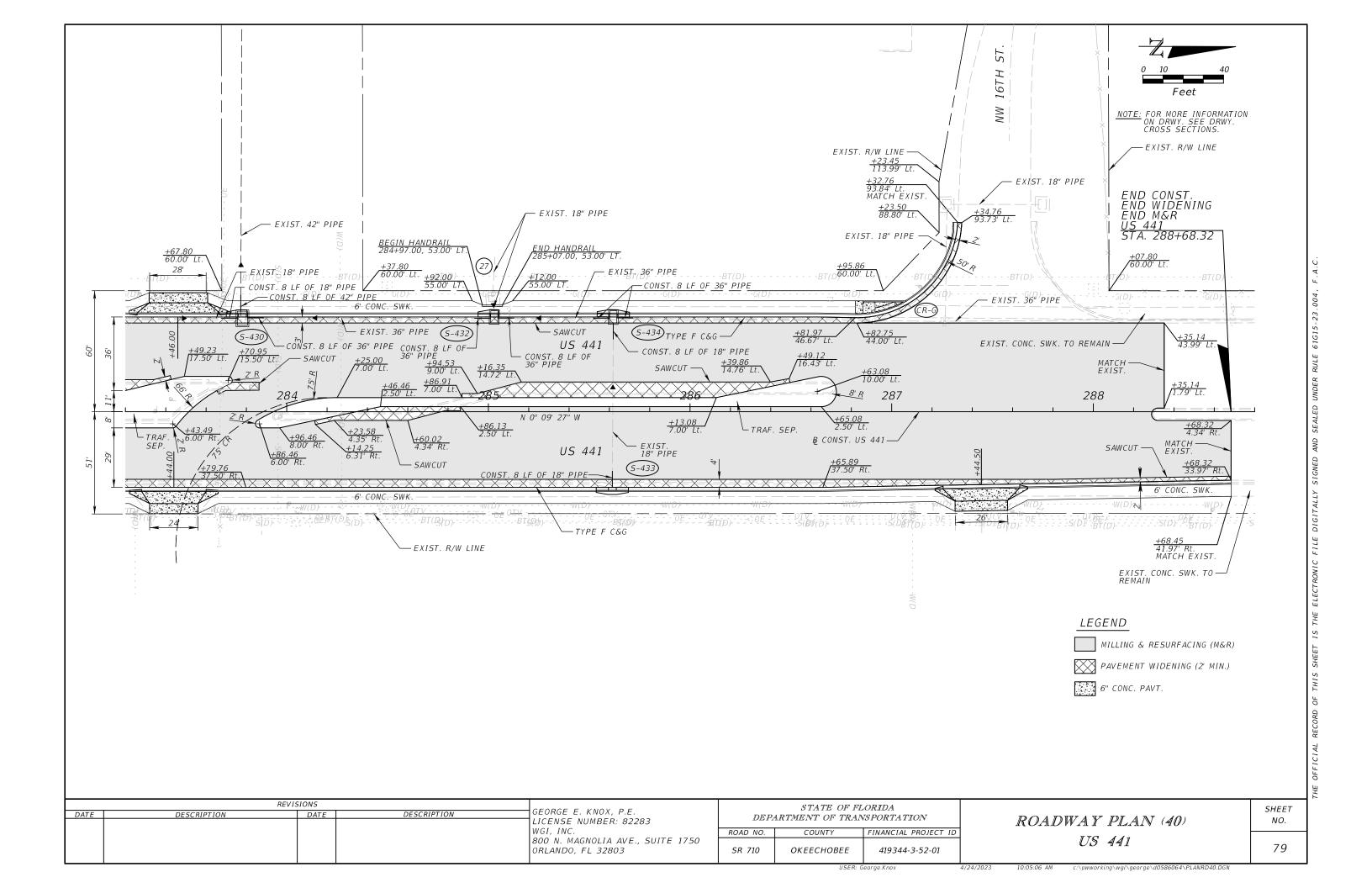


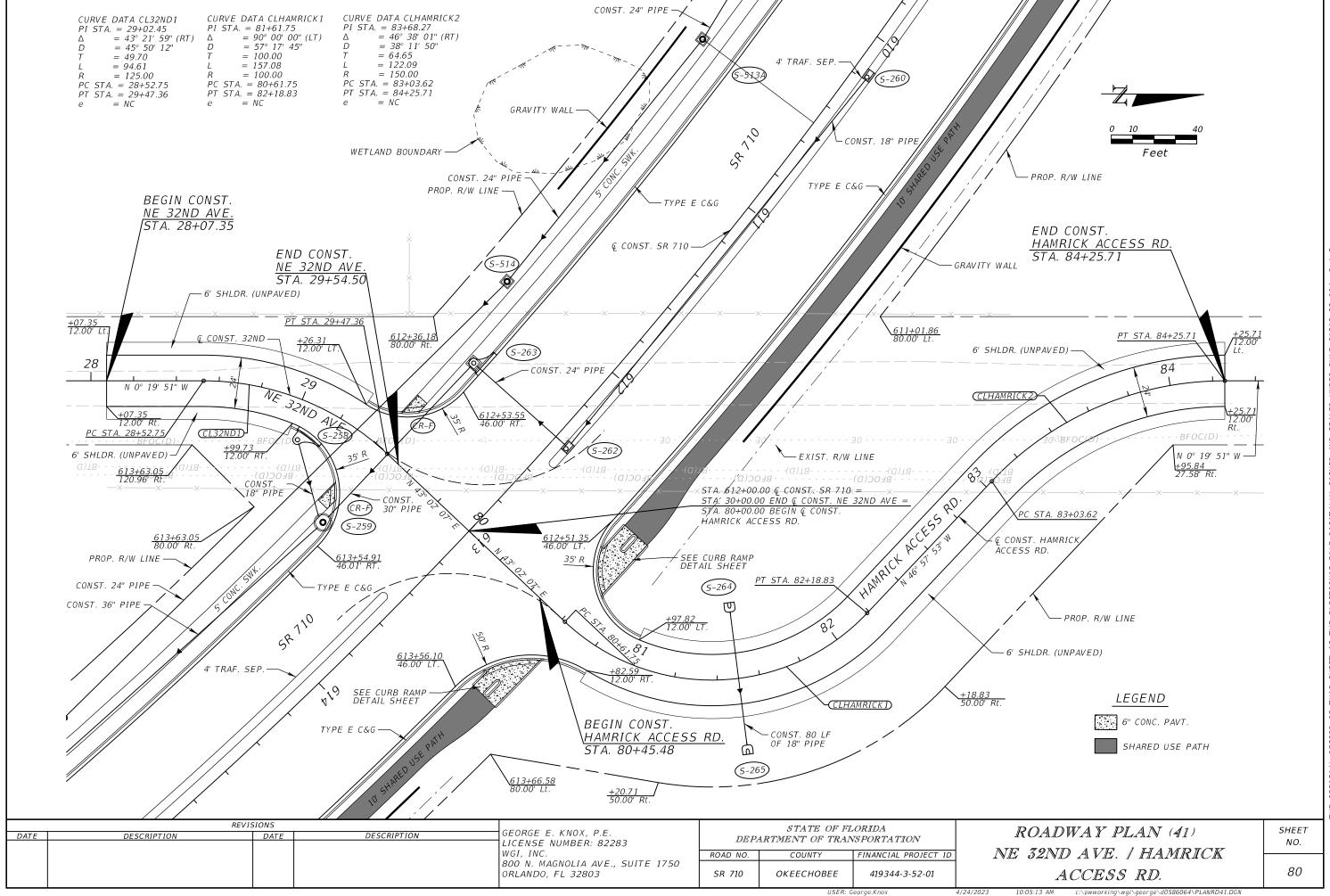


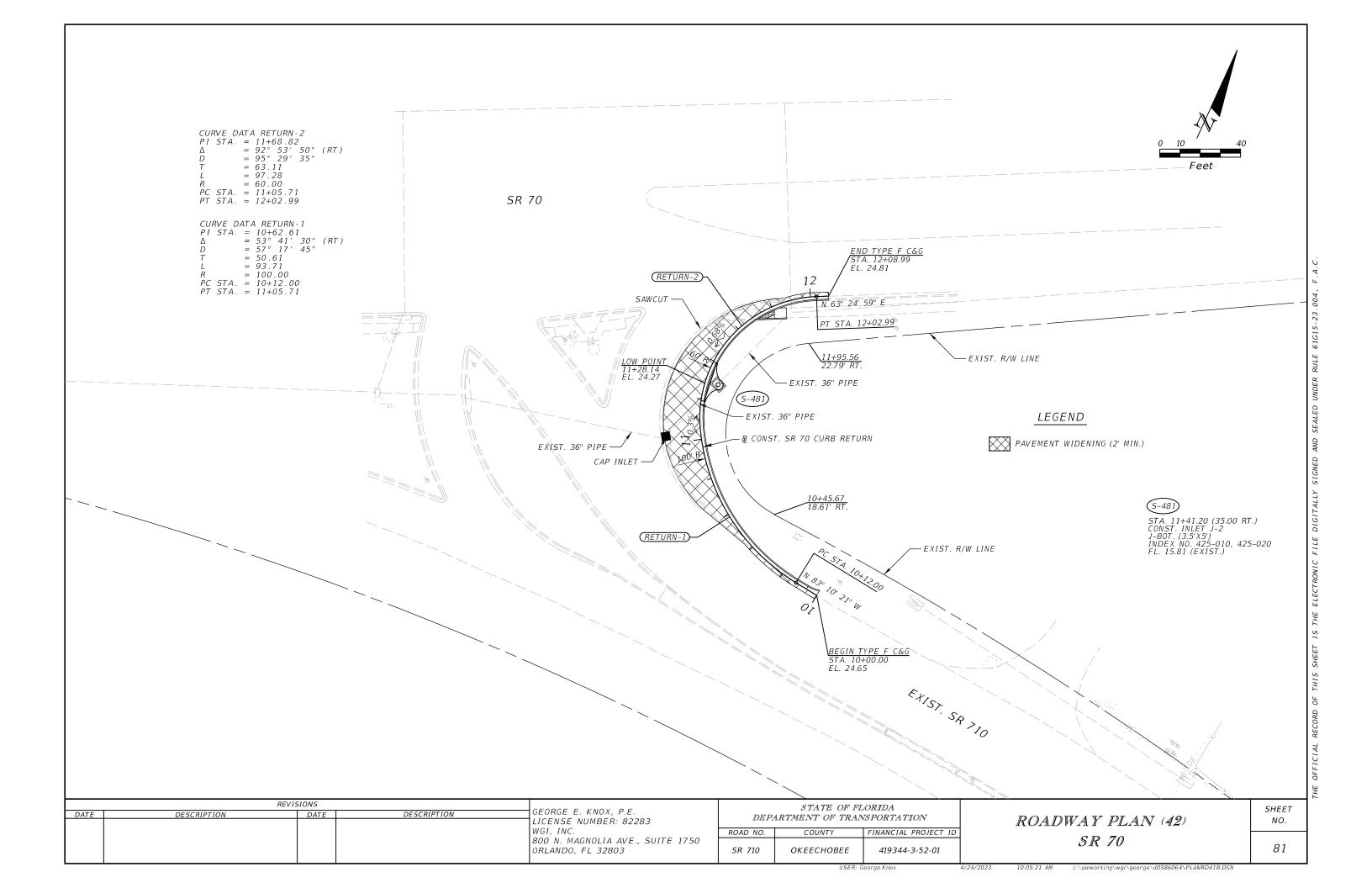


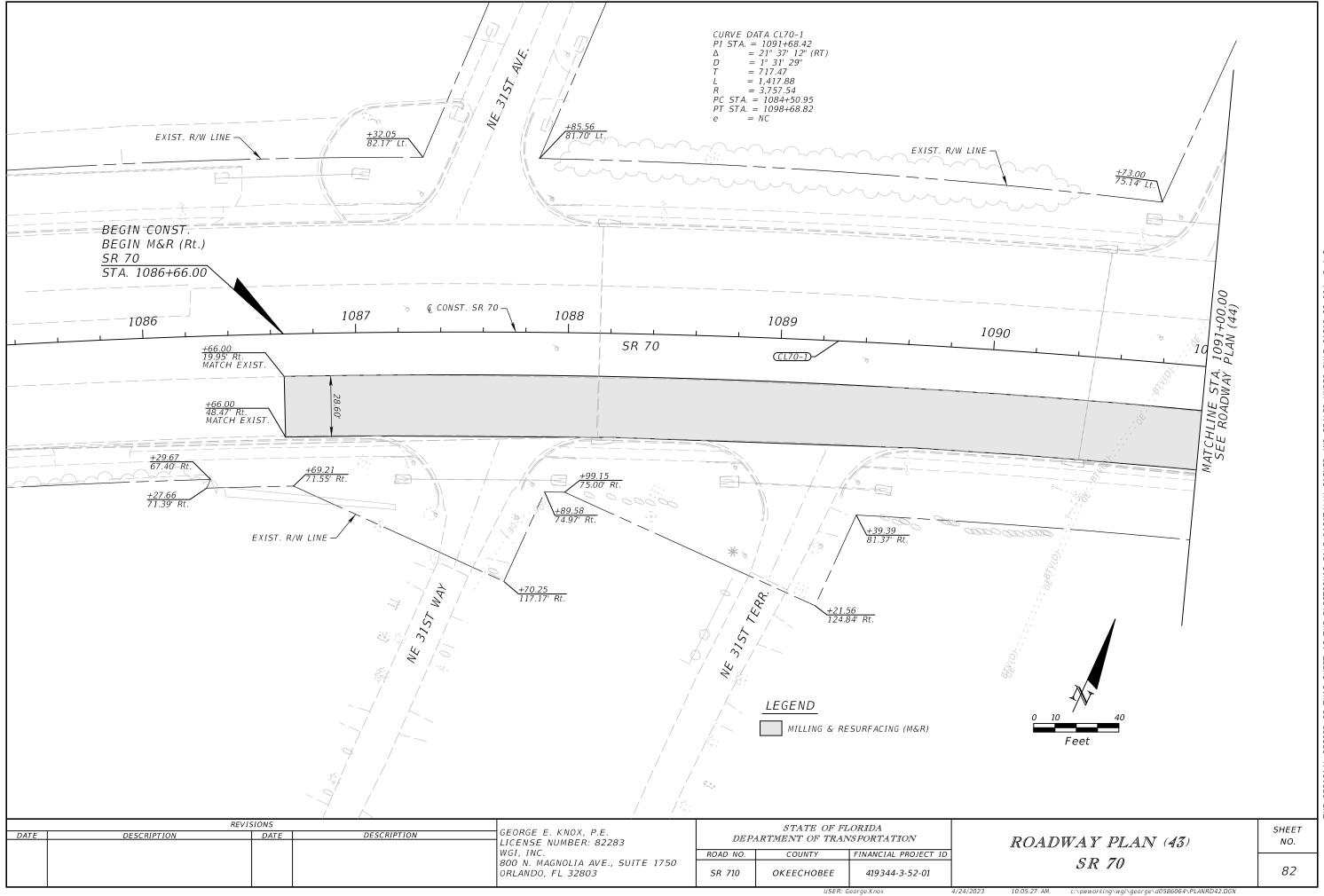


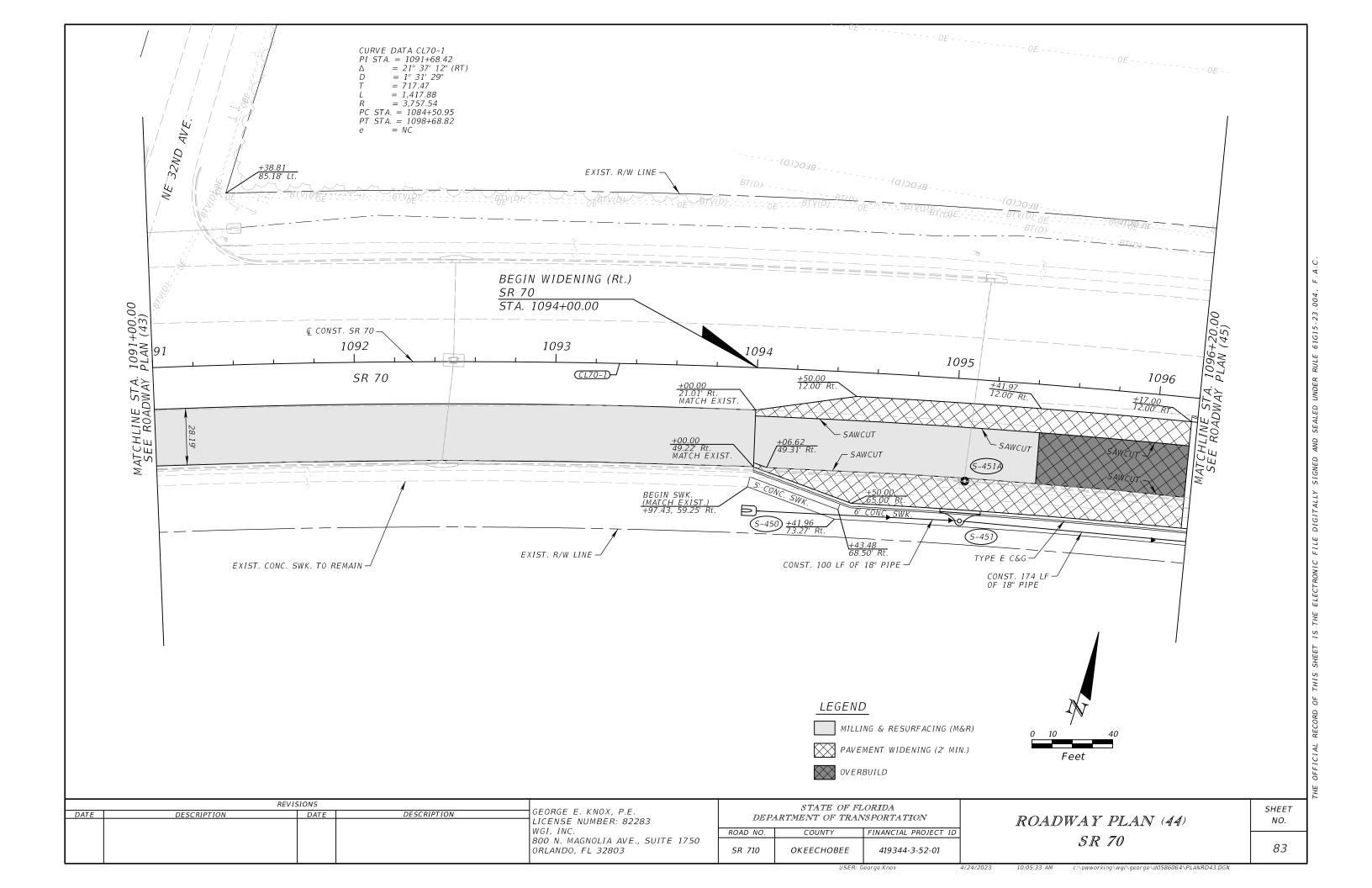


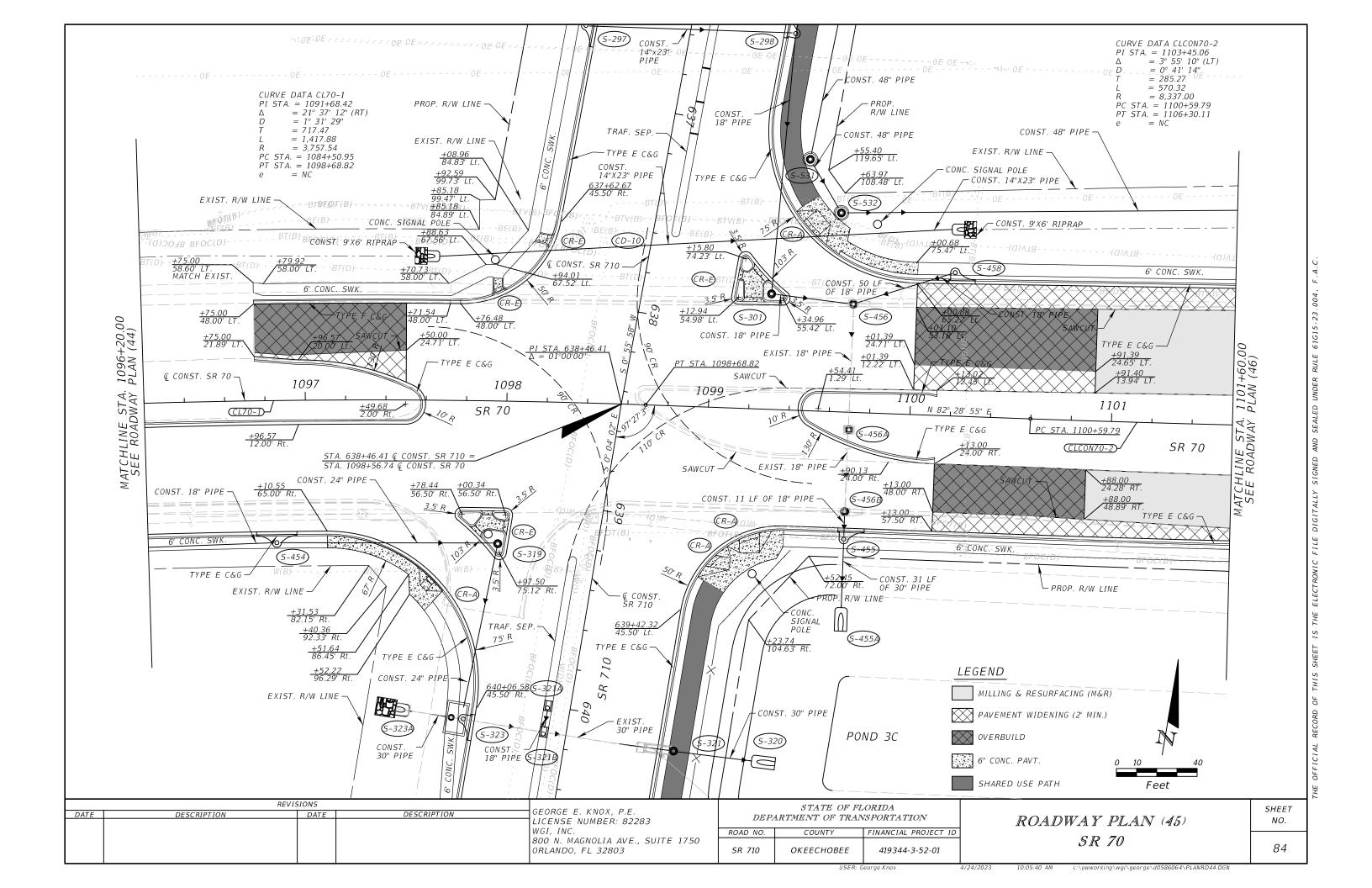


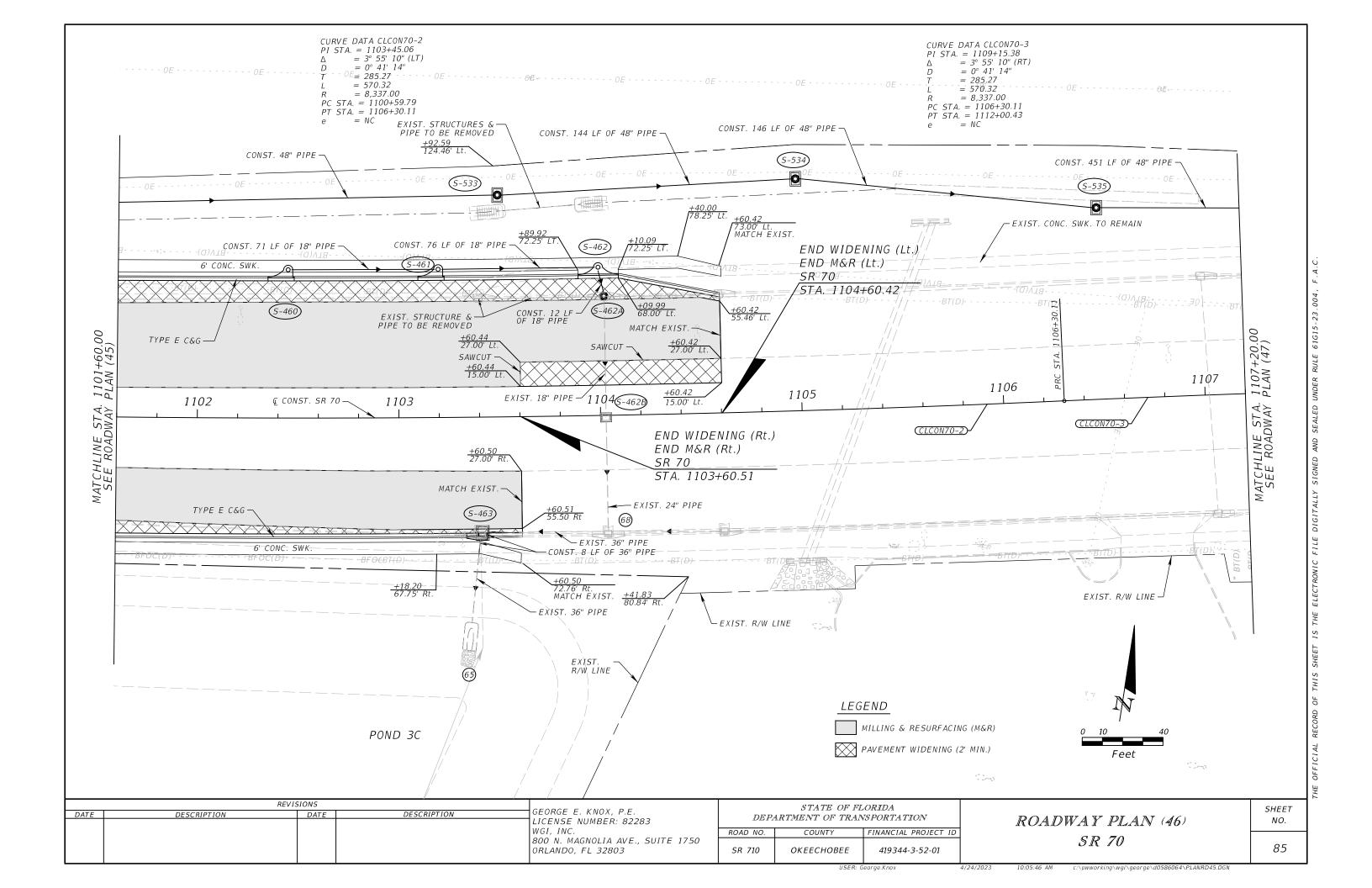


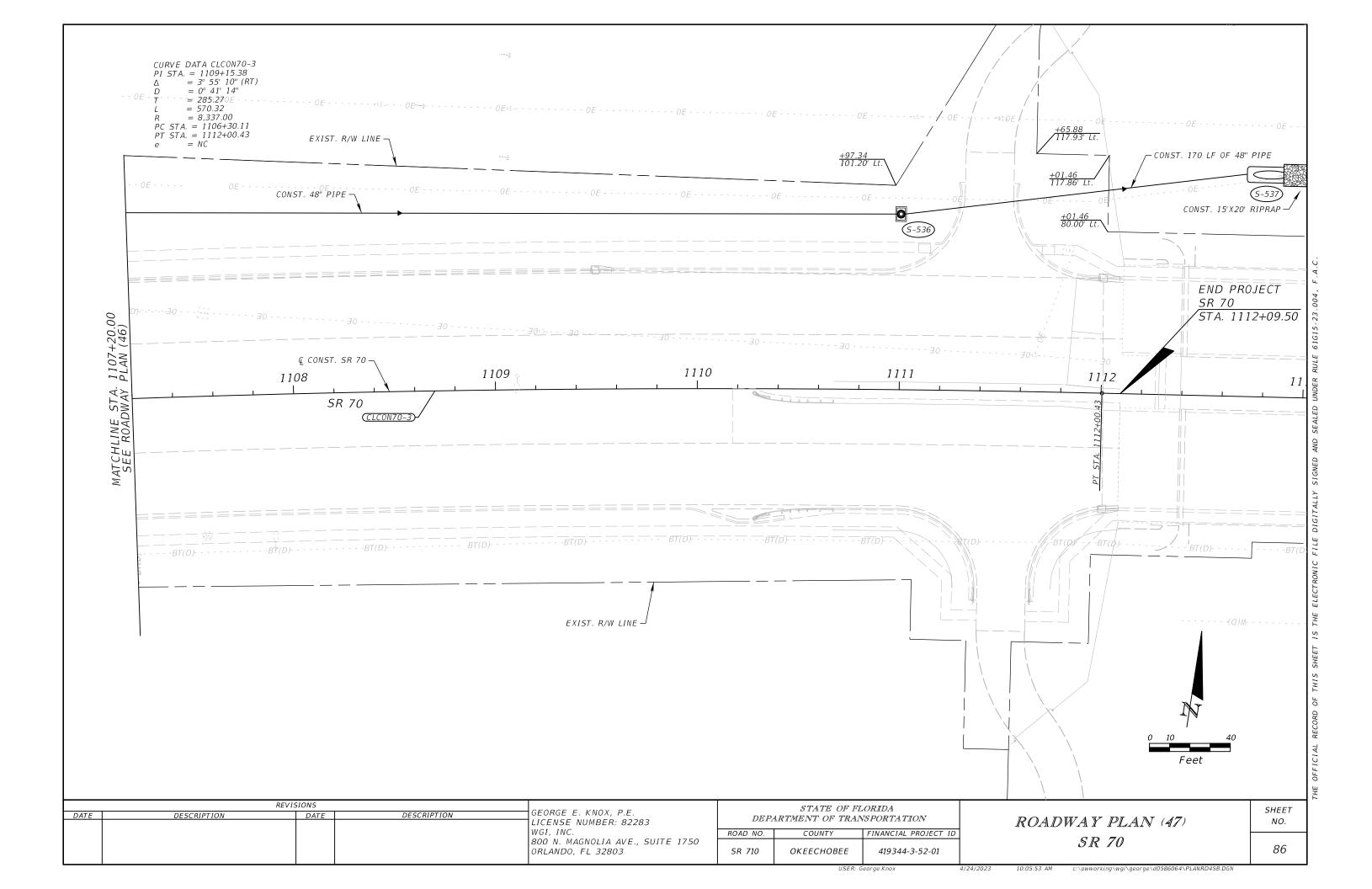


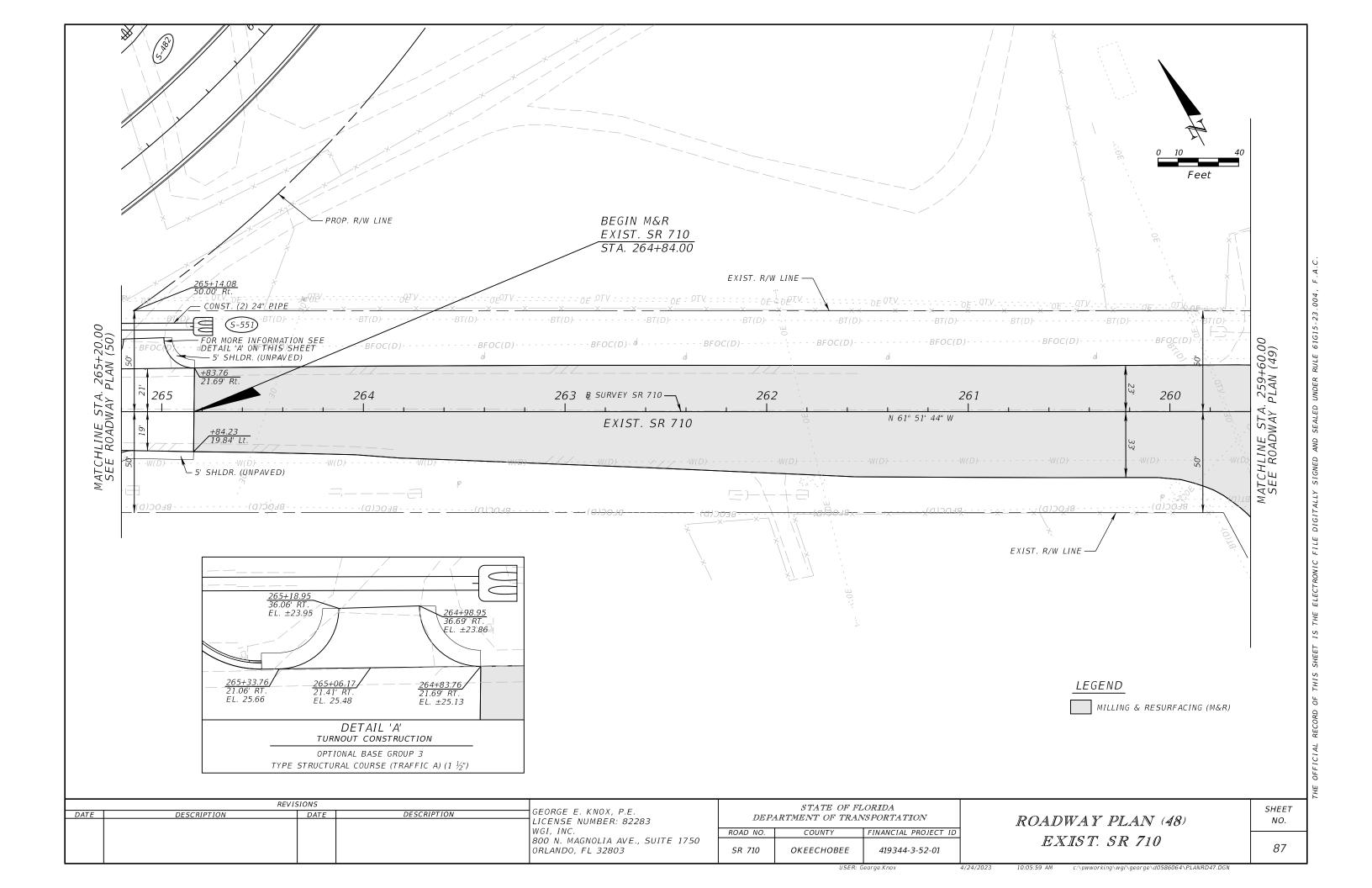


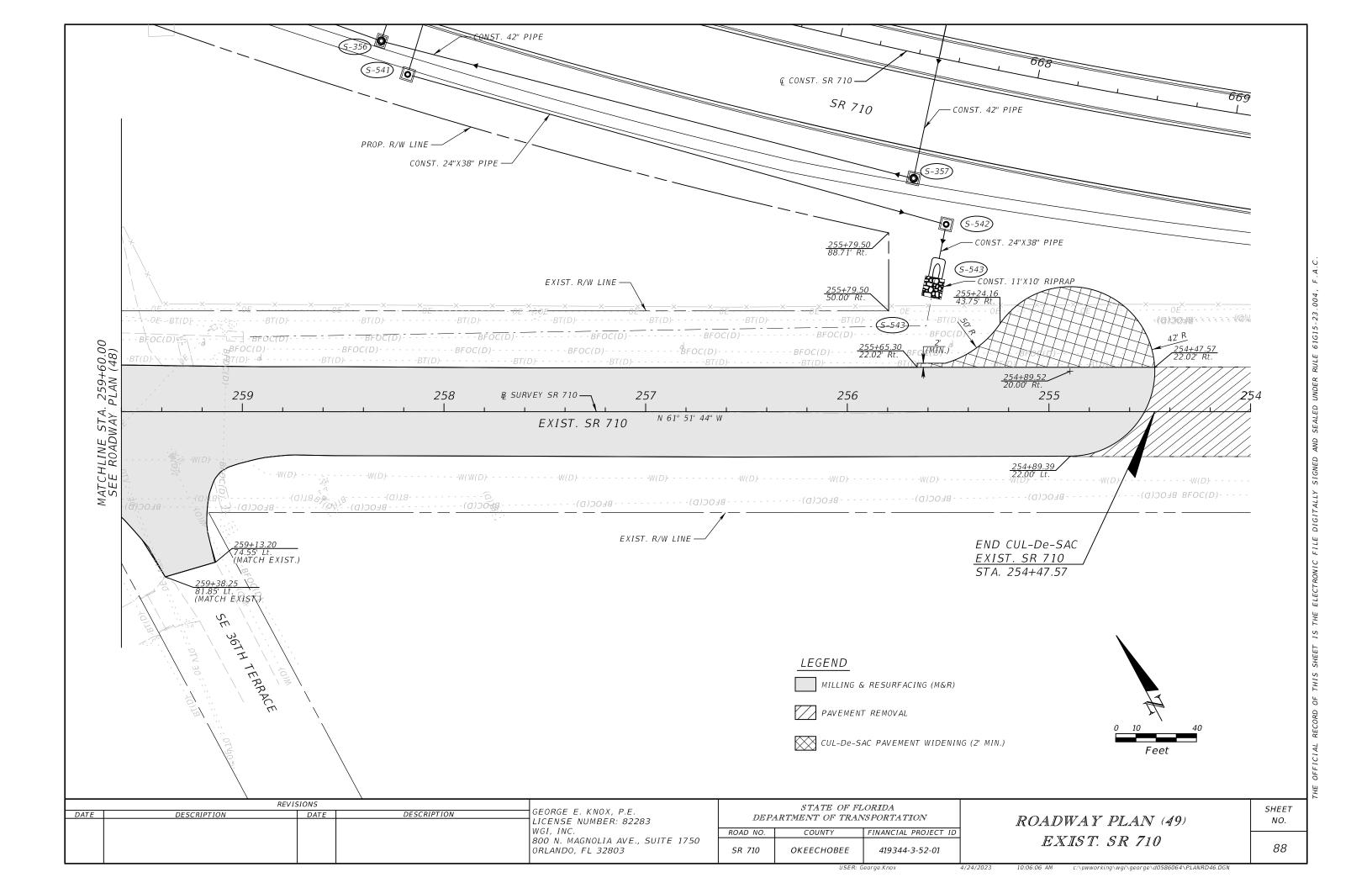


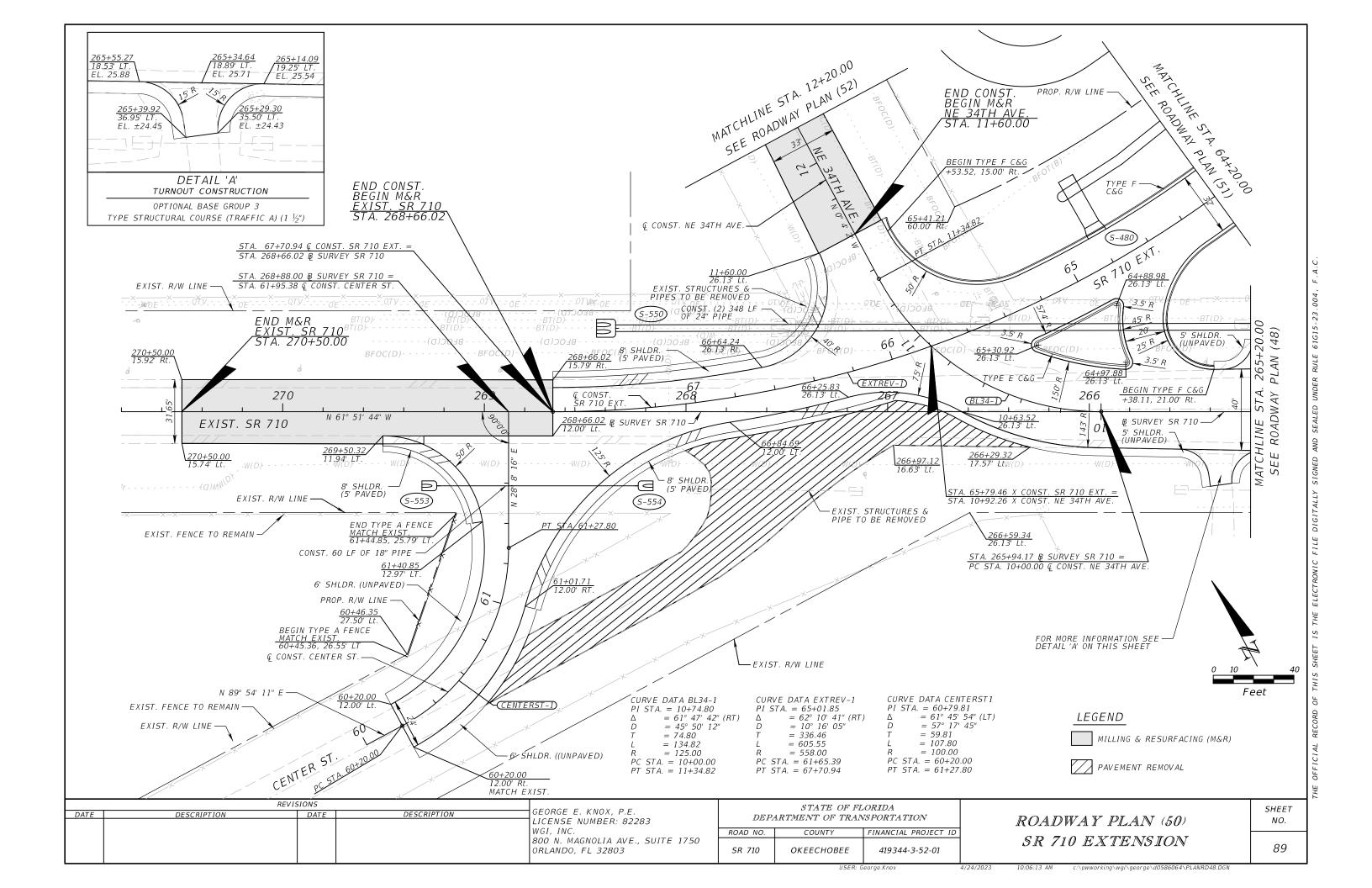


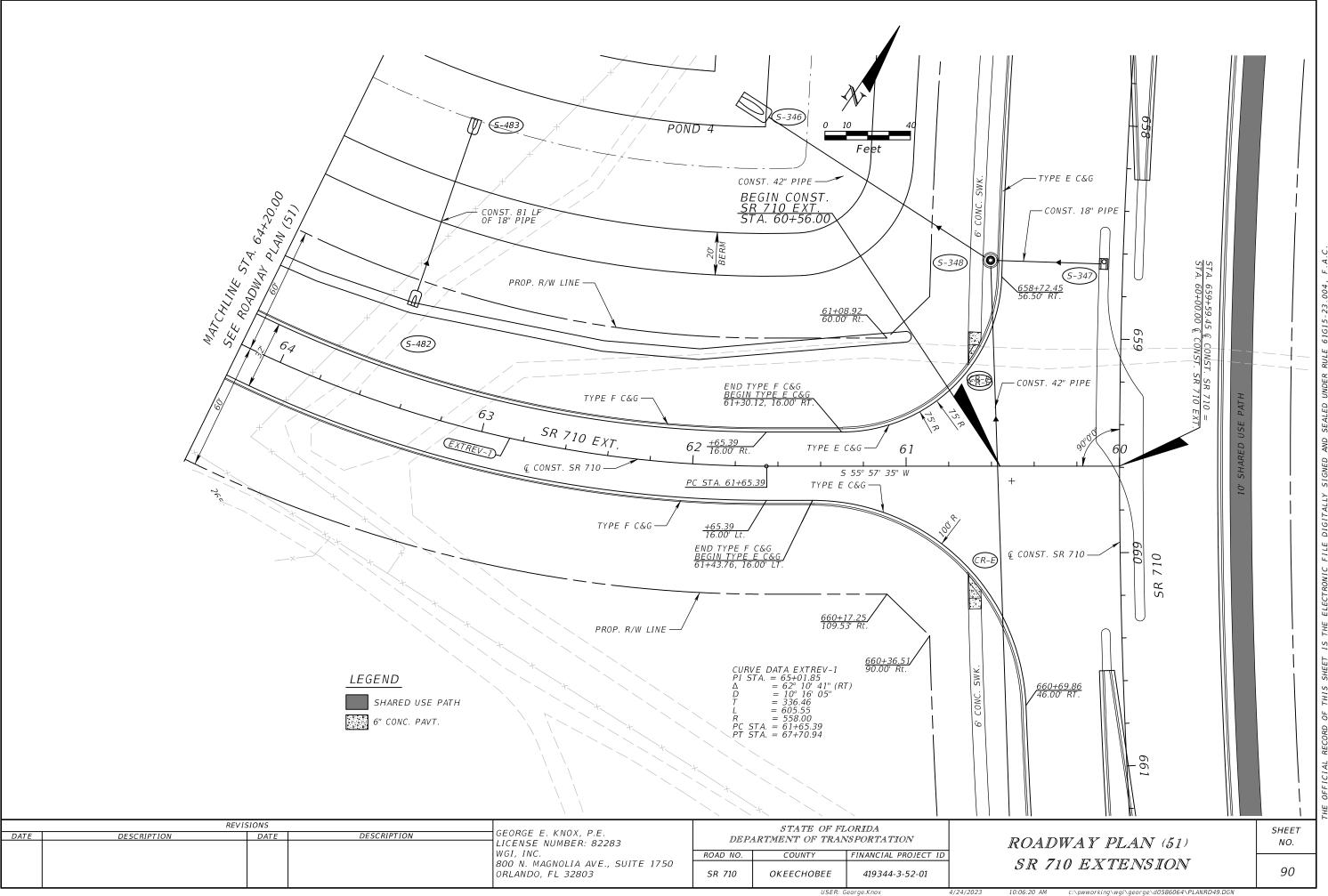


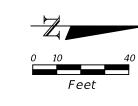


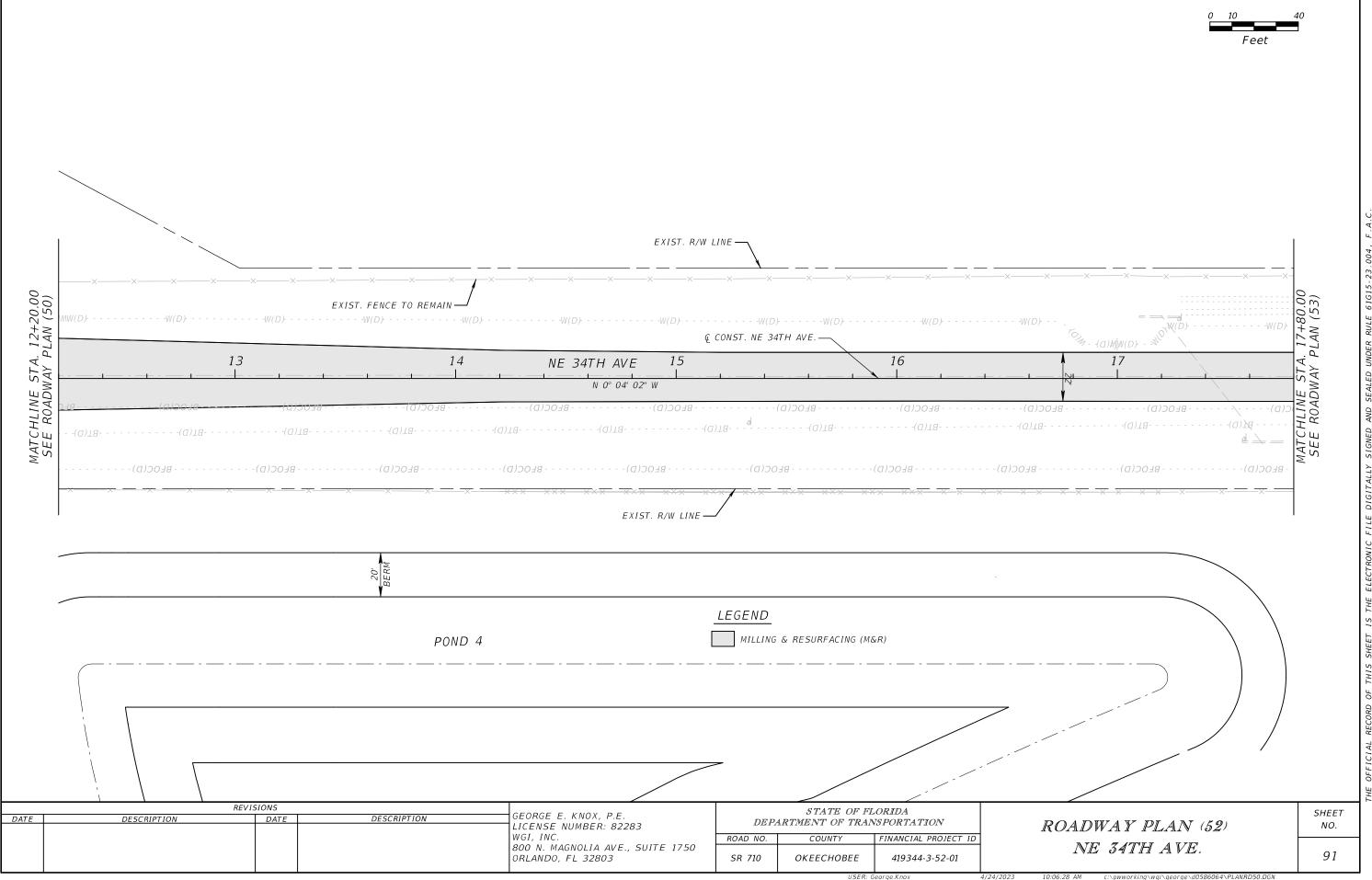


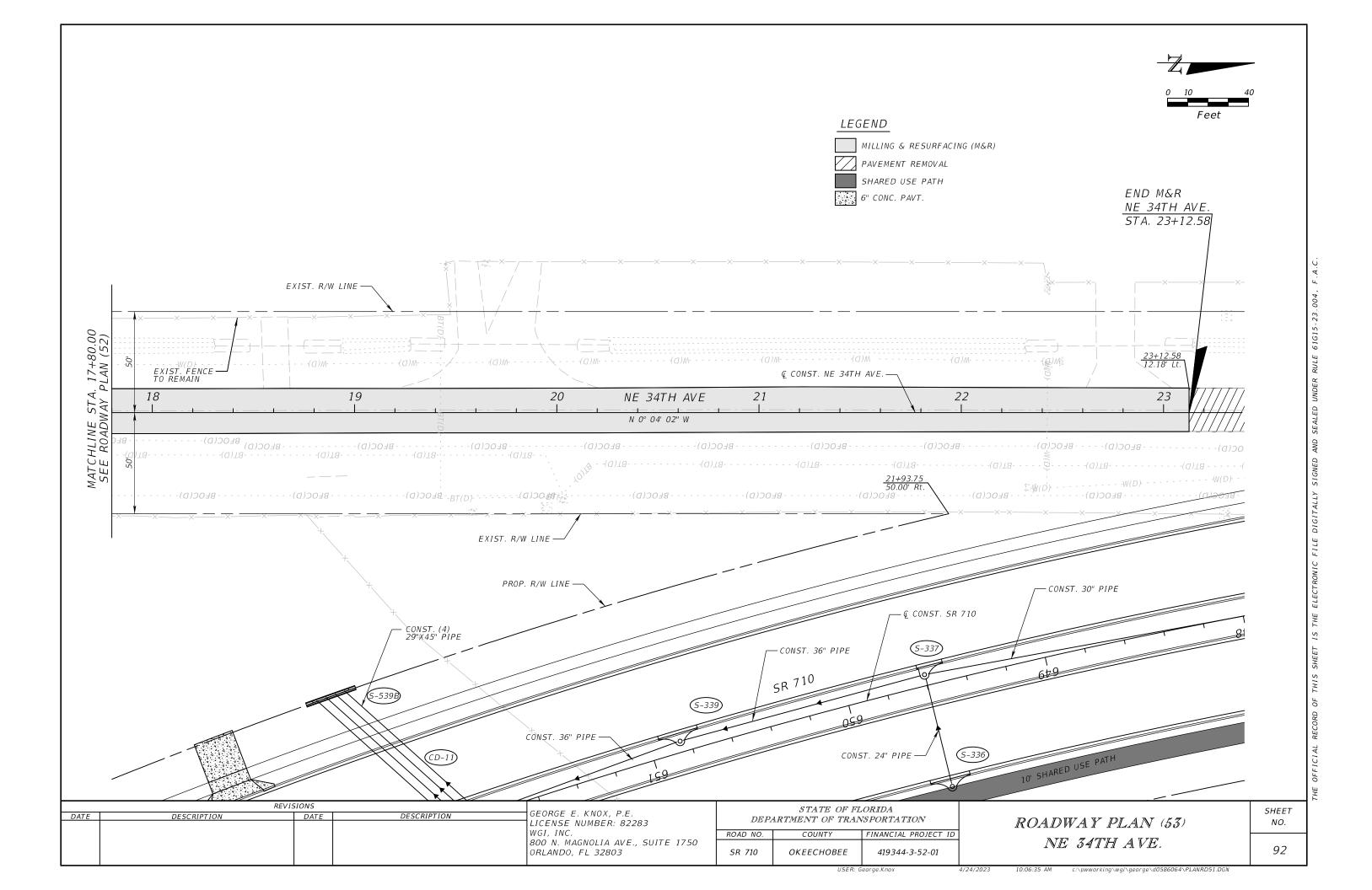


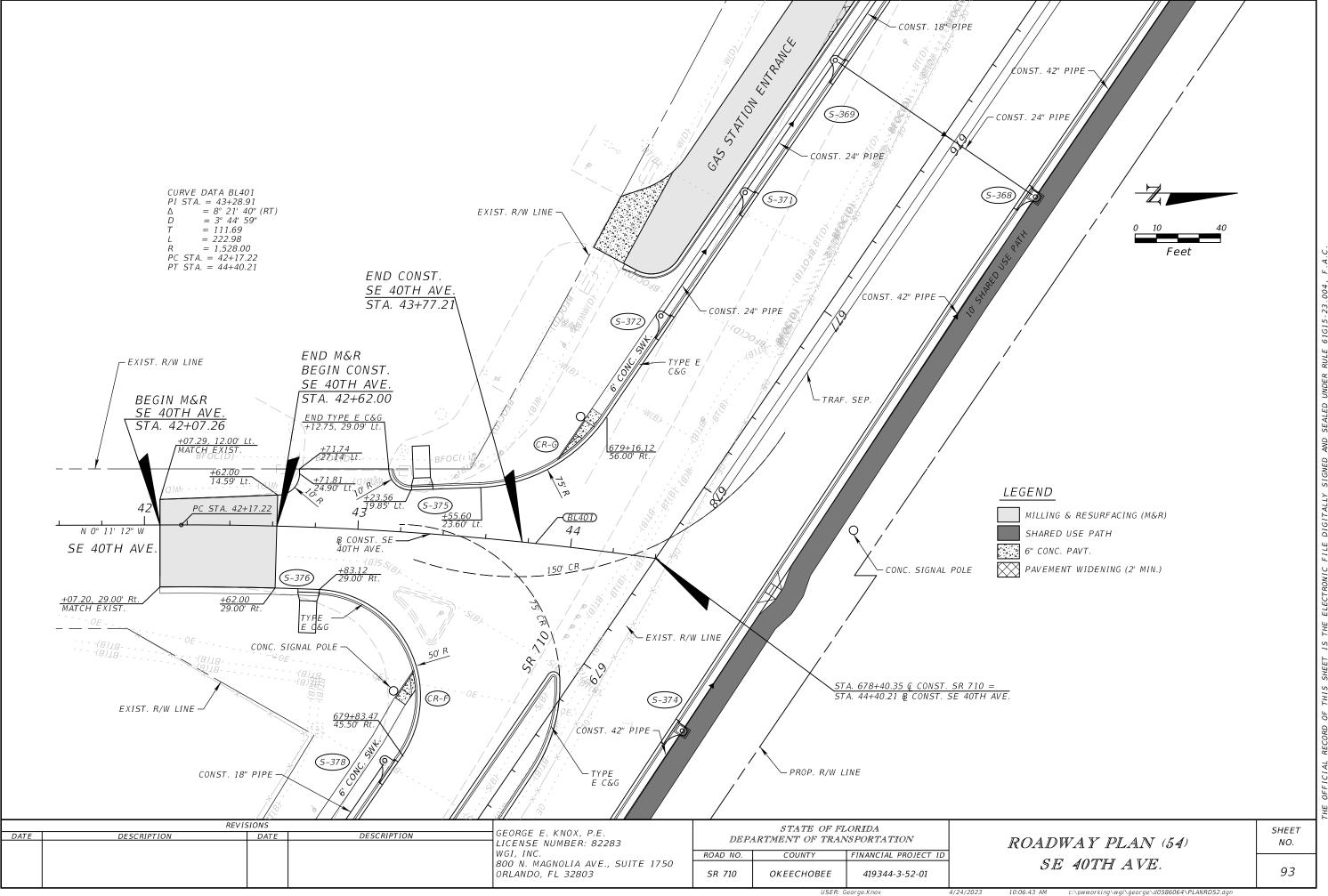












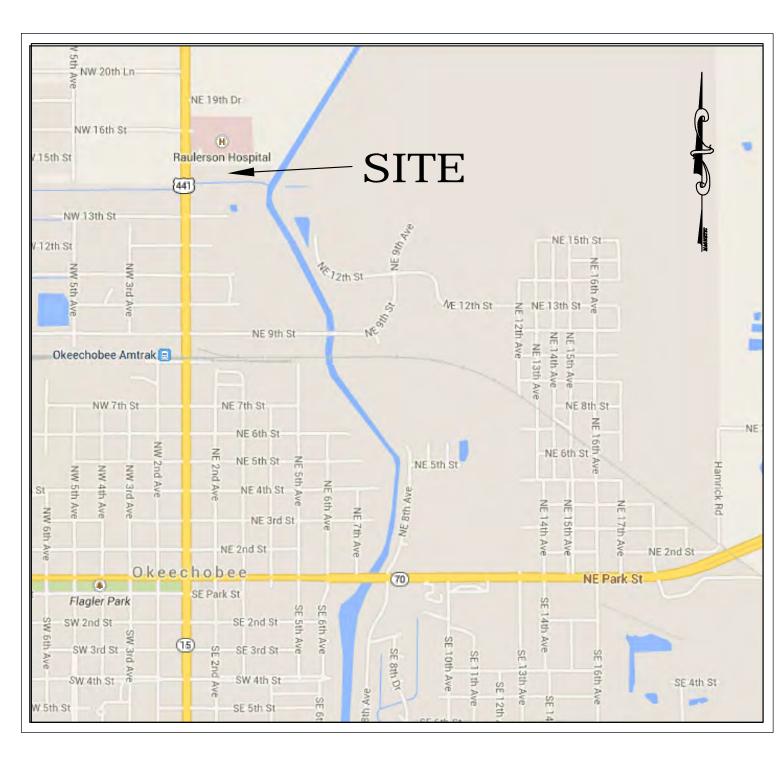
APPENDIX C
Referenced Okeechobee Health Care Facility Design Plans

# Construction Plans

# Okeechobee Healthcare Facility West Wing Expansion Seniors "R" Able, Inc.

Lying In Sec. 9, Township 37 South, Range 35 East Okeechobee County, Florida





**LOCATION MAP** SCALE: N.T.S.

### **INDEX OF SHEETS**

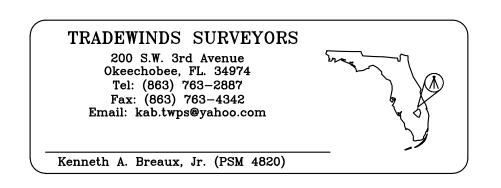
01 of 14	TITLE SHEET
02 of 14	OVERALL PLAN & KEY SHEET
03 of 14	EXISTING CONDITIONS / DEMOLITION PLAN
04 of 14	HORIZONTAL CONTROL, STRIPING & SIGNAGE PLAN (WEST VIEW)
05 of 14	HORIZONTAL CONTROL, STRIPING & SIGNAGE PLAN (SOUTH EAST VIEW
06 of 14	HORIZONTAL CONTROL, STRIPING & SIGNAGE PLAN (NORTH EAST VIEW
07 of 14	PAVING, GRADING & DRAINAGE PLAN (WEST VIEW)
08 of 14	PAVING, GRADING & DRAINAGE PLAN (SOUTH EAST VIEW)
09 of 14	PAVING, GRADING & DRAINAGE PLAN (NORTH EAST VIEW)
10 of 14	PAVING, GRADING & DRAINAGE DETAILS
11 of 14	UTILITY DETAILS
12 of 14	UTILITY DETAILS
13 of 14	UTILITY DETAILS
14 of 14	GENERAL NOTES & SPECIFICATIONS

### Survey Data



CERTIFICATE OF AUTHORIZATION LB# 7673

ELEVATIONS SHOWN HEREON ARE IN FEET RELATIVE TO THE NATIONAL GEODETIC VERTICAL DATUM (NGVD), 1929 ADJUSTMENT

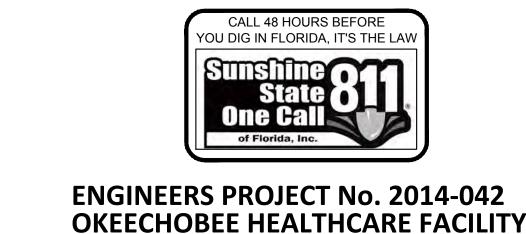


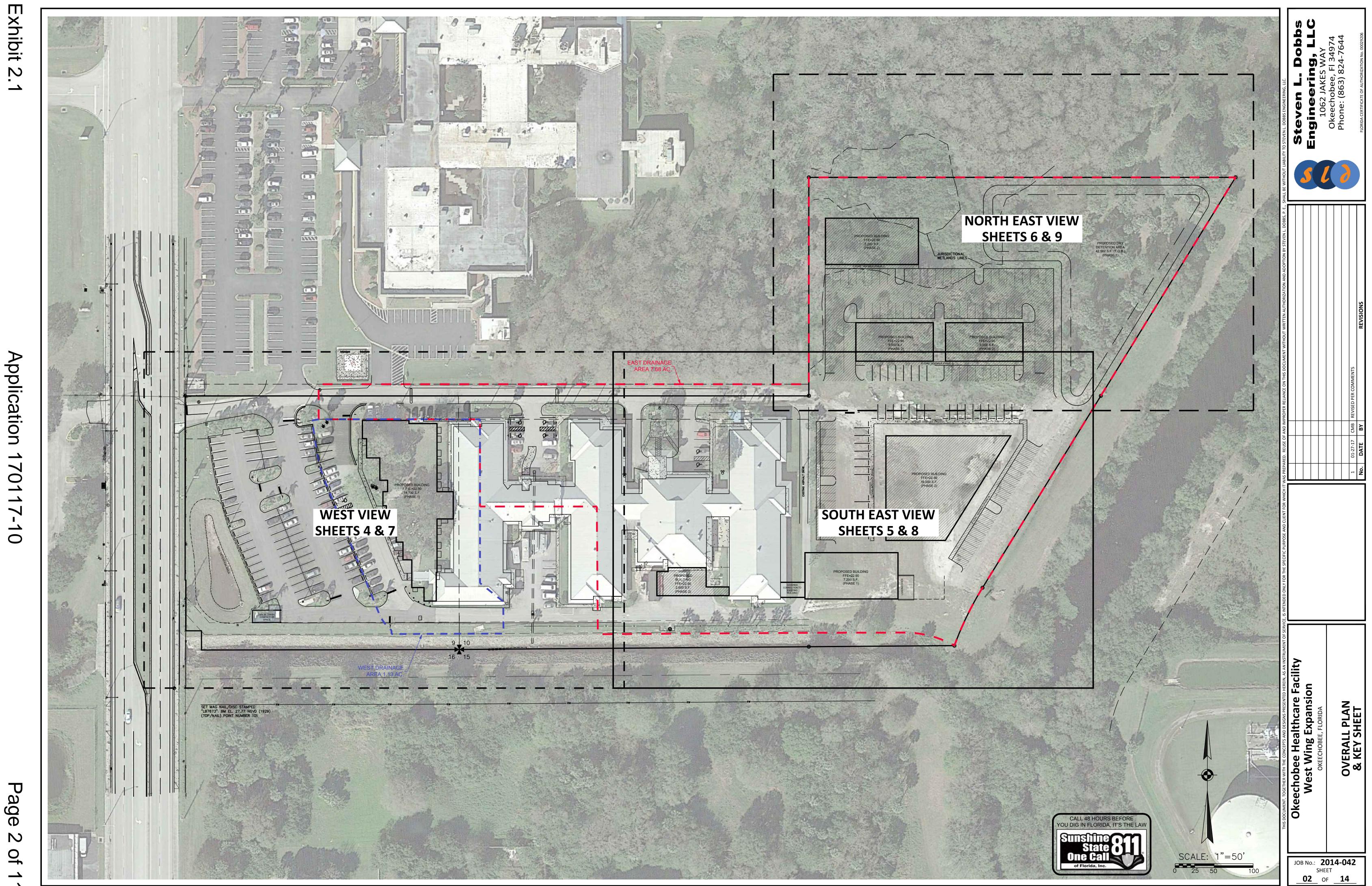


## Steven L. Dobbs Engineering, LLC **Consulting Engineers**

1062 Jakes Way - Okeechobee, FL 34974

Phone: (863) 824-7644 FLORIDA CERTIFICATE OF AUTHORIZATION No. 00029206





**03** OF **14** 

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CALL 48 HOURS BEFORE YOU DIG IN FLORIDA, IT'S THE LAY

> JOB No.: **2014-042** SHEET

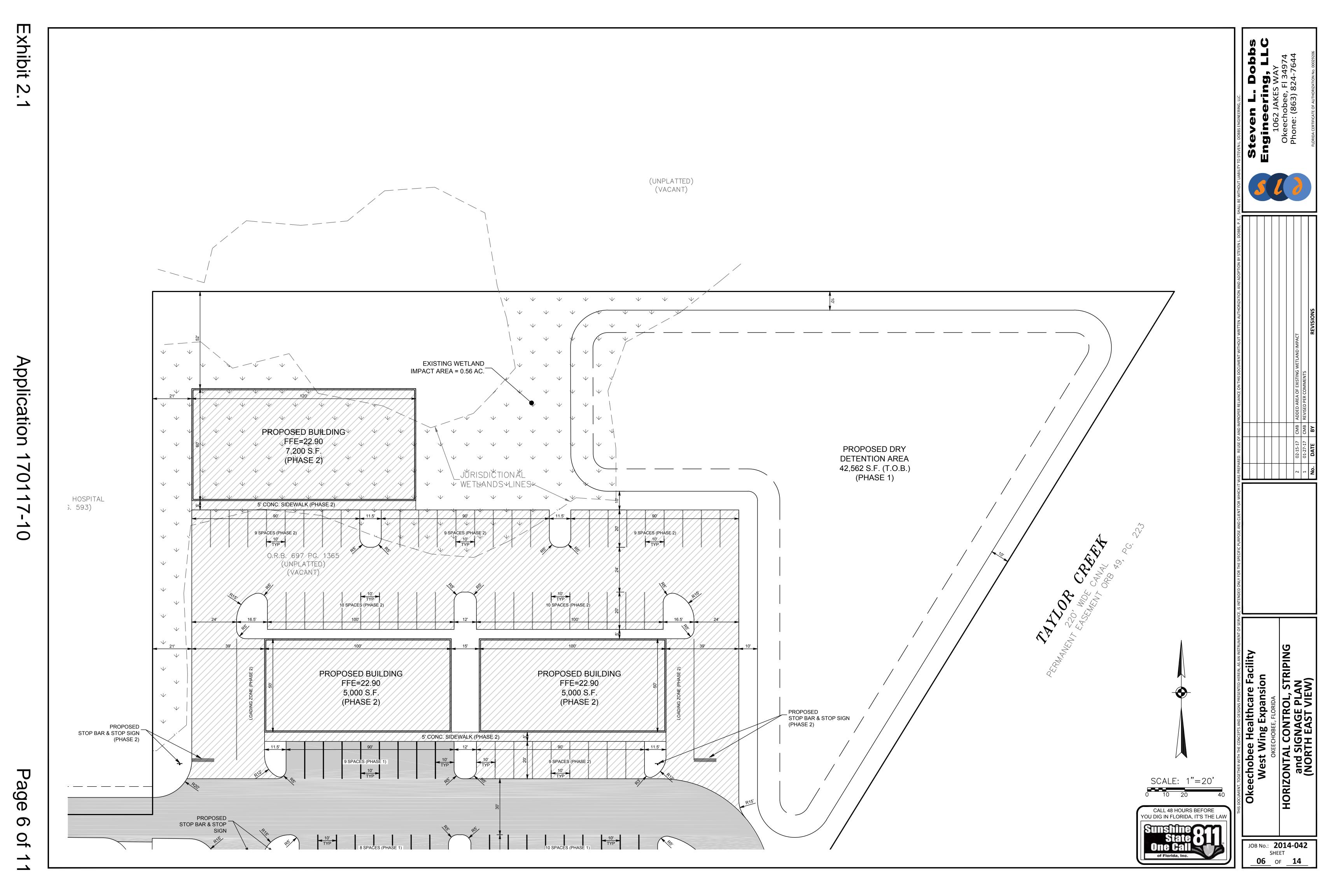
**05** OF **14** 

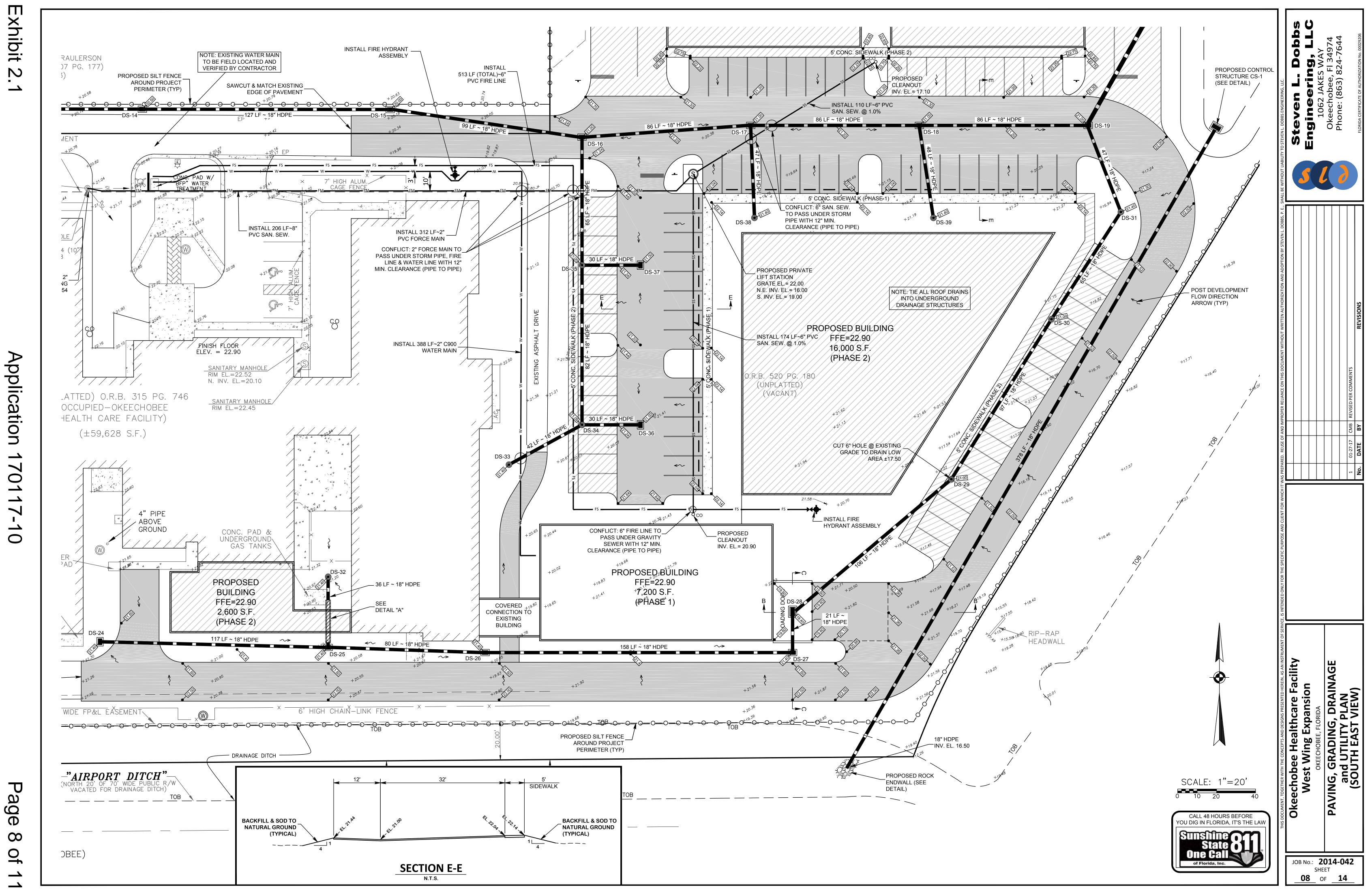
-"AIRPORT DITCH"

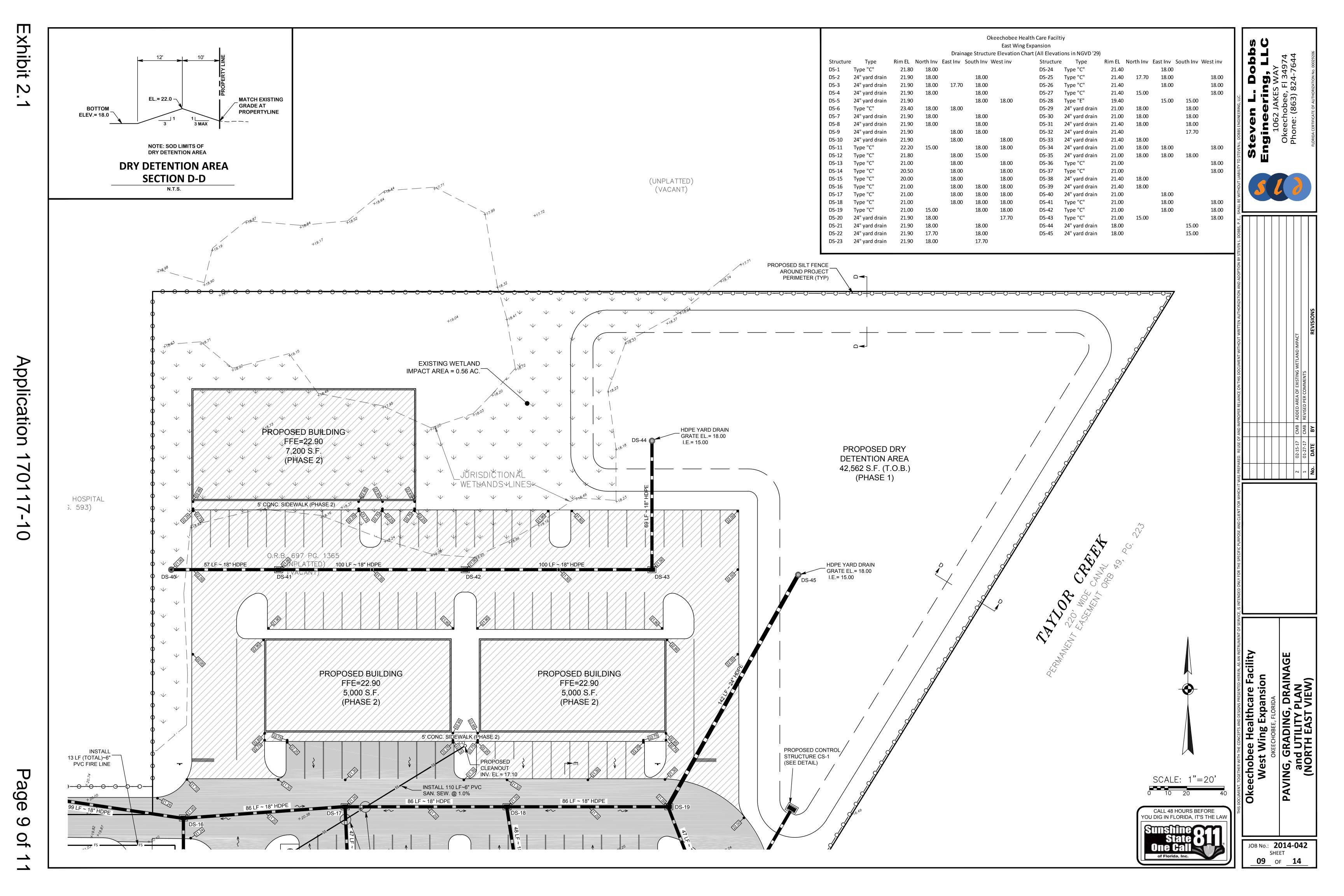
(NORTH 20' OF 70' WIDE PUBLIC R/W
VACATED FOR DRAINAGE DITCH)

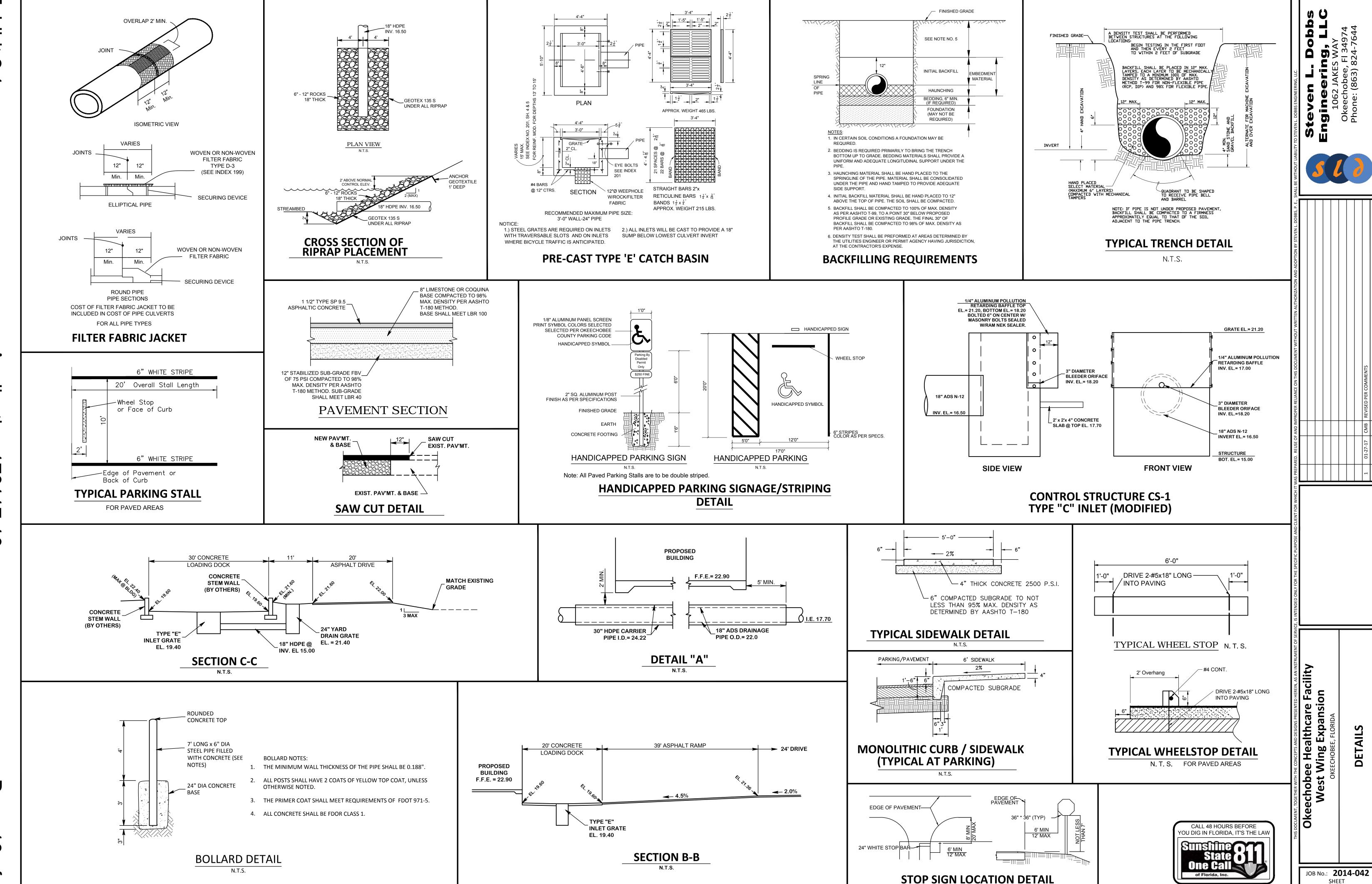
DBEE)

— DRAINAGE DITCH —









**10** OF **14** 

# ag 9

#### GENERAL NOTES

- 1. Contractor is responsible for checking actual site conditions before starting
- 2. Any discrepancies on the drawings shall be brought to the attention of the engineer before commencing work.
- 3. Contractor shall obtain all required building permits before commencing work.

4. Contractor shall be responsible for location of all existing utilities. The contractor

- shall contact all concerned utilities at least 48 hours in advance for construction operations.
- 5. No field changes or deviations from design to be made without prior approval of the engineer.
- 6. All construction shall be completed in accordance with the applicable ordinances of Glades County, Florida.
- 7. Contractor shall supply density tests to engineer on all sub-grade and base. Tests shall be prepared per AASHTO T-180 method.
- 8. Slope grades from elevations shown to existing grade at property line.
- 9. Engineer shall be notified at least 48 hours in advance for any inspection.
- 10. All traffic control devices shall be in accordance with M.U.T.C.D. Standards.
- 11. Erosion and sedimentation control techniques shall be incorporated during
- construction as follows: (1) silt screens shall be maintained at the project perimeter.
- (2) No off-site discharges shall occur during construction. In the event discharge is required, hay bales and/or turbidity curtains shall be incorporated at the discharge point as necessary to control turbidity.

#### **EROSION AND SEDIMENTATION CONTROL NOTES**

Construction activities can result in the generation of significant amounts of pollutants which may reach surface or ground waters. One of the primary pollutants of surface waters is sediment due to erosion. Excessive quantities of sediment which reach water bodies of floodplains have been shown to adversely affect their physical, biological and chemical properties. Transported sediment can obstruct stream channels, reduce hydraulic capacity of water bodies of floodplains, reduce the design capacity of culverts and other works, and eliminate ethic invertebrates and fish spawning substrates by siltation. Excessive suspended sediments reduce light penetration and therefore, reduce primary productivity.

#### MINIMUM STANDARDS

- 1. Sediment basin and traps, perimeter dikes, sediment barriers and other measures intended to trap sediment shall be constructed as a first step in any land-distributing activity and shall be made functional before unslope land disturbance takes place.
- 2. All sediment control measures are to be adjusted to meet field conditions at the time of construction and be constructed prior to any grading or disturbance of existing surface material on balance of site. Perimeter sediment barriers shall be constructed to prevent sediment or trash from flowing or floating on to adjacent properties.
- 3. Permanent or temporary soil stabilization shall be applied to denuded areas within seven days after final grade is reached on any portion of the site. Temporary soil stabilization shall be applied within seven days to denuded areas that may not be at final grade but will remain undisturbed for longer than 30 days. Permanent stabilization shall be applied to areas that are to be left undisturbed for more than one year.
- 4. During construction of the project, soil stockpiles shall be stabilized or protected with sediment trapping measures. The applicant is responsible for the temporary protection and permanent stabilization of all soil stockpiles on site as well as soil intentionally transported from the project site.
- 5. A permanent vegetative cover shall be established on denuded areas not otherwise permanently stabilized. Permanent vegetation shall not be considered established until a ground cover is achieved that, in the opinion of the Reviewer, is uniform, mature enough to survive and will inhibit erosion.
- 6. Stabilization measures shall be applied to earthen structures such as dams, dikes and diversions immediately after installation.

	F.B.V.		DENSITY		L.B.R.		THICKNESS	
	MAX. SPACING		MAX. SPACING		MAX. SPACING		MAX. SPACING	
	LINEAR FEET	SQUARE FEET	LINEAR FEET	SQUARE FEET	LINEAR FEET	SQUARE FEET	LINEAR FEET	SQUARE FEET
COMPACTED OR STABILIZED GRADE	200	5,000	200	5,000	200	5,000	300	10,000
ROCK BASE			300	10,000	300	10,000	300	10,000
SHELL ROCK			300	10,000			300	10,000
ASPHALT							PER INSP.	PER INSP.

#### **EROSION AND SEDIMENTATION CONTROL NOTES - (continued)**

- 7. Surface runoff from disturbed areas that is comprised of flow from drainage areas greater than or equal to three acres shall be controlled by a sediment basin. The sediment basin shall be designed and constructed to accommodate the anticipated sediment loading from the land-disturbing activity. The outfall device or system design shall take into account the total drainage area flowing through the disturbed area to be served by the basin.
- 8. After any significant rainfall, sediment control structures will be inspected for integrity. Any damaged devices shall be corrected immediately.
- 9. Concentrated runoff shall not flow down cut or fill slopes unless contained within an adequate temporary or permanent channel, flume or slope drain structure.
- 10. Whenever water seeps from a slope face, adequate drainage or other protection shall
- 11. Sediment will be prevented from entering any storm drain system, ditch or channel. All storm sewer inlets that are made operable during construction shall be protected so that sediment-laden water cannot enter the conveyance system without first being filtered or otherwise treated to remove sediment.
- 12. Before temporary or newly constructed stormwater conveyance channels are made operational, adequate outlet protection and any required temporary or permanent channel lining shall be installed in both the conveyance channel and receiving channel.
- 13. When work in a live watercourse is performed, precautions shall be taken to minimize encroachment, control sediment transport and stabilize the work area to the greatest extent possible during construction. Nonerodible material shall be used for the construction of causeways and cofferdams. Earthen fill may be used for these structures if armored by nonerodible cover materials.
- 14. When a live watercourse must be crossed by construction vehicles, a temporary stream crossing constructed of nonerodible material shall be provided.
- 15. The bed and banks of a watercourse shall be stabilized immediately after work in the watercourse is completed.
- 16. Periodic inspection and maintenance of all sediment control structures must be provided to ensure intended purpose is accomplished. The Developer, owner and/or contractor shall be continually responsible for all sediment leaving the property. Sediment control measures shall be in working condition at the end of each working day.
- 17. Underground utility lines shall be installed in accordance with the following standards in addition to other applicable criteria:
- A. No more than 500 linear feet of trench may be opened at one time.
- B. Excavated material shall be placed on the uphill side of trenches.
- C. Effluent from dewatering operations shall be filtered or passed through an approved sediment trapping device, or both, and discharged in a manner that does not adversely affect flowing streams or off-site property.
- D. Restabilization shall be accomplished in accordance with these regulations.
- 18. Where construction vehicle access routes intersect paved public roads, provisions shall be made to minimize the transport of sediment by tracking onto the paved surface, where sediment is transported onto a public road surface with curbs and gutters, the road shall be cleaned thoroughly at the end of each day. Sediment shall be removed from the roads by shoveling or sweeping and transported to a sediment control disposal area. Street washing shall be allowed only after sediment is removed in this manner. This provision shall apply to individual subdivision lots as well as to larger land-distributing activities.
- 19. All temporary erosion and sediment control measures shall be removed within 30 days after final site stabilization or after the temporary measures are no longer needed, in the opinion of the Reviewer. Disturbed soil areas resulting from the disposition of temporary measures shall be permanently stabilized to prevent further erosion and sedimentation.
- 20. Properties and waterways downstream from construction site shall be protected from sediment disposition and erosion.
- 21. Phased projects should be cleared in conjunction with construction of each phase.
- 22. Erosion control design and construction shall follow the requirements in Index Nos. 101, 102 and 103 of FDOT Roadway and Traffic Design Standards.
- 23. The Reviewer may approve modifications or alter plans to these erosion control criteria due to site specific conditions.

ENGINEER OF RECORD INSPECTION REQUIREMENTS CONTRACTOR TO CALL CONTRACT ENGINEER OF RECORD 48 HOURS ADVANCE FOR FOLLOWING INSPECTIONS: 1. PRECONSTRUCTION MEETING 2. DRAINAGE PIPE (UNCOVERED) 3. PAVEMENT SUBGRADE 4. PAVEMENT BASE

5. FINAL

#### Earthwork and Drainage Specifications

1. Clearing and Grubbing: Clearing and grubbing shall be performed within the limits of the project work in accordance with Section 110, Florida Department of Transportation (FDOT) Specifications. This item shall include, but is not limited to, the complete removal and legal disposal of all trees, brush, stumps, roots, grass, weeds, rubbish and other undesirable material to a depth of 18 inches below natural ground or proposed finished grade, whichever is lower. The areas to be cleared generally consist of the entire site with the exception of areas specifically noted on the landscape plans as preserve areas or as areas to remain un-cleared. Care shall be taken to insure that no preserve areas or wetland areas are impacted by the clearing operation. Prior to initiating the clearing operation, all adjacent wetland and preserve areas shall be marked and flagged in accordance with the City of Okeechobee and South Florida Water Management District (SFWMD) requirements All such areas immediately adjacent to the clearing operation shall also be protected by the installation of temporary silt barriers in accordance with the requirements of The City of Okeechobee and the SFWMD. Further erosion control shall be accomplished by seeding and mulching all disturbed areas as soon as they are at final grade, per the specifications for seeding and mulching found elsewhere on this sheet.

All material shall be removed from the site and shall be legally disposed of in accordance with all local, state and federal requirements.

- **2. Earthwork and Grading:** All earthwork and grading shall be performed as required to achieve the final grades, typical sections and elevations shown on the plans. In all other respects, materials and construction methods for earthwork, embankment, excavation and grading shall conform to the requirements of FDOT Specifications, Section 120. Any plastic or otherwise undesirable material within 36 inches of finished road grade shall be removed and replaced with suitable material. The contractor shall also refer to the Soils Report, if available. The specifications and recommendations included in that report shall be considered as a part of these plans and specifications. Should there be any conflict between that document and any requirements of these drawings or specifications, the most restrictive requirement shall govern.
- **3. Paving Improvements:** All areas proposed for paving shall be constructed in accordance with the design grades and typical sections shown on the drawings, and in conformance to the requirements of the City of Okeechobee and Florida Department of Transportation.
  - **A. Asphalt:** Prime Coat and tack coat for base course and between lifts of asphalt shall conform to the requirements of Sections 300-1 through 300-7 of the FDOT Specifications. Prime Coat shall be applied at a rate of 0.25 gallons per square yard and tack coat at a rate of 0.10 gallons per square yard, unless otherwise approved by the Engineer.

Asphalt surface course thickness and material shall be as shown on the typical sections and shall in all ways conform to the requirements of FDOT.

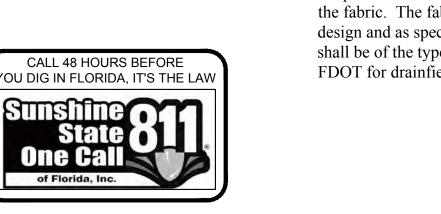
- **Base:** Limerock base material shall be compacted to 98% of maximum density per AASHTO T-180. All limerock shall meet the minimum requirements of FDOT Section 911. As an alternate, cemented coquina conforming to FDOT Section 915 may be substituted and shall be subject to the compaction specifications detailed above and included in the Soils Engineer's report.
- C. Sub-grade: Sub-grade shall be compacted to 98% of maximum density per AASHTO T-180, and stabilized to a minimum FBV of 50psi. Sub-grade shall be thoroughly rolled with a pneumatic tired roller prior to scheduling any sub-grade inspection.
- D. Valley Gutter/ F-Curb/D-Curb/Flush Curb: Shall be constructed per the typical section by extruding machine or forms as shown on the plans. Minimum concrete compressive strength shall be 3,000psi after 28 days. Sub-grade shall be moistened at the time concrete is placed to insure a uniformly damp surface. Ready-mix concrete shall have a slump of between 2 and 4 inches. No water shall be added to increase workability. Test cylinders shall be made for the strength testing of each batch of concrete for at least 7 and 28 day testing.
- **E.** Sod: A minimum of a two-foot wide strip of sod, or as otherwise shown on the plans, shall be placed along the back of curb of all constructed pavement to aid in prevention of erosion and soil stability. Sod shall be placed in conformance to FDOT Section 570, 575 and 981. Generally, the sodding requirements shall be as specified on the landscape plans, prepared by
- **F.** Seed, Fertilize and Mulch: All disturbed areas shall be stabilized with seed, fertilizer and mulch upon completion and acceptance by Engineer of final grading. Seed, fertilizer and mulch shall be in conformance to FDOT Sections 570, 575 and 981. The Contractor is responsible for establishing a stand of grass sufficient to prevent erosion prior to removal of the temporary silt fences. This applies only to those areas not covered by the sodding specified in the landscape plans, prepared by Others.
- G. Testing: The Contractor shall secure the services of an approved independent testing laboratory to conduct all required testing on sub-grade, base, asphalt and concrete. Locations required for these tests shall be as required by the City of Okeechobee, and/or in the case of the turn-lane improvements as required by the City of Okeechobee. At a minimum, testing shall be as recommended by FDOT. Should any tests fail, contractor shall at his own expense, repair the deficiencies and retest the work until compliance with the specifications is demonstrated.
- **H.** Traffic Control: The installation of Traffic Control Devices shall be in conformance to the requirements of the Manual of Uniform Traffic Control Devices, The City of Okeechobee. Maintenance of traffic During Construction shall be as required by FDOT.

#### **Earthwork and Drainage Specifications - (continued)**

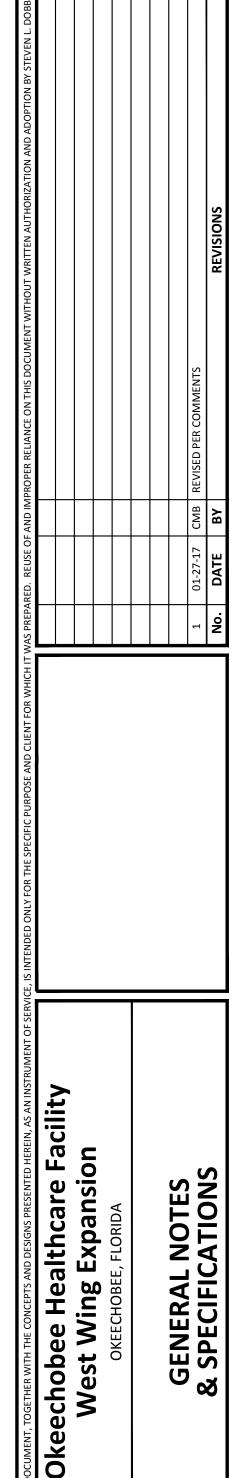
**4. Drainage Improvements:** All labor, materials and construction methods shall be in conformance to the minimum engineering and construction standards of the City of Okeechobee and FDOT Specifications. Trench excavation and back-filling operations shall meet or exceed the requirements of FDOT Specifications, Section 125. The Contractor shall provide the necessary back-fill compaction testing required to demonstrate compliance with this section. The pipe trench shall be dry when pipe is laid and the pipe shall be bedded per the details and per FDOT specifications.

The Contractor shall comply with Chapter 90-96, Laws of Florida, which requires the Contractor performing trench excavations over five feet in depth comply with all applicable trench safety standards and shoring requirements as set forth in the Occupational Safety and Health Administration's (OSHA) excavation and safety standards, 29 C.F.R. 19926.650, Sub-part P and incorporated as the State of Florida standard, as revised and/or updated. The cost of compliance with this requirement shall be included as a separate line item on the Contractor's bid. Otherwise, Contractor certifies that the cost of compliance is included in the unit cost of all items of work to which this requirement applies.

- **A. Reinforced Concrete Pipe (RCP):** RCP shall conform to the requirements of ASTM Specifications C-76, Class III, Wall Thickness "B", latest revision. All joints shall be soil-tight. Pipe gasket shall conform to FDOT Specifications, Section 942.
- **B.** Corrugated Metal Pipe (CMP): All CMP shall be Steel, round, helical-wound corrugated pipe conforming to AASHTO-M 36 and FDOT Section 943. Pipe ends at joints shall be reformed to a minimum of 2 annular corrugations for the complete band width. All joints shall be soil-tight. All connecting bands shall be corrugated annular coupling bands. A Neoprene gasket of at least 7 inches wide by 3/8 inch thick shall be used for all pipes of 36-inch diameter and smaller. Larger pipe sizes require gaskets of at least 10-1/2 inches in width. All CMP shall be installed at maximum lengths to reduce the number of joints.
- C. Corrugated Aluminum Pipe (CAP): All CAP shall be aluminum alloy, round, helical-wound corrugated pipe conforming to AASHTO-M 196 and FDOT Section 945. Pipe ends at joints shall be reformed to a minimum of 2 annular corrugations for the complete band width. All joints shall be soil-tight. All connecting bands shall be corrugated annular coupling bands. A Neoprene gasket of at least 7 inches wide by 3/8 inch thick shall be used for all pipes of 36-inch diameter and smaller. Larger pipe sizes require gaskets of at least 10-1/2 inches in width. All CAP shall be installed at maximum lengths to reduce the number of joints.
- **D.** Corrugated High Density Polyethylene Pipe (HDPE): All HDPE Pipe shall be resin conforming to ASTM D3350 minimum cell classification 435400C, round, only annular corrugations and conforming to FDOT Section 948-2.3. All joints shall be soil-tight. All connecting bands shall be corrugated annular coupling bands. A Neoprene gasket of at least 7 inches wide by 3/8 inch thick shall be used for all pipes of 36-inch diameter and smaller. Larger pipe sizes require gaskets of at least 10-1/2 inches in width. All HDPE shall be installed at maximum lengths to reduce the number of joints.
- Contech A-2000 PVC drainage pipe (A-2000): All A-2000 corrugated pipe with a smooth interior shall conform to the requirements of ASTM Designation F949 & F794 Dual Wall Corrugated Profile (DWCP) Pipe. Pipe and fittings shall be homogeneous throughout and free from visible cracks, holes, foreign inclusions or other injurious defects. Pipe shall be manufactured to 46 psi stiffness when tested in accordance with ASTM Test Method D2412. There shall be no evidence of splitting, cracking or breaking when the pipe is tested per ASTM Test Method D2412 and F949 section 7.5. The pipe shall be made of PVC compound having a minimum cell classification of 12454B as defined in ASTM Specification D1784.
- F. PVC Drainage Pipe: PVC Drainage Pipe shall be C-900 with push-on joints (no glued joints) and shall be as specified for sanitary sewer construction, except that it shall be white in color. Any portion of the PVC storm pipe that may be exposed to sunlight, such as its outlet to the detention pond, shall be painted to protect it from UV light.
- **G.** Inlets, Manholes, and Junction Boxes: All drainage inlets, manholes, and junction boxes shall be precast concrete conforming to ASTM C-478 and 64T. All concrete shall have not less than 4000-psi compressive strength at 28 days. Structure sections shall be joined with a mastic sealing compound. The remaining space shall be filled with the cement mortar and finished so as to produce a smooth continuous surface inside and outside the wall sections. All openings in precast structures shall be cast at the time of manufacture. Holes for piping shall be six inches larger than the outside diameter of the proposed pipe. All spaces between the manhole and the pipe shall be completely filled with mortar and finished smooth. Mortar used for concrete structures shall conform to M C-270. Mortar material shall be mixed one part Type 2 Portland cement to two parts aggregate by volume. Portland cement shall conform to ASTM C-144 and aggregate shall conform to ASTM C-144. The CONTRACTOR shall furnish the ENGINEER with shop drawings of all precast structures for his approval prior to fabrication. Shop drawings shall show all dimension, reinforcing steel and specifications. Storm Manholes shall be constructed with a traffic bearing cast-iron slotted grate.
- **H.** Trench Backfill shall be as shown in the Drainage Details. In addition, testing under paved areas shall be as follows: One test location midway between structures and one test location adjacent to each structure. Engineer may request additional locations. Testing in each location shall begin in the first foot above the culvert with tests every two feet to within two feet of the sub-grade. Density shall be to 100 percent of maximum as determined by AASHTO T-99.
- **Control Structures:** Shall be constructed per the above specifications for Inlets, Manholes, and Junction Boxes except that the structures shall include the bleeders and weirs as shown on the detail.
- **Rip-Rap Energy Dissipaters:** Shall be constructed per the details and as shown on the drawings at the control structures CS-1B and CS-2B, the downstream bubble-up structures. The rubble shall be of material and placed in accordance to FDOT Section 530-2.3 (material) and FDOT Section 530-3.3 (Construction Methods). Should broken concrete be used as the rubble, it shall be free from reinforcing bars or wire mesh. The contractor shall use care in the placement of the stone so that it is not dropped on thew fabric in such a fashion that tears the fabric. The fabric shall be as specified in FDOT Section 985 and shall be of the woven design and as specified for use with riprap per Table 1 of this section. The bedding stone shall be of the type typically used for drainfield rock and shall meet the requirements of FDOT for drainfield rock.







JOB No.: **2014-042 14** OF **14**